

# Solution

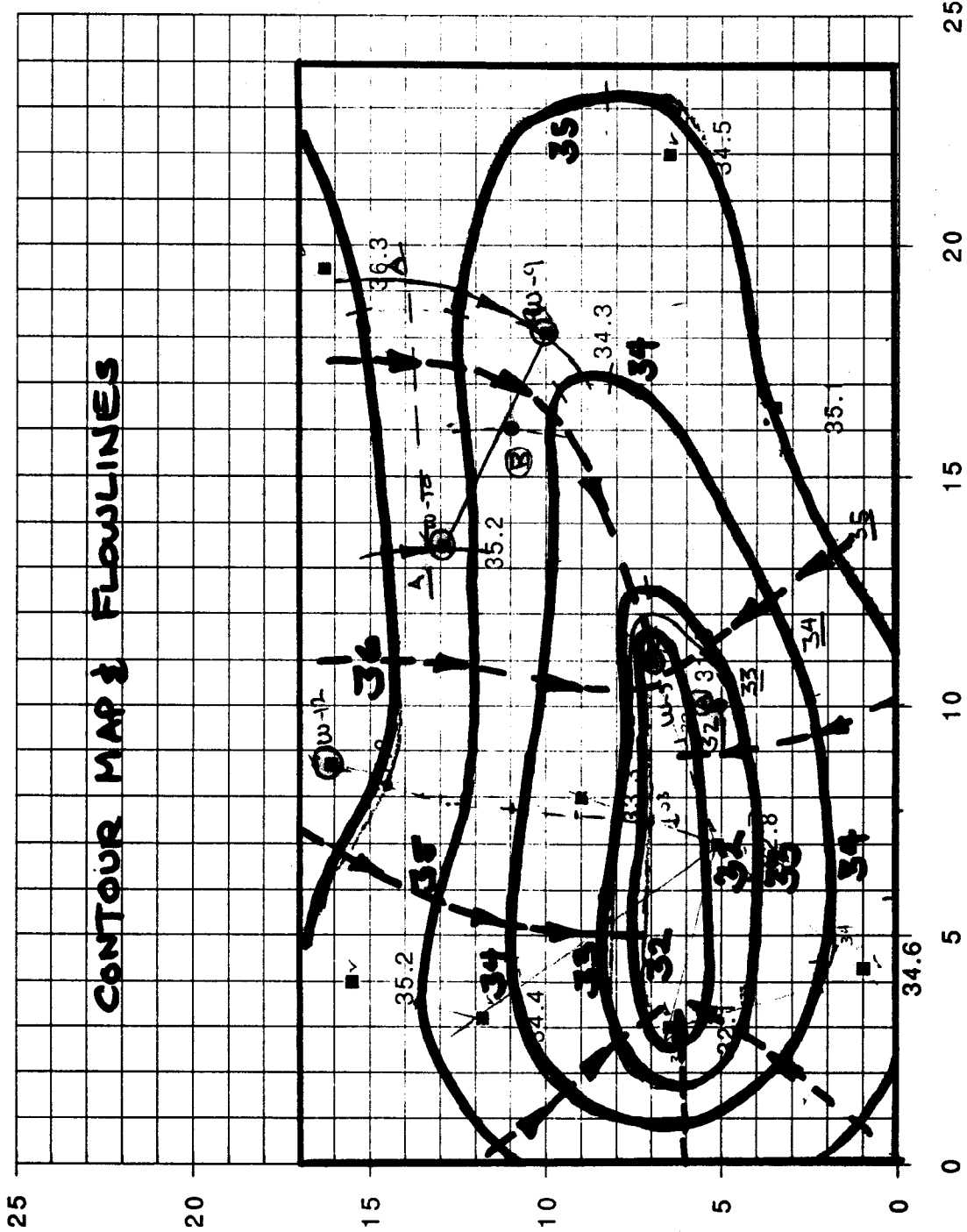
CE6361 Groundwater Hydrology , HW#4, Fall 1993 Due: \_\_\_\_\_

1) Piezometric heads are measured simultaneously in thirteen wells penetrating an isotropic confined aquifer of thickness  $B=50$  meters, hydraulic conductivity  $K = 20$  meters/day, and effective porosity of  $n=0.23$ .

