



# CE 3372 WATER SYSTEMS DESIGN

LESSON 11 PART 1: STORAGE AND EXTENDED PERIOD SIMULATION FALL 2020

# OVERVIEW

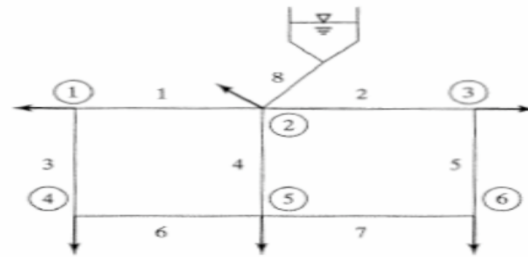
- EPANET Tank Model(s)
  - Single period simulation
- Multiple Period Simulation
  - Reasons
  - Multiplier Table

# READINGS

- EPA NET User Manual – how to model storage tanks in a water distribution system.
  - Interesting web-resources
    - <http://www.invisiblestructures.com/rainstore3.html>
    - <http://www.upout.com/blog/san-francisco-3/heres-what-it-looks-like-under-those-brick-circles-in-the-street>

# RECALL EARLIER EXAMPLE

Compute the discharge in each pipe and the pressure at each junction node for the 8-pipe system shown in Figure 1. The water surface elevation in the storage tank is 315.0 ft. Prepare your solution using EPA-NET. Report your results in U.S. Customary units. Identify the node with the lowest pressure in your solution. Include a transmittal letter with the solution.



Pipe Data

Pipe no.	Length		Diameter		Friction factor
	m	ft	mm	in.	
1	1,220	4,000	254	10	0.024
2	1,829	6,000	254	10	0.024
3	1,829	6,000	305	12	0.022
4	1,982	6,500	610	24	0.018
5	2,134	7,000	254	10	0.024
6	915	3,000	457	18	0.020
7	1,524	5,000	254	10	0.024
8	91	300	305	12	0.022

Junction Data

Junction node	Ground elevation		Demand	
	m	ft	¢ps	gpm
1	51.8	170	31.5	500
2	54.9	180	31.5	500
3	50.3	165	31.5	500
4	47.3	155	94.6	1,500
5	45.7	150	63.1	1,000
6	44.2	145	94.6	1,500

Figure 1: Network and Data for Problem 1

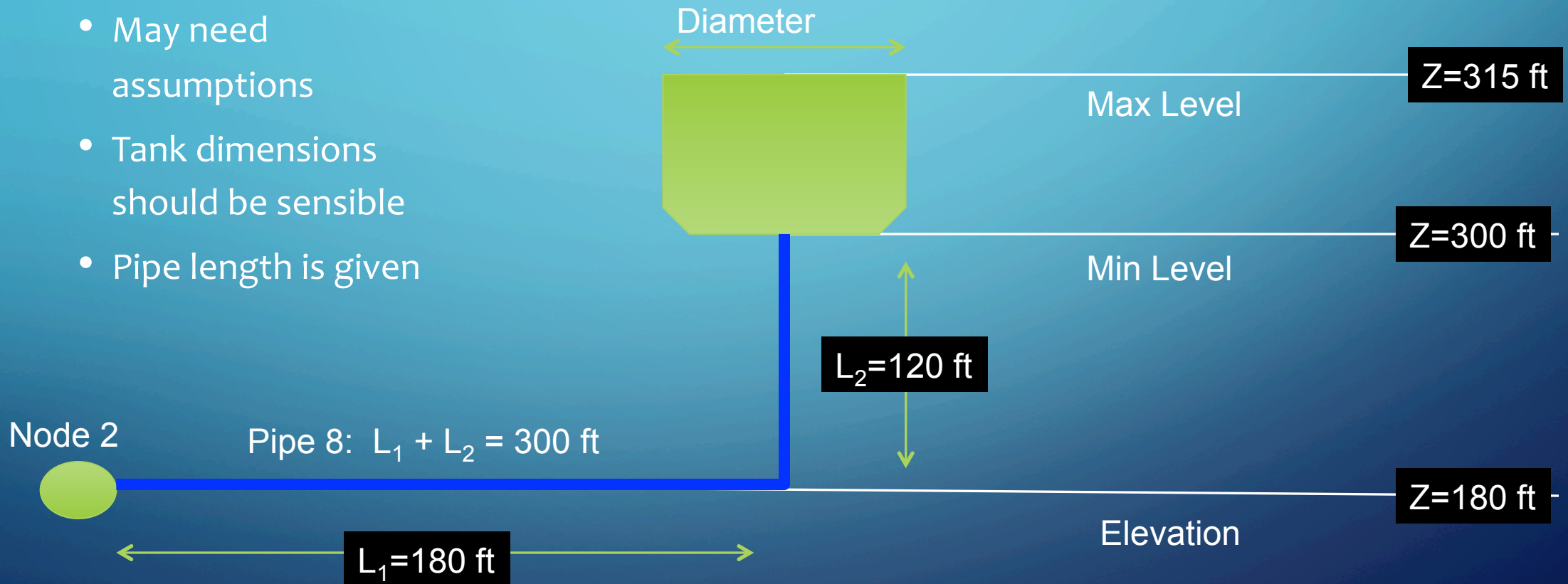
# MODELING PROTOCOL

- Sketch a layout on paper
- Identify pipe diameters; length; roughness values
- Identify node elevations; demands
- Supply reservoir (or tank); identify reservoir pool elevation
- Identify pumps; pump curve in problem units

# TANK

- Supply reservoir (or tank); identify reservoir pool elevation

- May need assumptions
- Tank dimensions should be sensible
- Pipe length is given



# EXTENDED PERIOD SIMULATION

- EPANET and similar programs find steady-flow solutions
- Extended period simulation produces a sequence of steady states with approximations for:
  - Tanks drain and fill
  - Pressures can change at beginning and end of a time interval
  - Pump operating points moving along a pump curve

# USES

- Extended period simulation used for:
  - Modeling pressure in systems during changing demand –usually at hourly time scale
  - Storage tank operation and sizing
  - Water quality simulation
    - EPANET can approximate water quality from multiple sources – has uses in
      - Water age in system
      - Detection of intrusions into a system
      - Severity of contamination (impact assessment)



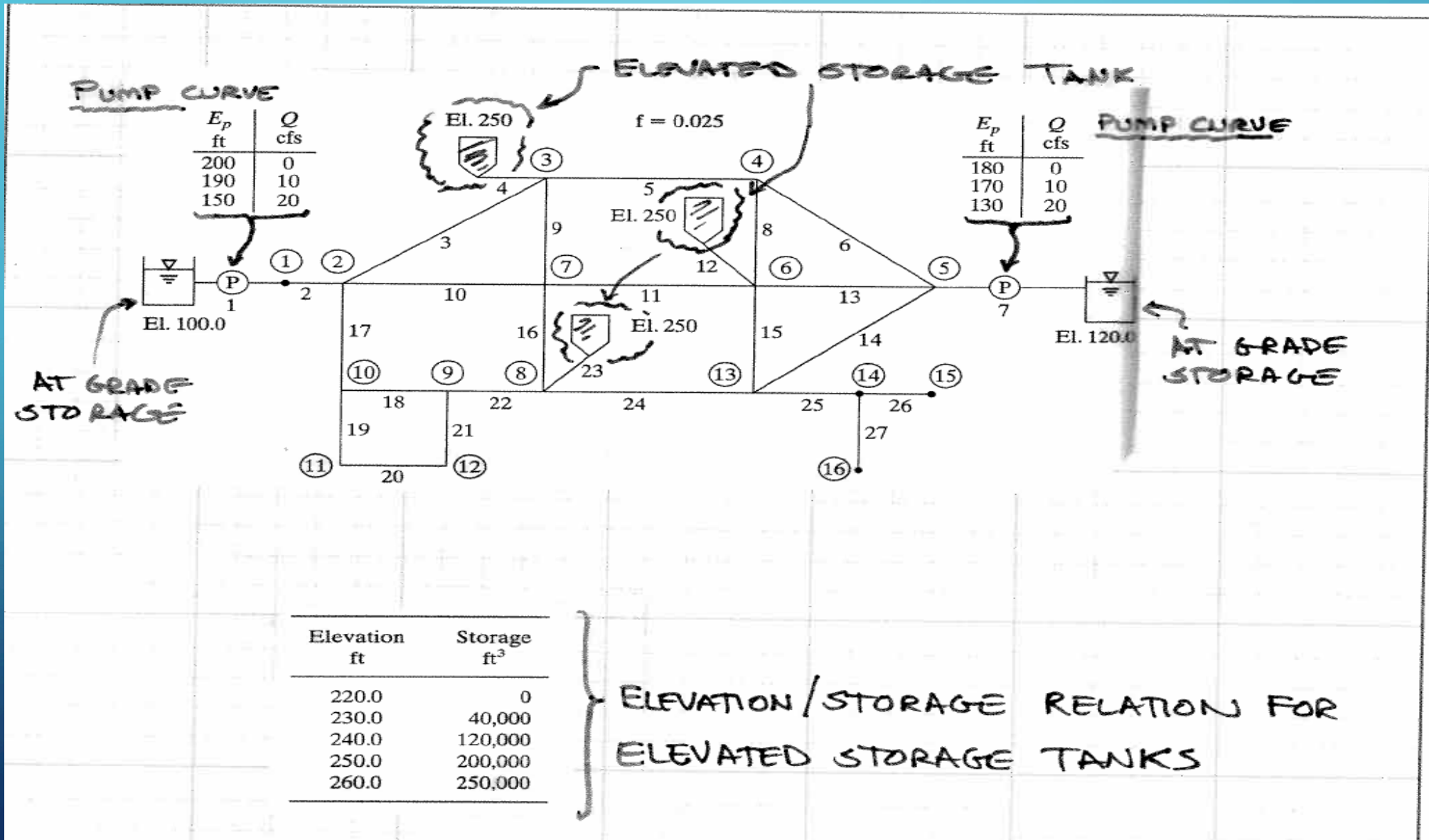
## HOW IMPLEMENTED?

- In EPANET assign a demand pattern to a node
- Set simulation times
- Program then follows the pattern

# MODELING PROTOCOL

- Sketch a layout on paper
- Identify pipe diameters; length; roughness values
- Identify node elevations; demands
- Supply reservoir (or tank); identify reservoir pool elevation
- Identify pumps; pump curve in problem units
- Identify demand pattern(s) and tank operating considerations

# EXAMPLE



# ILLUSTRATE BY EXAMPLE

Pipe no.	Node US	Node DS	Length (ft)	Diameter (in)	Minor loss coefficient	Fixed grade (ft)
1	0	1	2,000.0	24.0	0.5	100.0
2	1	2	800.0	24.0	0.0	
3	2	3	5,000.0	18.0	0.0	
4	3	0	700.0	18.0	0.5	250.0
5	3	4	3,700.0	12.0	0.0	
6	5	4	3,900.0	15.0	0.0	
7	0	5	2,100.0	24.0	0.5	120.0
8	6	4	2,500.0	10.0	0.0	
9	3	7	3,100.0	12.0	0.0	
10	2	7	5,500.0	18.0	0.0	
11	6	7	3,700.0	15.0	0.0	
12	0	6	900.0	18.0	0.5	250.0
13	5	6	2,900.0	15.0	0.0	
14	5	13	4,500.0	15.0	0.0	
15	6	13	2,500.0	15.0	0.0	
16	7	8	2,700.0	15.0	0.0	
17	2	10	3,100.0	18.0	0.0	
18	10	9	1,900.0	15.0	0.0	
19	10	11	1,600.0	8.0	0.0	
20	11	12	1,500.0	6.0	0.0	
21	9	12	1,650.0	8.0	0.0	
22	8	9	2,900.0	15.0	0.0	
23	0	8	1,900.0	18.0	7.5	250.0
24	13	8	3,100.0	15.0	0.0	
25	13	14	1,600.0	8.0	0.0	
26	14	15	1,750.0	6.0	0.0	
27	14	16	1,500.0	6.0	0.0	

PIPE CHARACTERISTICS

(ADJUST LOSS COEF. TO GET  $f = 0.025$  (given))

IN PRACTICAL CASE USE PIPE MATERIAL INFO.

# ILLUSTRATE BY EXAMPLE

Junction no.	Elevation (ft)	Demand (gpm)
1	90.00	0
2	110.00	694
3	95.00	694
4	105.00	2,083
5	100.00	694
6	103.00	2,428
7	97.00	2,083
8	103.00	1,044
9	107.00	0
10	112.00	0
11	115.00	350
12	112.00	350
13	110.00	0
14	120.00	0
15	135.00	175
16	130.00	175

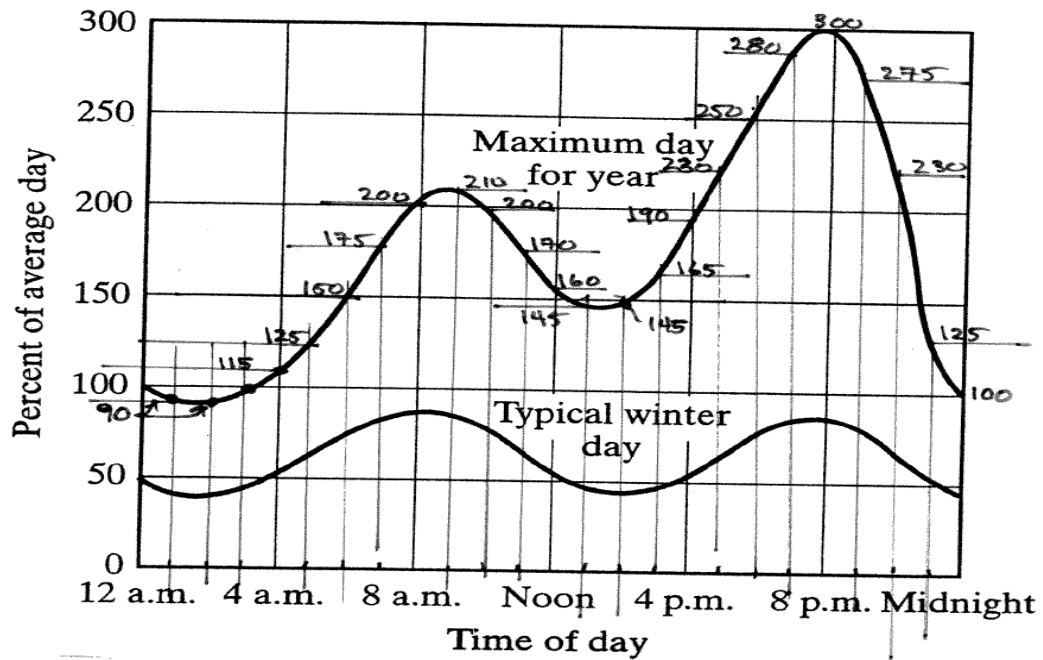
DEMAND (CFS)

0  
1.54  
1.54

$$\frac{1 \text{ gal}}{\text{min}} \times \frac{\text{ft}^3}{7.48 \text{ gal}} \times \frac{1 \text{ min}}{60 \text{ sec}}$$

BASE DEMAND &  
NODE TOPOGRAPHY

# ILLUSTRATE BY EXAMPLE



DEMAND MULTIPLIERS — READ FROM CHART FOR HOUR OF DAY,  
 BUILD MULTIPLIER TABLE

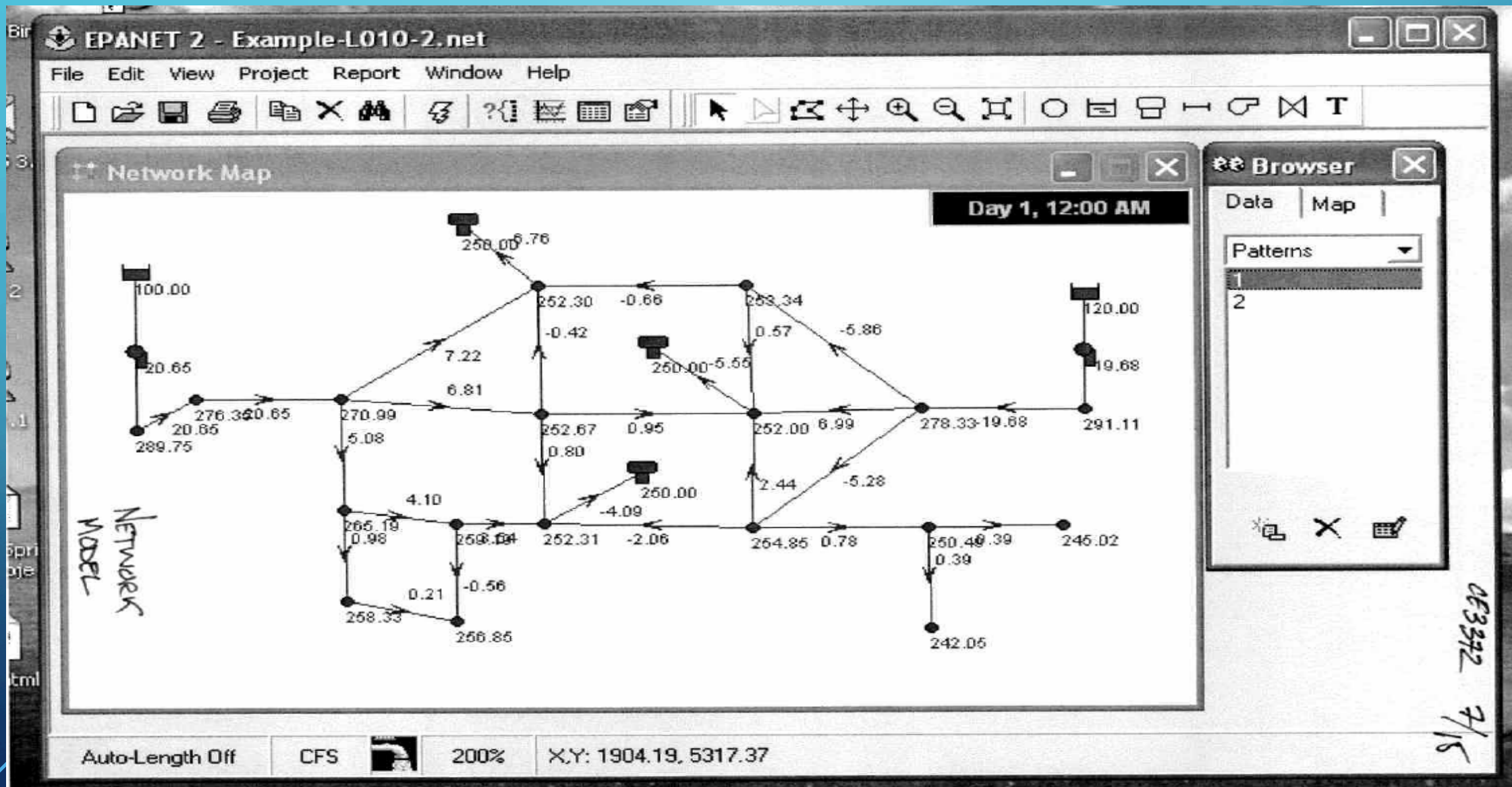
Hour	FACTOR (MULTIPLIER)	CLOCK TIME
1	1.0	0:00
2	0.9	1:00
3	0.9	2:00
4	1.0	3:00
5	1.15	4:00
6	1.25	5:00
7	1.5	6:00
8	1.75	7:00
9	2.0	8:00
10	2.10	9:00
11	2.0	10:00
12	1.7	11:00
13	1.6	12:00
14	1.45	13:00
15	1.45	14:00
16	1.65	15:00
17	1.90	16:00
18	2.30	17:00
19	2.50	18:00
20	2.80	19:00
21	3.00	20:00
22	2.75	21:00
23	2.30	22:00
24	1.25	23:00

REPEATS CYCLE

# ILLUSTRATE BY EXAMPLE

- Build network layout
  - Nodes (junctions, tanks, reservoirs)
  - Links (pipes, pumps, valves)
- Add pump curves
  - BROWSER/DATA/CURVES/ADD (TYPE=PUMP)
- Add storage curves
  - BROWSER/DATA/CURVES/ADD (TYPE=STORAGE)
- Add demand pattern(s)
  - BROWSER/PATTERNS/ADD

# ILLUSTRATE BY EXAMPLE





# ILLUSTRATE BY EXAMPLE

The screenshot shows the EPANET 2 software interface. The main window is titled "EPANET 2 - Example-L010-2.net". A "Pattern Editor" dialog box is open, showing a table with 8 time periods, all with a multiplier of 1. Below the table is a graph showing a constant demand of 1.00 over 24 hours. The background shows a network map with a demand node and handwritten annotations.