Well	1	2	3	4	5	6	7	8	9	10	11	12	13
x	4.3	16.5	7.0	3.0	11.0	22.0	8.0	3.2	18.1	13.5	4.0	8.7	19.5
y	1.0	3.5	5.1	6.5	7.0	6.5	9.0	11.8	10.0	12.9	15.5	16.1	16.3
h	34.6	35.1	32.8	32.1	31.5	34.5	33.3	34.4	34.3	35.2	35.2	37.3	36.3

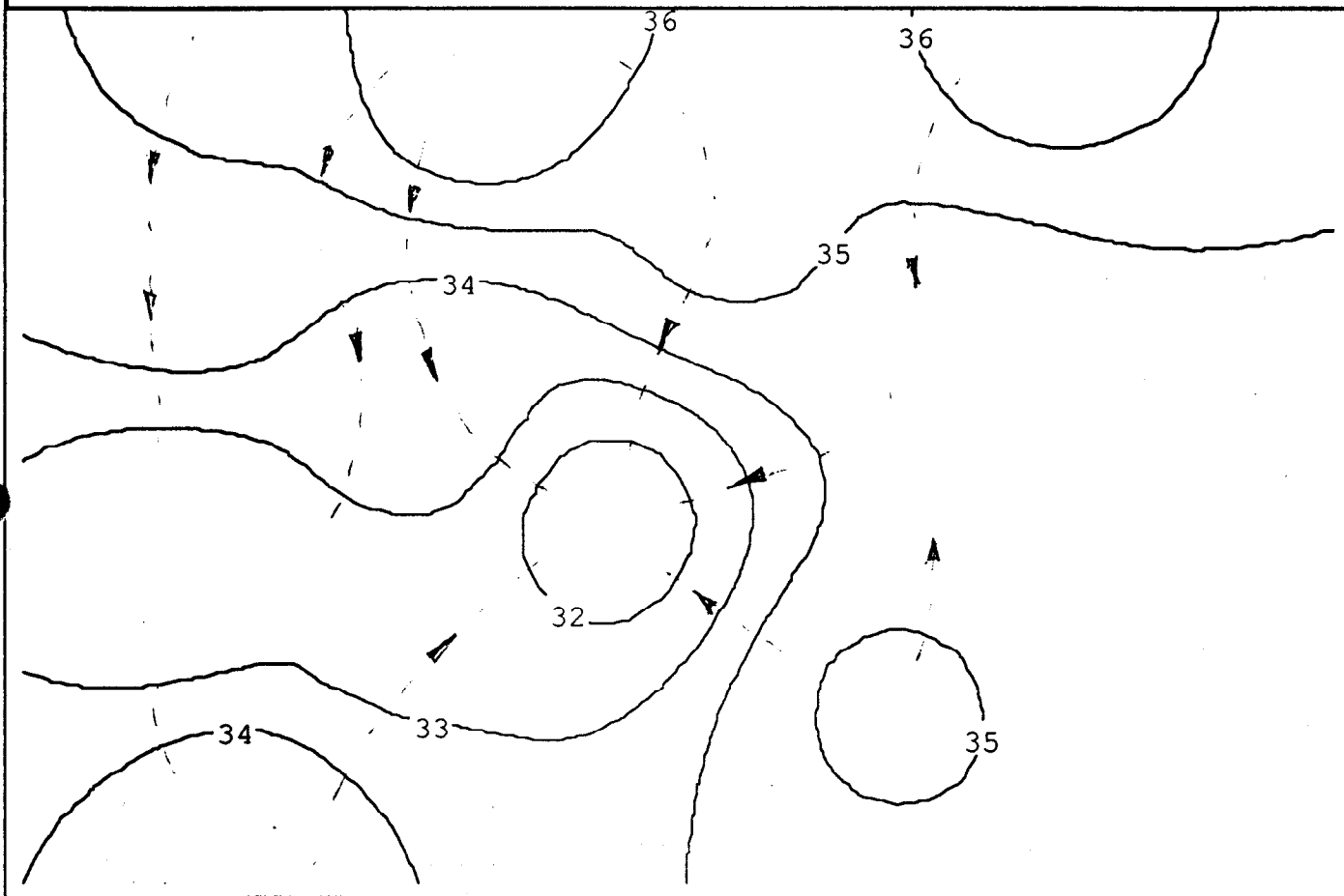
Each x,y coordinate unit = 200 meters

- ✓(a) Draw a contour map of the head distribution (1 meter contour intervals) and the flowlines.
- ✓(b) Use inverse-distance weighting to grid the data onto a  $40 \times 40$  grid (with the lower left corner of the grid at (0,0) ). Use the gridded data to draw a second contour map and compare the results to the map in part (a). What are the advantages to gridding data for mapping? What are the disadvantages?
- ✓(c) Using either map, determine the specific discharge (direction and magnitude) at points A(10,4) and B(16,11).
- ✓(d) Estimate the total flow through the aquifer between wells No. 10 and No. 9.
- ✓(e) Estimate the time of travel for a pollutant introduced into the aquifer in the vicinity of well No. 12 to reach a pumping well near well No. 5.



## B) CONTOUR MAP & FLOWLINES

### INVERSE DISTANCE GRIDDING



- b) - Results, Pumping becol more pronounced.
- Isolated points produce "injector" features that may not exist
  - advantages ① fast ② automatic
  - disadvantages Unless search distance is limited may identify non-existent features.

c) A(10, 4)

$$q = K \frac{dh}{dr}$$