**Pattern Editor**

Pattern ID	Description							
1	Demand Pattern Constant							
Time Period	1	2	3	4	5	6	7	8
Multiplier	1	1	1	1	1	1	1	1

**Graph Data:**

Time (hrs)	Demand
0	1.00
1	1.00
2	1.00
3	1.00
4	1.00
5	1.00
6	1.00
7	1.00
8	1.00
9	1.00
10	1.00
11	1.00
12	1.00
13	1.00
14	1.00
15	1.00
16	1.00
17	1.00
18	1.00
19	1.00
20	1.00
21	1.00
22	1.00
23	1.00
24	1.00

**Network Map Annotations:**

- Node values: 100.00, 20.65, 276.36, 20.65, 289.75
- Handwritten text: "BASE DEMAND" (vertical), "CE3372 8/5" (vertical)

# ILLUSTRATE BY EXAMPLE

EPANET 2 - Example-L010-2.net

File Edit View Project Report Window Help

Network Map

Times Options 12:00 AM

Property	Hrs:Min
Total Duration	24:00 ← CHANGE
Hydraulic Time Step	1:00
Quality Time Step	0:05
Pattern Time Step	1:00
Pattern Start Time	0:00
Reporting Time Step	1:00
Report Start Time	0:00
Clock Start Time	12 am (CLOCKED)
Statistic	None

100.00  
20.65  
278.340  
20.65  
289.75

120.00  
19.68  
291.11  
245.02

(EPS) SEMIUS  
MULTIPLE PERIOD

CE3372 9/15

Auto-Length Off CFS 200% X,Y: -4976.05, 6916.17

# ILLUSTRATE BY EXAMPLE

EPANET 2 - Example-L010-2.net

File Edit View Project Report Window Help

Network Map

**Curve Editor**

Curve ID: E Description: Storage Tank Depth above Tank bottom

Curve Type: VOLUME Equation:

Height	Volume
0	0
10	40000
20	120000
30	200000
40	250000
50	320000

Volume (cubic ft)

Height (ft)

TANK STORAGE CURVE

I ADDED THIS POINT

ORIGINAL PROBLEM NOT BIG ENOUGH TANKS

CE3392

Auto-Length Off CFS 200% X,Y: 1904.19, 5317.37

# ILLUSTRATE BY EXAMPLE

The screenshot shows the EPANET 2 software interface. The main window is titled "EPANET 2 - Example-L010-2.net". A "Pattern Editor" dialog box is open, showing the configuration for a "Variable Demand" pattern. The dialog includes a table of multipliers for 8 time periods and a bar chart showing the multiplier values over a 24-hour period. The average multiplier is 1.76. The background shows a network map with a demand node and handwritten notes.

Time Period	1	2	3	4	5	6	7	8
Multiplier	1	.9	.9	1	1.15	1.25	1.5	1.7

Avg. = 1.76

Time (Time Period = 1:00 hrs)

Handwritten notes on the network map: "VARIABLE DEMANDS" and "C83372 1/15".

# REPORT OUTPUT

12/15

Page 1 9/27/2010 7:02:32 PM  
.....  
\* E P A N E T \*  
\* Hydraulic and Water Quality \*  
\* Analysis for Pipe Networks \*  
\* Version 2.0 \*  
.....

FIRST FEW PAGES  
OF FULL STATUS REPORT

Input File: Example-L010-2.net

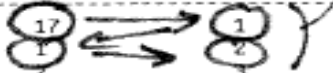
## Link - Node Table:

Link ID	Start Node	End Node	Length ft	Diameter in
3	17	1	2000	24
4	1	2	800	24
5	2	3	5000	18
6	3	4	3700	12
7	4	5	3900	15
8	5	18	2100	24
9	5	6	2900	15
10	4	6	2500	10
11	3	7	3100	12
12	2	7	5500	18
13	7	6	3700	15
14	2	10	3100	18
15	10	9	1900	15
16	9	8	2900	15
17	8	13	3100	15
18	13	14	1600	8
19	14	15	1750	6
20	14	16	2700	6
21	13	6	2500	15
22	13	5	4500	15
23	10	11	1600	8
24	11	12	1500	6
25	12	9	1650	8
26	7	8	2700	15
29	StorageTank1	3	700	18
30	StorageTank2	6	900	18
31	StorageTank3	8	1900	18
1	Left_Reservoir	17	#N/A	#N/A Pump
2	Right_Reservoir	18	#N/A	#N/A Pump

SKILLED USER CAN INFER NETWORK  
FROM START & END NODES

TOPOLOGY

PIPE#



# REPORT OUTPUT

Page 2  
Energy Usage:

Pump	Usage Factor	Avg. Effic.	Kw-hr /Mgal	Avg. Kw	Peak Kw	Cost /day
1	100.00	75.00	802.66	438.17	452.22	0.00
2	100.00	75.00	714.04	378.48	391.05	0.00
Demand Charge:						0.00
Total Cost:						0.00

1<sup>st</sup> STRESS PERIOD

Node Results at 0:00 Hrs:

Node ID	Demand CFS	Head ft	Pressure psi	Quality
1	0.00	276.35	80.75	0.00
2	1.54	270.99	69.76	0.00
3	1.54	252.30	68.16	0.00
4	4.64	253.34	64.27	0.00
5	1.54	278.33	77.27	0.00
6	5.40	252.00	64.56	0.00
7	4.64	252.67	67.45	0.00
8	2.32	252.31	64.70	0.00
9	0.00	259.19	65.94	0.00
10	0.00	265.19	66.38	0.00
11	0.77	258.33	62.11	0.00
12	0.77	256.85	62.76	0.00
13	0.00	254.85	62.76	0.00
14	0.00	250.49	56.54	0.00
15	0.39	245.02	47.67	0.00
16	0.39	242.05	48.55	0.00
17	0.00	289.75	82.22	0.00
18	0.00	291.11	74.14	0.00
Left_Reservoir	-20.65	100.00	0.00	0.00 Reservoir
Right_Reservoir	-19.68	120.00	0.00	0.00 Reservoir
StorageTank1	6.76	250.00	13.00	0.00 Tank

PRESSURES IN PSI

HEADS

# REPORT OUTPUT

StorageTank2 5.55 250.00 13.00 0.00 Tank  
StorageTank3 4.09 250.00 13.00 0.00 Tank

TANK BEHAVIOR

Link Results at 0:00 Hrs:

TANKS FILLING

Link ID	Flow CFS	Velocity fps	Unit Headloss ft/Kft	Status
3	20.65	6.57	6.70	Open
4	20.65	6.57	6.70	Open
5	7.22	4.09	3.74	Open
6	-0.66	0.84	0.28	Open
7	-5.86	4.78	6.41	Open
8	-19.68	6.26	6.09	Open
9	6.99	5.70	9.08	Open

Page 3

Link Results at 0:00 Hrs: (continued)

Link ID	Flow CFS	Velocity fps	Unit Headloss ft/Kft	Status
10	0.57	1.04	0.54	Open
11	-0.42	0.54	0.12	Open
12	6.81	3.85	3.33	Open
13	0.95	0.77	0.18	Open
14	5.08	2.88	1.87	Open
15	4.10	3.34	3.16	Open
16	3.54	2.89	2.37	Open
17	-2.06	1.68	0.82	Open
18	0.78	2.23	2.73	Open
19	0.39	1.99	3.12	Open
20	0.39	1.99	3.12	Open
21	2.44	1.99	1.14	Open
22	-5.28	4.31	5.22	Open
23	0.98	2.82	4.29	Open
24	0.21	1.09	0.99	Open
25	-0.56	1.59	1.42	Open
26	0.80	0.65	0.13	Open
29	-6.76	3.82	3.28	Open
30	-5.55	3.14	2.22	Open
31	-4.09	2.31	1.22	Open

1	20.65	0.00	-189.75	Open Pump
2	19.68	0.00	-171.11	Open Pump

PUMP BEHAVIOR

Node Results at 1:00 Hrs:

Node ID	Demand CFS	Head ft	Pressure psi	Quality
1	0.00	280.04	82.34	0.00
2	1.39	274.94	71.47	0.00
3	1.39	257.10	70.24	0.00
4	4.18	258.53	66.52	0.00

NEXT STRESS PERIOD

# REPORT OUTPUT

30	-5.55	3.14	2.22	Open
31	-4.09	2.31	1.22	Open
1	20.65	0.00	-189.75	Open Pump
2	19.68	0.00	-171.11	Open Pump

PUMP BEHAVIOR

NEXT STRESS PERIOD

## Node Results at 1:00 Hrs:

Node ID	Demand CFS	Head ft	Pressure psi	Quality
1	0.00	280.04	82.34	0.00
2	1.39	274.94	71.47	0.00
3	1.39	257.10	70.24	0.00
4	4.18	258.53	66.52	0.00
5	1.39	281.90	78.82	0.00
6	4.86	256.42	66.48	0.00
7	4.18	257.20	69.41	0.00
8	2.09	256.32	66.43	0.00
9	0.00	263.43	67.78	0.00
10	0.00	269.35	68.18	0.00
11	0.69	263.37	64.29	0.00
12	0.69	261.74	64.88	0.00
13	0.00	259.08	64.59	0.00
14	0.00	255.52	58.72	0.00
15	0.35	251.06	50.29	0.00
16	0.35	248.63	51.40	0.00
17	0.00	292.78	83.53	0.00
18	0.00	294.01	75.40	0.00
Left_Reservoir	-20.13	100.00	0.00	0.00 Reservoir

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## Node Results at 1:00 Hrs: (continued)

Node ID	Demand CFS	Head ft	Pressure psi	Quality
Right_Reservoir	-19.14	120.00	0.00	0.00 Reservoir
StorageTank1	6.66	254.87	15.11	0.00 Tank
StorageTank2	6.12	253.99	14.73	0.00 Tank
StorageTank3	4.95	252.94	14.27	0.00 Tank

## Link Results at 1:00 Hrs:

Link ID	Flow CFS	Velocity fps	Unit Headloss ft/Kft	Status
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# REPORT OUTPUT

```
1          19.25      0.00   -197.62   Open Pump
2          18.32      0.00   -178.24   Open Pump
```

## Node Results at 6:00 Hrs:

Node ID	Demand CFS	Head ft	Pressure psi	Quality
1	0.00	285.74	84.82	0.00
2	2.31	281.07	74.12	0.00
3	2.31	268.66	75.25	0.00
4	6.96	265.69	69.63	0.00
5	2.31	286.57	80.84	0.00
6	8.10	266.66	70.91	0.00
7	6.96	266.48	73.43	0.00
8	3.48	266.24	70.73	0.00
9	0.00	270.36	70.78	0.00
10	0.00	275.37	70.79	0.00
11	1.15	263.58	64.38	0.00
12	1.15	262.89	65.38	0.00
13	0.00	268.12	68.52	0.00
14	0.00	258.51	60.02	0.00
15	0.58	246.50	48.31	0.00
16	0.58	239.97	47.65	0.00
17	0.00	297.44	85.55	0.00
18	0.00	297.78	77.03	0.00
Left_Reservoir	-19.28	100.00	0.00	0.00 Reservoir
Right_Reservoir	-18.41	120.00	0.00	0.00 Reservoir
StorageTank1	1.37	268.55	21.04	0.00 Tank
StorageTank2	-1.24	266.76	20.26	0.00 Tank
StorageTank3	1.66	265.85	19.86	0.00 Tank

TANK DRAINING



# REPORT OUTPUT

23	1.50	3.72	7.30	Open
24	0.14	0.73	0.46	Open
25	-1.01	2.90	4.52	Open
26	0.64	0.52	0.09	Open
29	-1.37	0.78	0.15	Open
30	1.24	0.70	0.12	Open
31	-1.66	0.94	0.21	Open
1	19.28	0.00	-197.44	Open Pump
2	18.41	0.00	-177.78	Open Pump

) PUMPS PRODUCING LESS Q

Node Results at 7:00 Hrs:

Node ID	Demand CFS	Head ft	Pressure psi	Quality
1	0.00	285.05	84.51	0.00
2	2.69	280.32	73.80	0.00
3	2.69	269.26	75.51	0.00
4	8.12	262.59	68.28	0.00
5	2.69	284.82	80.08	0.00
6	9.45	265.78	70.53	0.00
7	8.12	265.85	73.16	0.00
8	4.06	266.43	70.81	0.00
9	0.00	269.68	70.49	0.00
10	0.00	274.44	70.39	0.00
11	1.35	259.39	62.56	0.00
12	1.35	258.86	63.63	0.00

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Node Results at 7:00 Hrs: (continued)

Node ID	Demand CFS	Head ft	Pressure psi	Quality
13	0.00	267.47	68.23	0.00
14	0.00	254.47	58.27	0.00
15	0.68	238.23	44.73	0.00
16	0.68	229.42	43.08	0.00
17	0.00	296.87	85.31	0.00
18	0.00	296.36	76.42	0.00
Left_Reservoir	-19.39	100.00	0.00	0.00 Reservoir
Right_Reservoir	-18.69	120.00	0.00	0.00 Reservoir
StorageTank1	-0.20	269.26	21.34	0.00 Tank
StorageTank2	-2.25	266.13	19.99	0.00 Tank
StorageTank3	-1.36	266.70	20.23	0.00 Tank

] TANKS DRAINING

Link Results at 7:00 Hrs:

Link ID	Flow CFS	Velocity fps	Unit Headloss ft/Kft	Status
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# CE 3372 WATER SYSTEMS DESIGN

LESSON 11 PART 2 : WATER QUALITY IN EPANET FALL 2020

# OUTLINE

- EPA-NET Water Quality Models

- Theory:

- Advective Transport in Pipeline
- Decay

- Practice:

- Estimate water age in system
- Estimate concentration of constituent at different points in network
- Respond to intrusions into the system

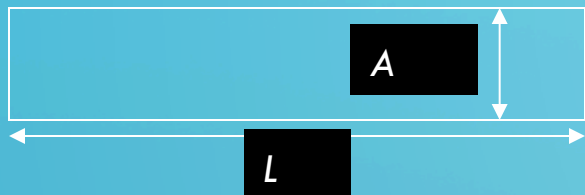
# WATER QUALITY IN EPANET

- Transport theory in EPANET
  - Lagrangian Approach  
(Discrete Parcel Advection)
  - Mixing Approach (in tanks)

# ADVECTIVE TRANSPORT

- Advection (convection) is the transport of dissolved or suspended material by motion of the host fluid.
- Requires knowledge of the fluid velocity field (the velocity of a fluid particle)
- Velocity from EPANET hydraulics

# MEAN SECTION VELOCITY



Pipe Segment:  
Volume =  $L \cdot A$   
Flow Rate =  $Q$

Displacement of one segment  
volume takes a certain time,  $\Delta t$

$$\Delta t = \frac{L \cdot A}{Q}$$

Distance traveled by marker is segment  
length,  $L$ ;  
Marker velocity is distance/time