$$dh = 1.0 \text{ m}$$

$$dr = (1.1)(200 \text{ m})$$

$$K = 20 \text{ m/d}$$

$$q = 0.0909 \text{ m/d} \quad \begin{array}{c} \uparrow \\ \text{BOTTOM} \\ \text{OF MAP} \end{array} \quad (\text{INTO PUMPING BOWL})$$

(OR SLIGHTLY W OF DUE NORTH)

B(16, 11)

$$q = K \frac{dh}{dr}$$

$$dh = 1.0$$

$$dr = (1.9)(200 \text{ m})$$

$$K = 20 \text{ m/d}$$

$$q = 0.0526 \text{ m/d} \quad \begin{array}{c} \text{TOP OF MAP} \\ \downarrow \end{array}$$

(OR SLIGHTLY W OF DUE SOUTH)

d) See map #1

By continuity, flow through section A-A on map same as through section (w-10)-(w-9).

$$Q = KA \frac{dh}{dl}$$

$$K = 20 \text{ m/d}$$

$$A = Bw, \quad B = 50 \text{ m} \quad w = (5.8)(200 \text{ m})$$

$$dh = 1.0 \text{ m}$$

$$dl = (2.0)(200 \text{ m})$$

$$Q = (20 \text{ m/d})(50 \text{ m})(200 \text{ m})(5.8) \frac{(1.0 \text{ m})}{(2.0)(200 \text{ m})}$$

$$Q = 2900 \text{ m}^3/\text{day}$$

e) See map #1

- gradient  $\approx$  constant <sup>slope</sup> except near pumping well.

$$\frac{dh}{dl} \approx \frac{1.0 \text{ m}}{(2.0)(200 \text{ m})} = 0.0025$$

$$v = \frac{q}{n_e} = \frac{K}{n_e} \frac{dh}{dl} = \left( \frac{20 \text{ m/d}}{0.23} \right) (0.0025) = 0.2174 \text{ m/d}$$

$$x = vt \quad \therefore \quad \frac{x}{v} = t$$

$$x \approx (7.7)(200 \text{ m}) = 1540 \text{ m}$$

$$t = \frac{1540 \text{ m}}{0.2174 \text{ m/d}} = 7083.7 \text{ days} \approx 19\frac{1}{2} \text{ years}$$