$$u = \frac{Q}{A}$$

$Q$

$t=0$

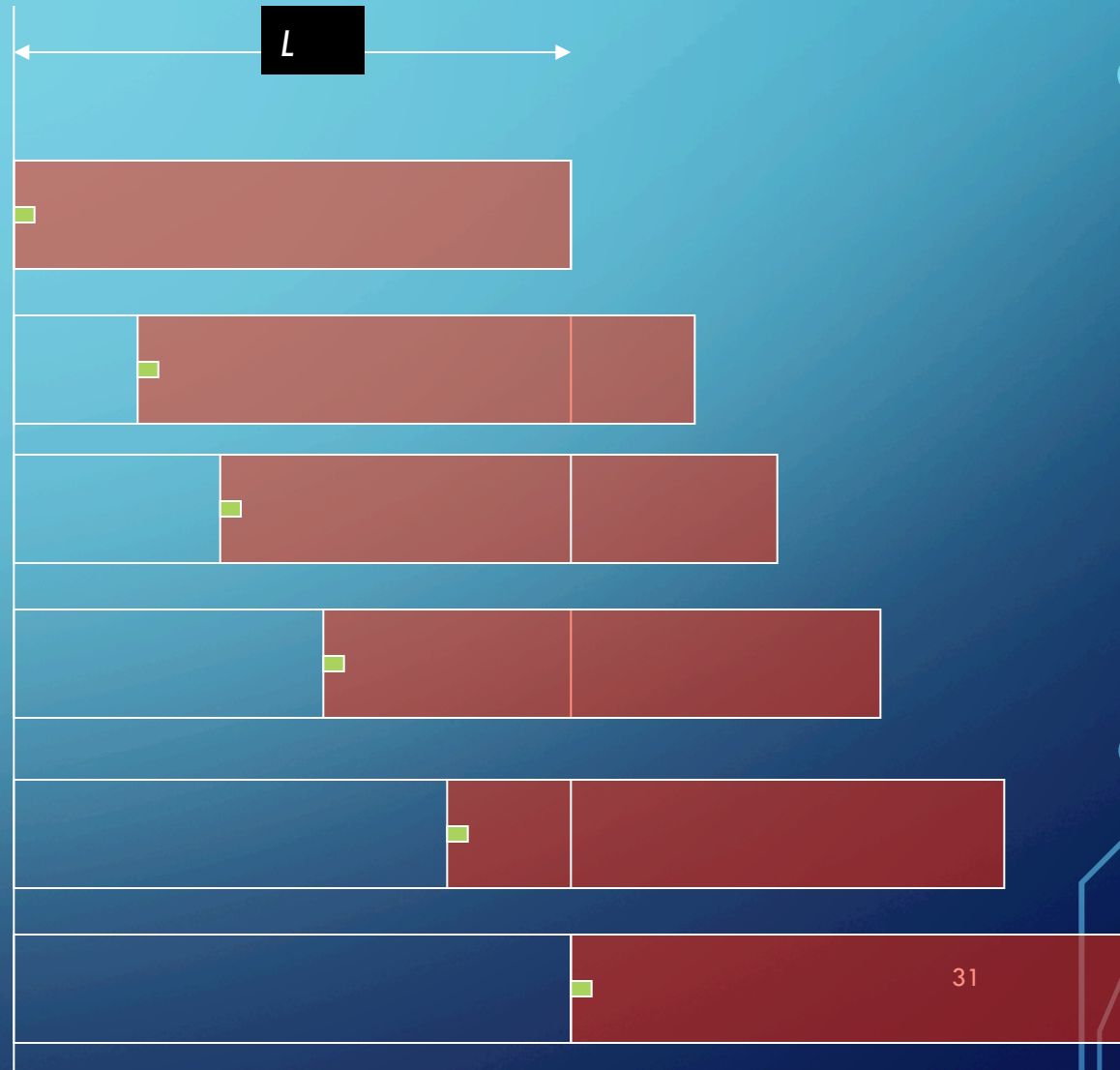
$t=0.2 \Delta t$

$t=0.4 \Delta t$

$t=0.6 \Delta t$

$t=0.8 \Delta t$

$t=1.0 \Delta t$



# MASS FLUX



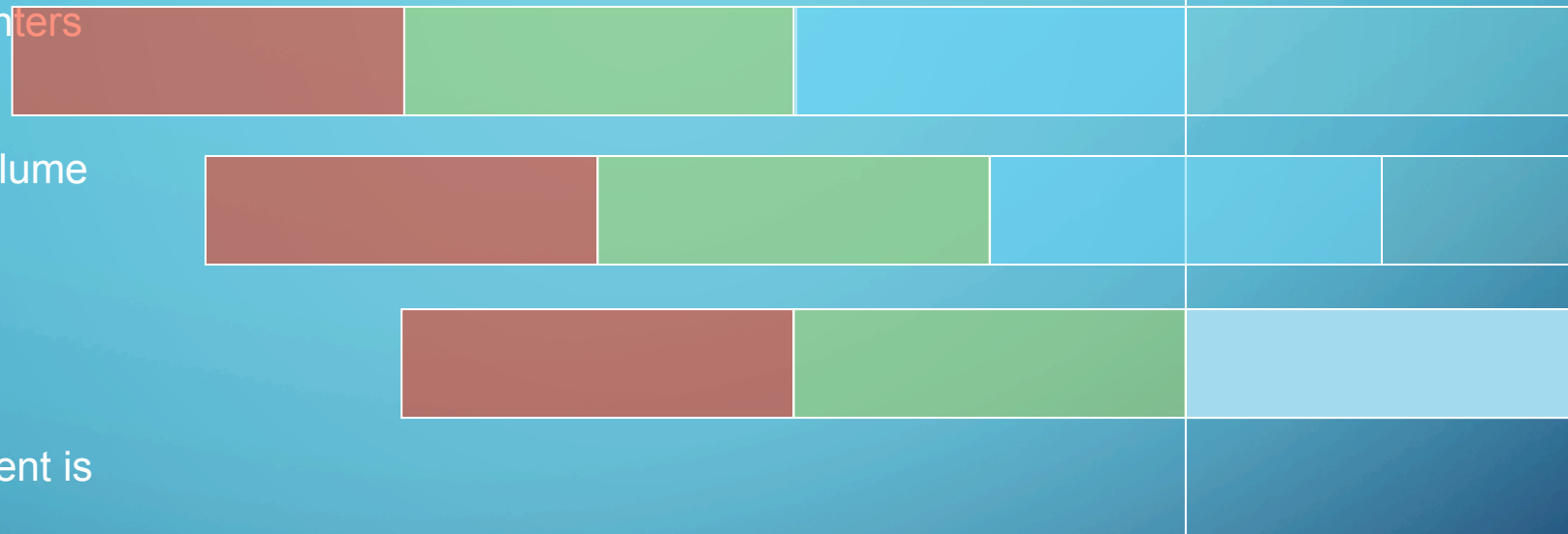
Suppose one “blue” volume enters the pipe segment.

The mass of “blue” per unit volume is the concentration of blue.

Let one pipe volume enter the segment.

Total mass of blue in the segment is the concentration\*fluid volume

Rate of blue entering the segment is mass/time



$t=0$

$t=0.5\Delta t$

$t=1.0\Delta t$

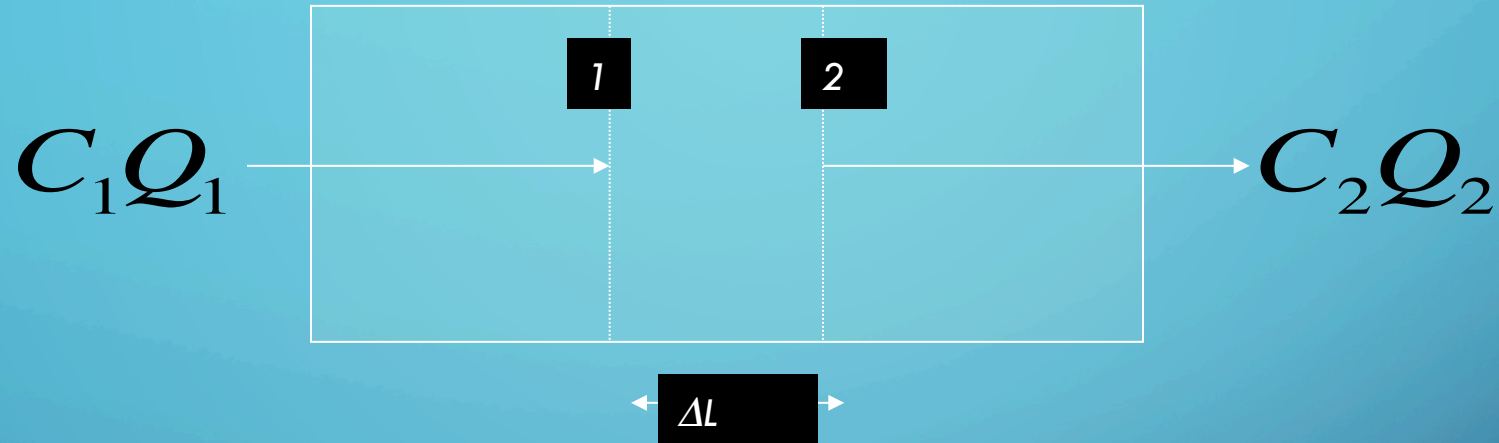
$$M_{blue} = ALC_{blue}$$

$$\frac{M_{blue}}{\Delta t} = \frac{ALC_{blue}Q}{AL} = C_{blue}Q$$



# MASS BALANCE

Now consider a small portion of the pipe.



Mass flow into segment.

Mass flow out of segment.

Rate of accumulation in segment.

$$C_1 Q_1 - C_2 Q_2 = \frac{\partial}{\partial t} [CA\Delta L]$$

# BALANCE EQUATIONS

For a non-deforming medium this mass balance is expressed as:

$$\frac{C_1 Q_1 - C_2 Q_2}{A \Delta L} = \frac{\partial C}{\partial t}$$

Substituting the definition of average linear velocity:

$$\frac{C_1 u_1 - C_2 u_2}{\Delta L} = \frac{\partial C}{\partial t}$$

Taking the limit as  $\Delta L$  vanishes produces the fundamental equation governing convective transport.

$$-\frac{\partial (uC)}{\partial L} = \frac{\partial C}{\partial t}$$

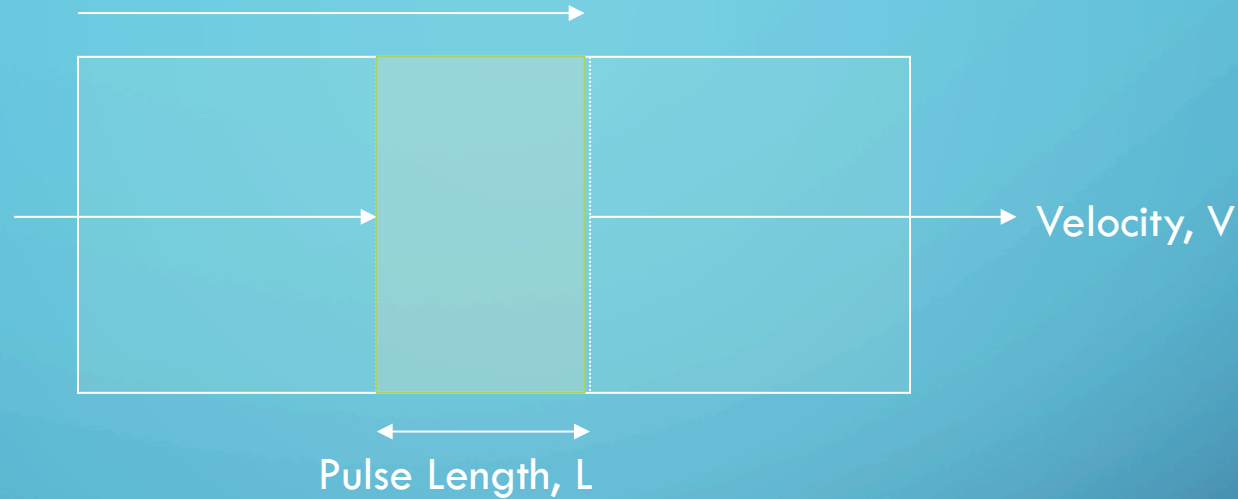
## GENERALIZATION

$$\frac{\partial C}{\partial t} = -\text{div}(\vec{U}C)$$

- Express last term in more conventional form – divergence of the mass flux is equal to the rate of change of concentration at a point.
- Observe the obvious dependence on the velocity field  $(u, v, w)$ .
- In order to compute any mass fluxes we must first determine the velocity values in the domain of interest.

# ANALYTICAL MODEL

Distance along flow path,  $x$



- Water at a constant velocity,  $V$ , is flowing through the zone carrying the dissolved component at a specific concentration,  $C_0$ .
- There is no degradation of the component, no dispersion of the component, nor is there any interaction with the solid phase (walls).
- The zone translates in space at a rate determined by the water velocity.
- The contaminant is dissolved, and does not alter the density of the flowing water.
- The contaminant is assumed to be uniformly mixed in contaminated zone.

# GOVERNING EQUATIONS, INITIAL, AND BOUNDARY CONDITIONS

The governing equation of mass transport for this case is:

$$\frac{\partial C}{\partial t} = -v \frac{\partial C}{\partial x}$$

The initial conditions throughout the pipe segment are:

$$C(x, t) = 0 \text{ for all } t \geq \frac{L}{v}, x = 0$$

$$C(x, t) = C_o \text{ for all } t \geq 0, t \leq \frac{L}{v} \text{ at } x = 0$$

$$C(x, t) = 0 \text{ for all } x > 0, t = 0$$

The boundary conditions at the source are:

$$C(x, t) = 0 \text{ for } t \leq 0; x \notin [-L, 0]$$

The solution for this case is:

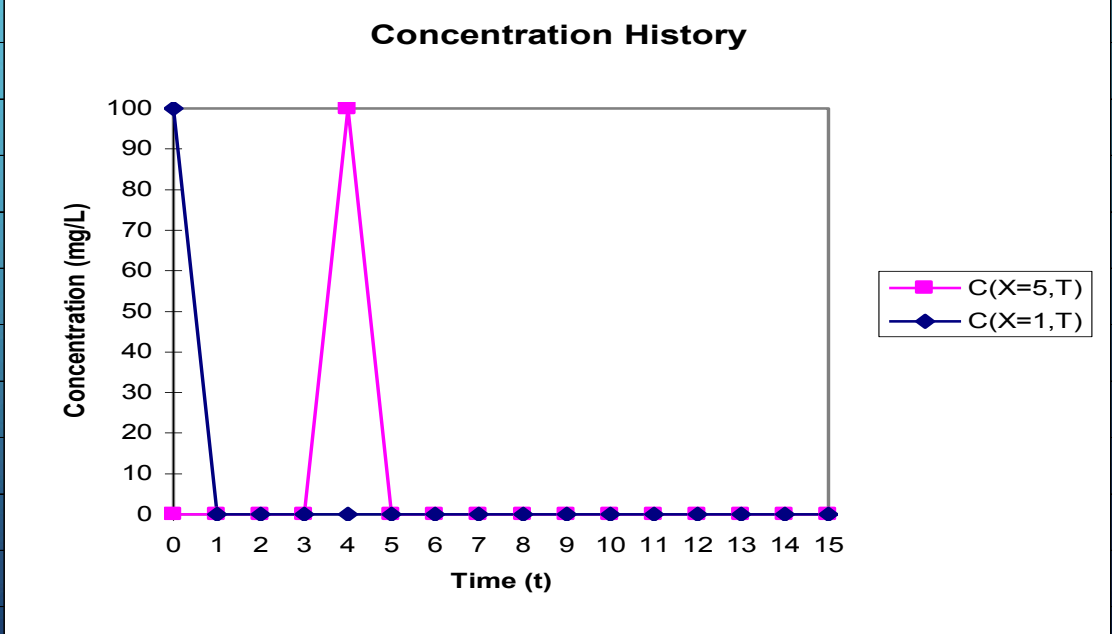
$$C(x, t) = 0 \text{ when } x \leq vt - L$$

$$C(x, t) = C_o \text{ when } vt - L \leq x \leq vt$$

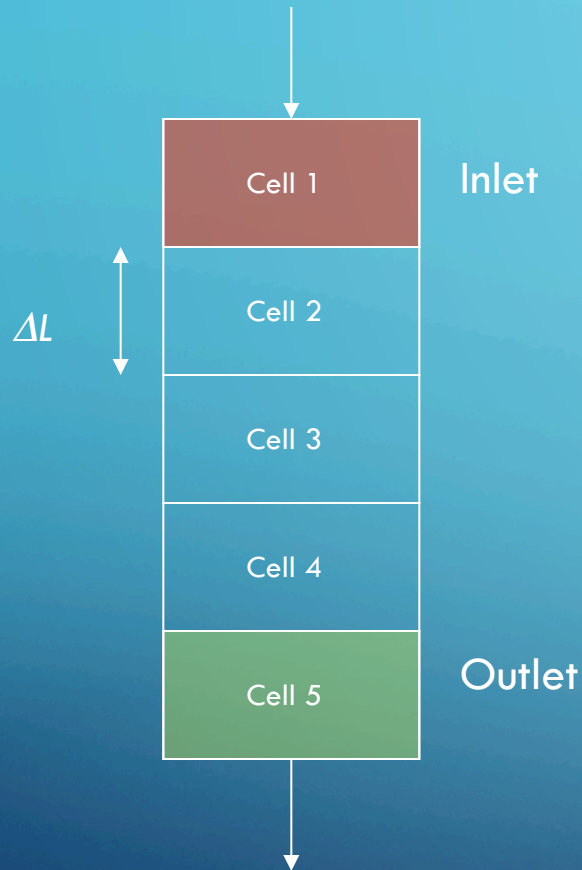
$$C(x, t) = 0 \text{ when } x > vt$$

	A	B	C	D	E	F	G	H	I
1	Co	100	=B\$2*A6		=C6-\$B\$3				
2	V	1							
3	L	1							
4									
5	T	X	VT	VT-L	C(X=1,T)	X	VT	VT-L	C(X=5,T)
6	0	0	0	-1	100	4	0	-1	0
7	1	0	1	0	0	4	1	0	0
8	2	0	2	1	0	4	2	1	0
9	3	0	3	2	0	4	3	2	0
10	4	0	4	3	0	4	4	3	100
11	5								0
12	6								0
13	7								0
14	8								0
15	9								0
16	10								0
17	11								0
18	12								0
19	13								0
20	14								0
21	15								0
22	16								0

=IF(OR(B6<=D6,B6>C6),0,\$B\$)



# CELL BALANCE MODEL APPROACH

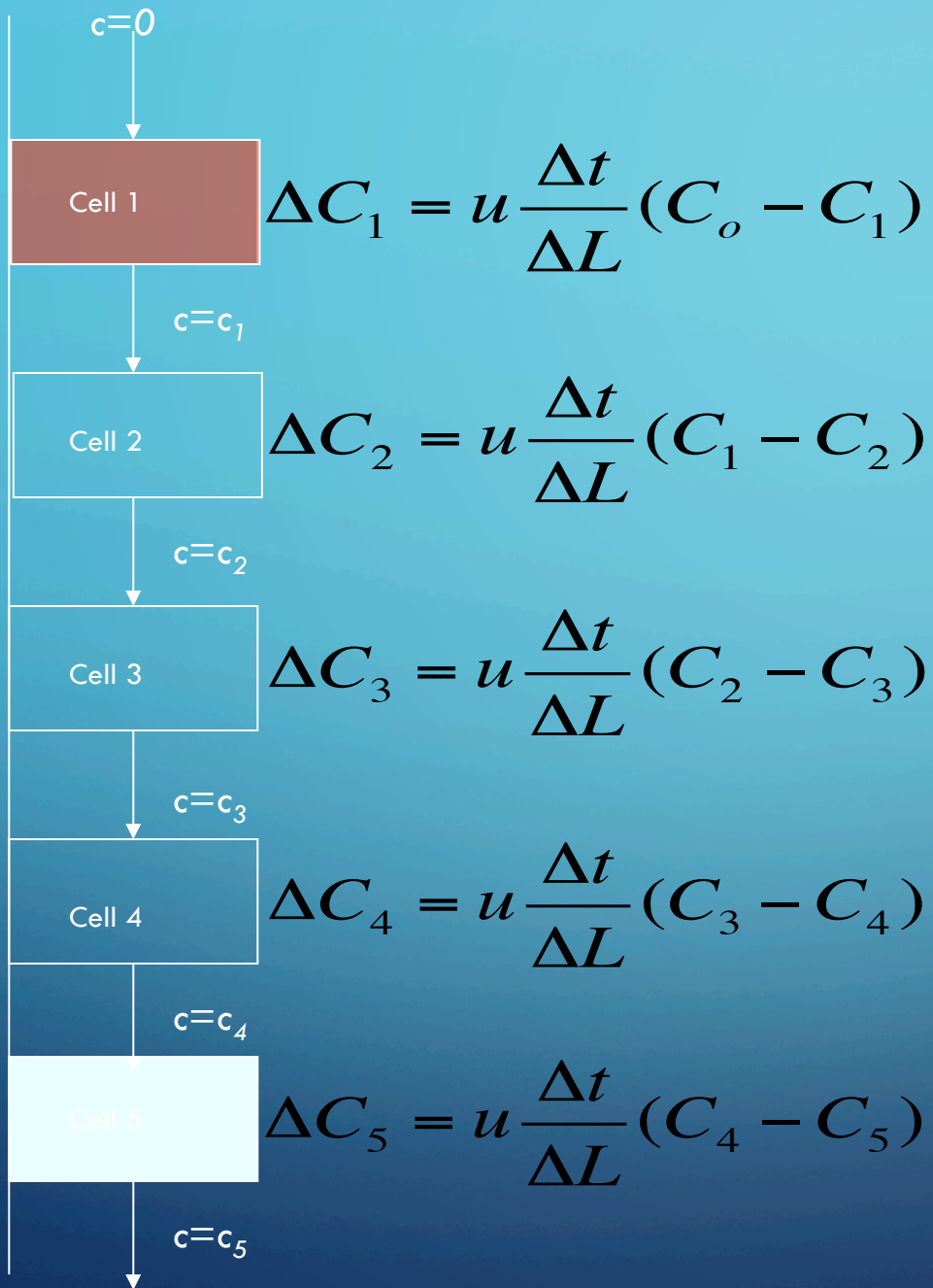


Suppose at  $t=0$  the concentration in the inlet cell is  $C_0$ . We want to determine the concentration in the pipe segment at future times.

We will assume the velocity is identical throughout the column.

A simple modeling approach is to treat each cell as completely mixed.

This means that the concentration at the cell exit is identical to the concentration in the cell.

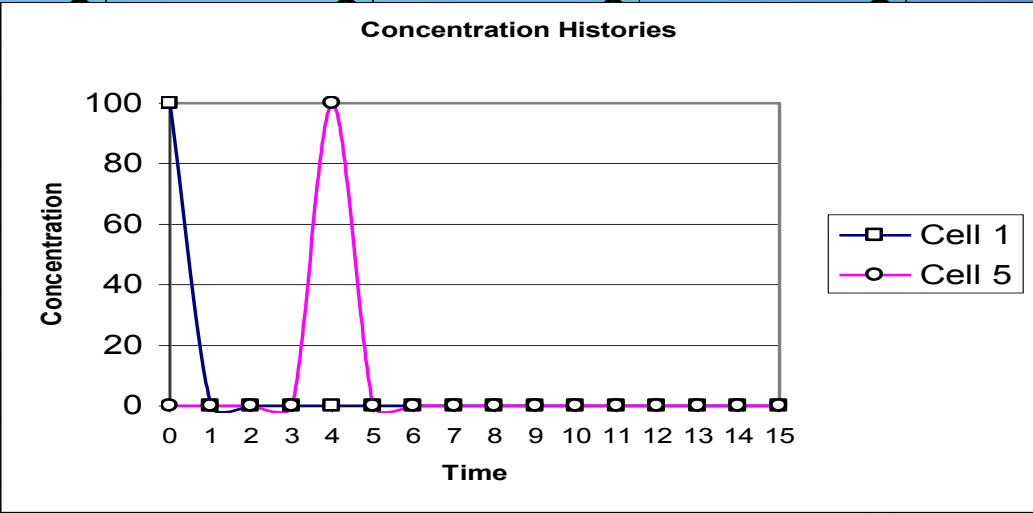
$\Delta L$ 

$$\frac{C_{in}u_{in} - C_{out}u_{out}}{\Delta L} = \frac{\Delta C}{\Delta t}$$

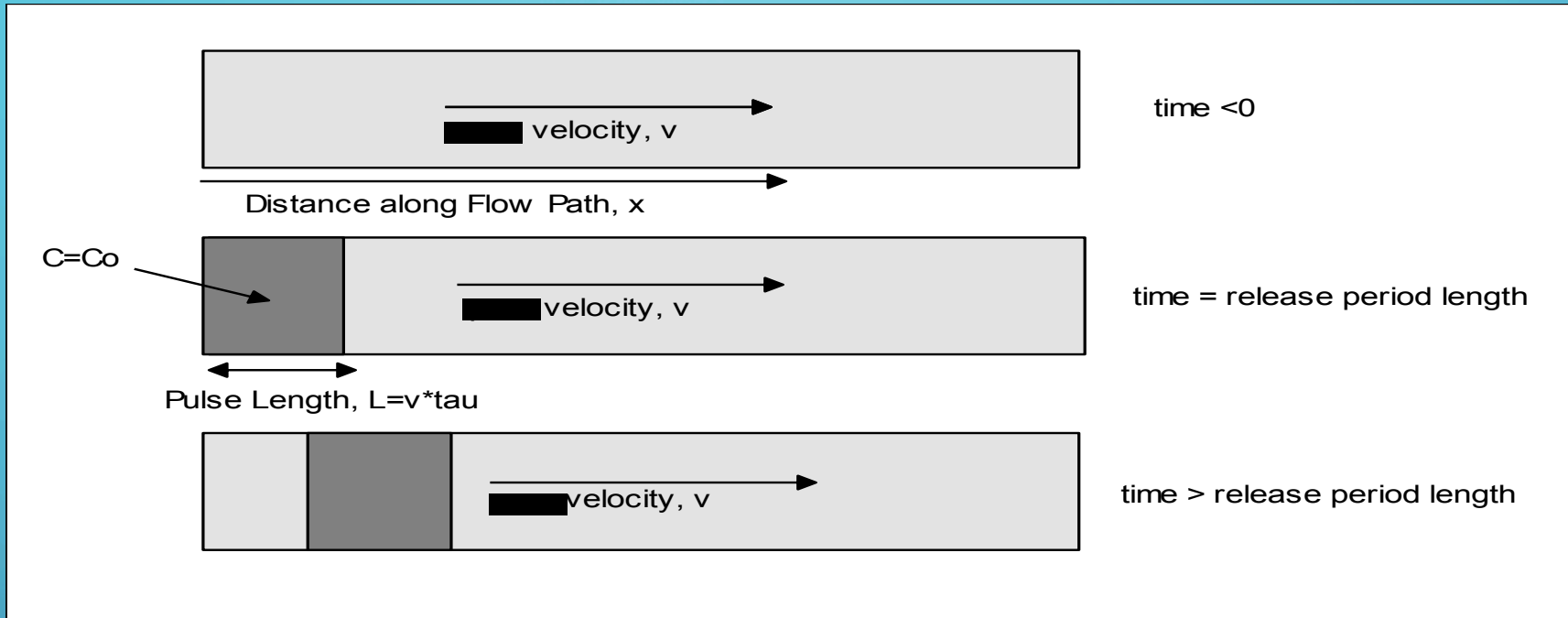


	A	B	C	D	E	F
1	$u$	1	$Co$	100		
2	$\Delta L$	1				
3	$\Delta t$	1				
4	$t$	Cell 1	Cell 2	Cell 3	Cell 4	Cell 5
5	0	100	0	0	0	0
6	1	0	100	0	0	0
7	2	0	0	100	0	0
8	3	0	0	0	100	0
9	4	0	0	0	0	100
10	5	0	0	0	0	0
11	6	0	0	0	0	0
12	7	0	0	0	0	0
13	8	0	0	0	0	0
14	9	0	0	0	0	0
15	10	0	0	0	0	0
16	11	0	0	0	0	0
17	12	0	0	0	0	0
18	13	0	0	0	0	0
19	14	0	0	0	0	0
20	15	0	0	0	0	0

$$=C5+(\$B\$1*\$B\$3/\$B\$2)*(B5-C5)$$



# TIMED RELEASE CASE



- At the origin ( $x=0$ ) a contaminant is added to the flowing water at fixed concentration  $C_0$  for a period of time  $t$ .
- At the end of the time period the contaminant addition is stopped.
- By the end of the time period a “parcel” of contaminated water is created.
- Mechanism of release does not disturb the local flow field in any fashion.
- Contaminant is assumed to be uniformly mixed in the parcel (zone)

# SOLUTION

$$C(x, t) = 0 \text{ when } x \leq vt - v\tau$$

$$C(x, t) = C_o \text{ when } vt - v\tau \leq x \leq vt$$

$$C(x, t) = 0 \text{ when } x > vt$$

- Solution identical to first case.
- Substitute  $v\tau = L$  into the previous solution.

# IN EPANET HOW THESE ARE IMPLEMENTED

## Basic Transport

EPANET's water quality simulator uses a Lagrangian time-based approach to track the fate of discrete parcels of water as they move along pipes and mix together at junctions between fixed-length time steps. These water quality time steps are typically much shorter than the hydraulic time step (e.g., minutes rather than hours) to accommodate the short times of travel that can occur within pipes.

# MIXING AT NODE

- When parcels (concentration) reaches a node where there are multiple mass fluxes, a flow-weighted mixing model is used to compute the concentration at that node (which will become a new  $C_0$  for any downstream links)

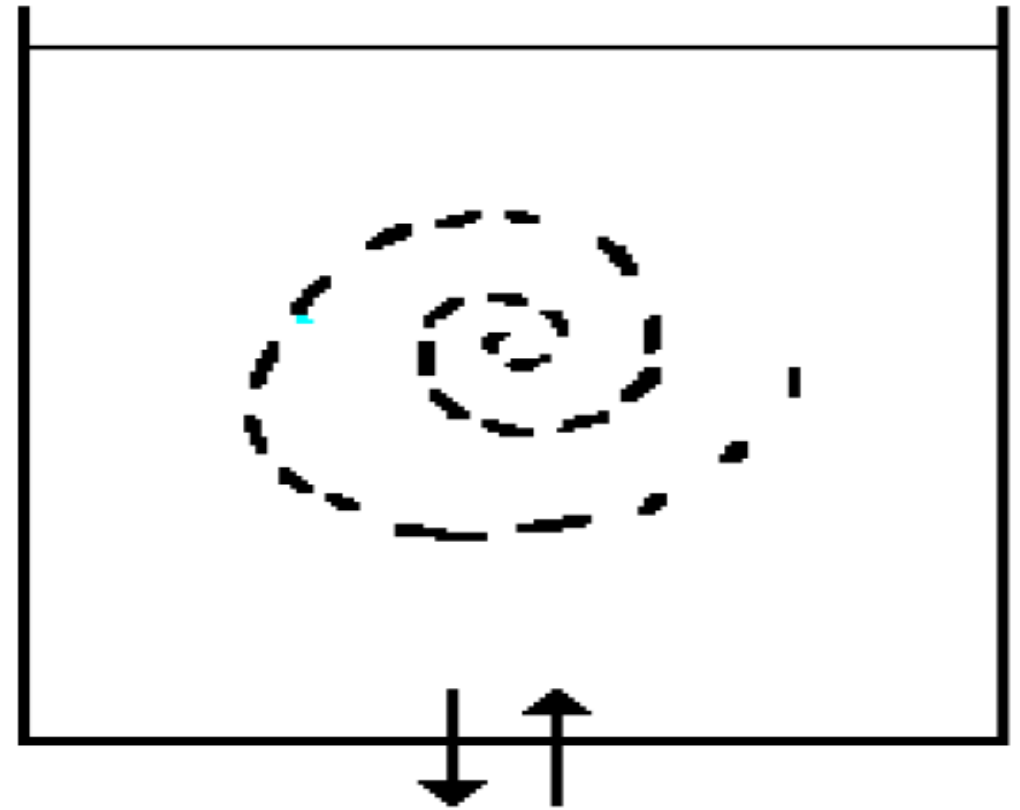
$$C_{out} = \frac{\sum C_{in} Q_{in}}{\sum Q_{in}}$$

# MIXING IN A TANK

- Tank mixing is handled by four possible models:
  - Completely mixed (CFSTR)
  - Two-Compartment Mixing
  - FIFO Plug Flow
  - LIFO Plug Flow

# COMPLETELY MIXED

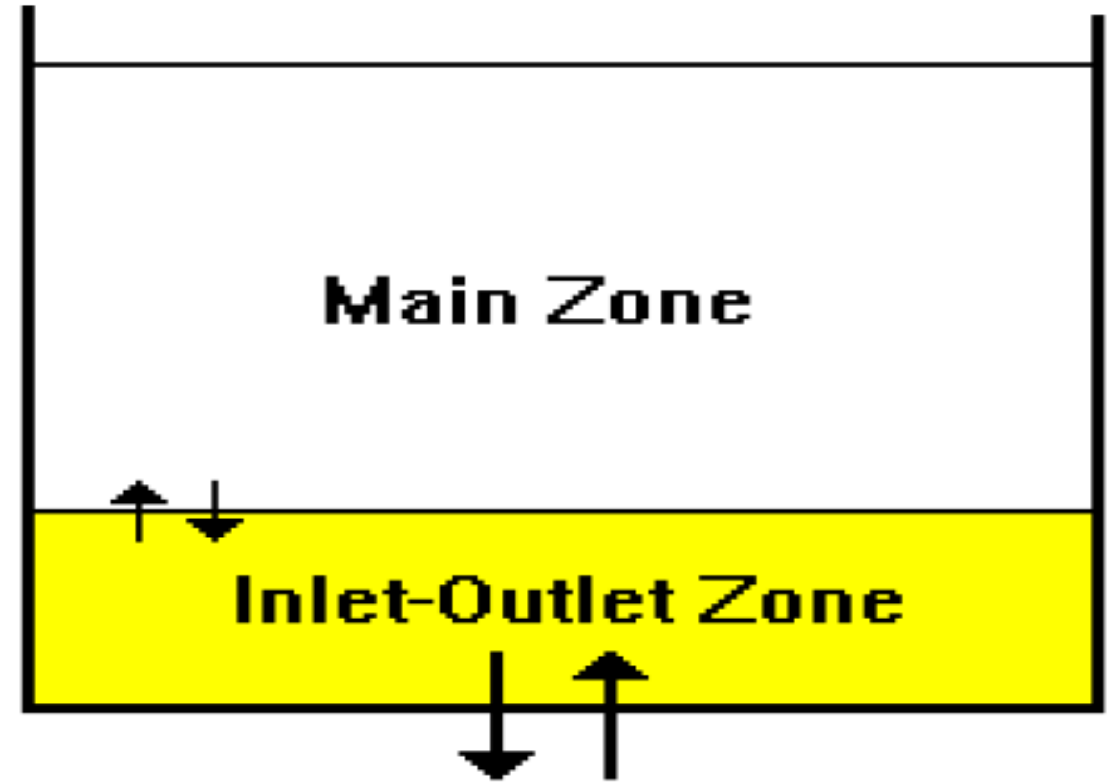
- All water entering tank instantly and completely mixes
  - Reasonable for small tanks, or hydraulic time steps that are long compared to transport time steps



(A) Complete Mixing

# TWO-COMPARTMENT MIXING

- Tank storage divided into two compartments
  - Inlet/Outlet zone
  - Main Zone
- When Inlet/Outlet zone is filled, then spills into main zone

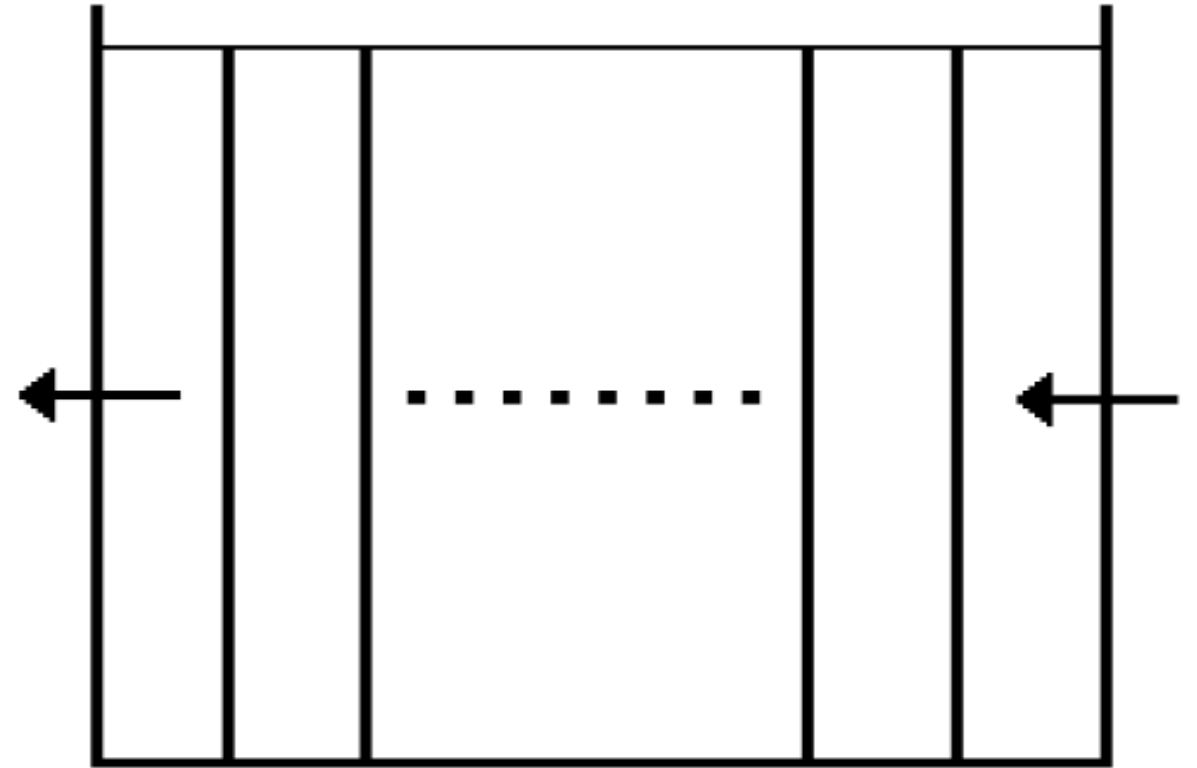


(B) Two-Compartment Mixing



# FIFO MIXING

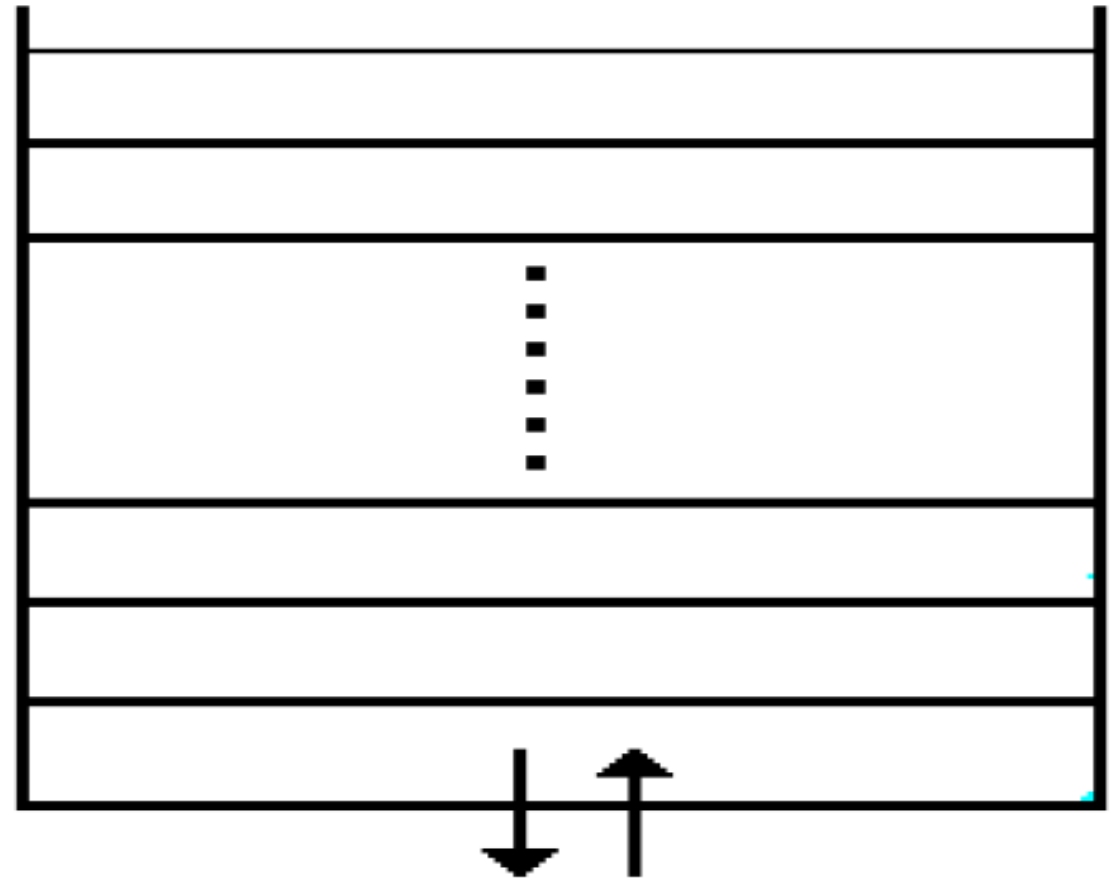
- The first parcel (volume) of water to enter the tank, is first parcel to leave
- Essentially plug-flow in the tank



(C) Plug Flow - FIFO

# LIFO MIXING

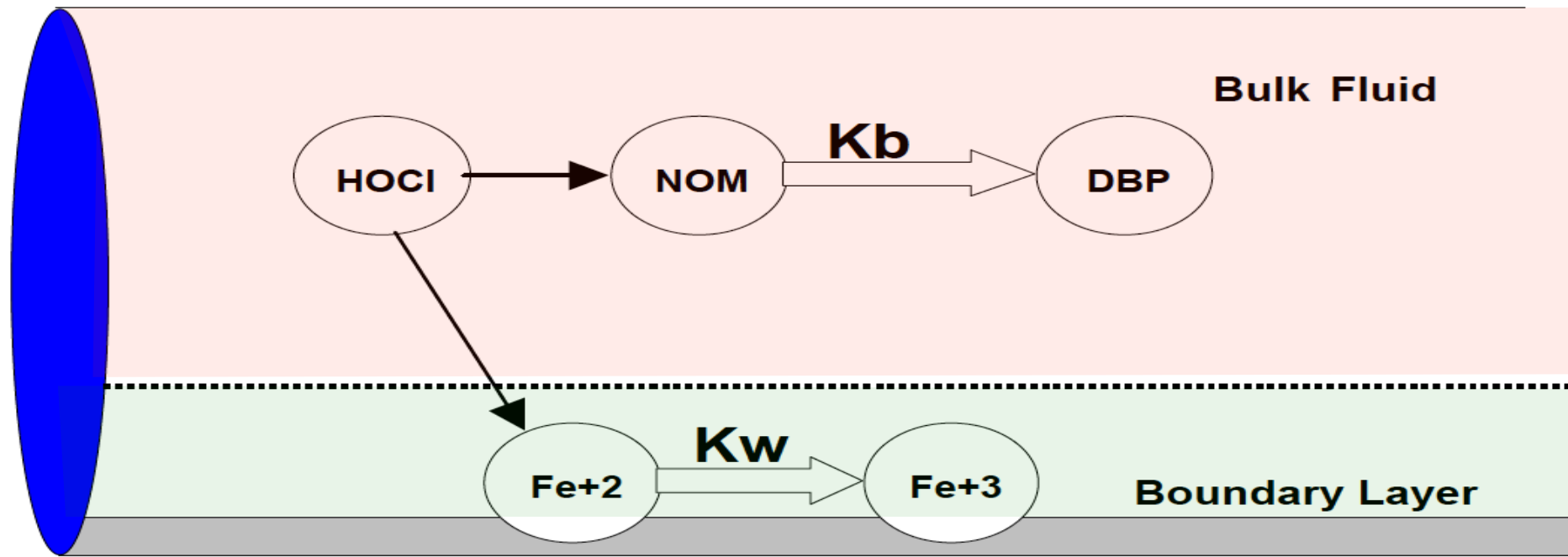
- The last (most recent) parcel (volume) of water to enter the tank, is first parcel to leave
- Essentially stratified-flow in the tank



(D) Plug Flow - LIFO

# WATER QUALITY REACTIONS

- **Bulk** reactions (in the parcel)
- **Wall** reactions (at the parcel, pipe-wall interface)



**Figure 3.6** Reaction Zones Within a Pipe

# BULK REACTIONS

- Growth and decay of constituent in the bulk phase (parcel)
- Uses choice of
  - No reaction
  - Zero-Order kinetics
  - 1-st Order Decay
  - 1-st Order Saturation

$$\frac{\partial C}{\partial t} = -div(\vec{U}C) + r(C)$$

$$r(C) = \begin{cases} K_b C^n \\ K_b (C_L - C) \\ K_b C (C - C_L) \\ \frac{K_b C}{C_L - C} \end{cases}$$

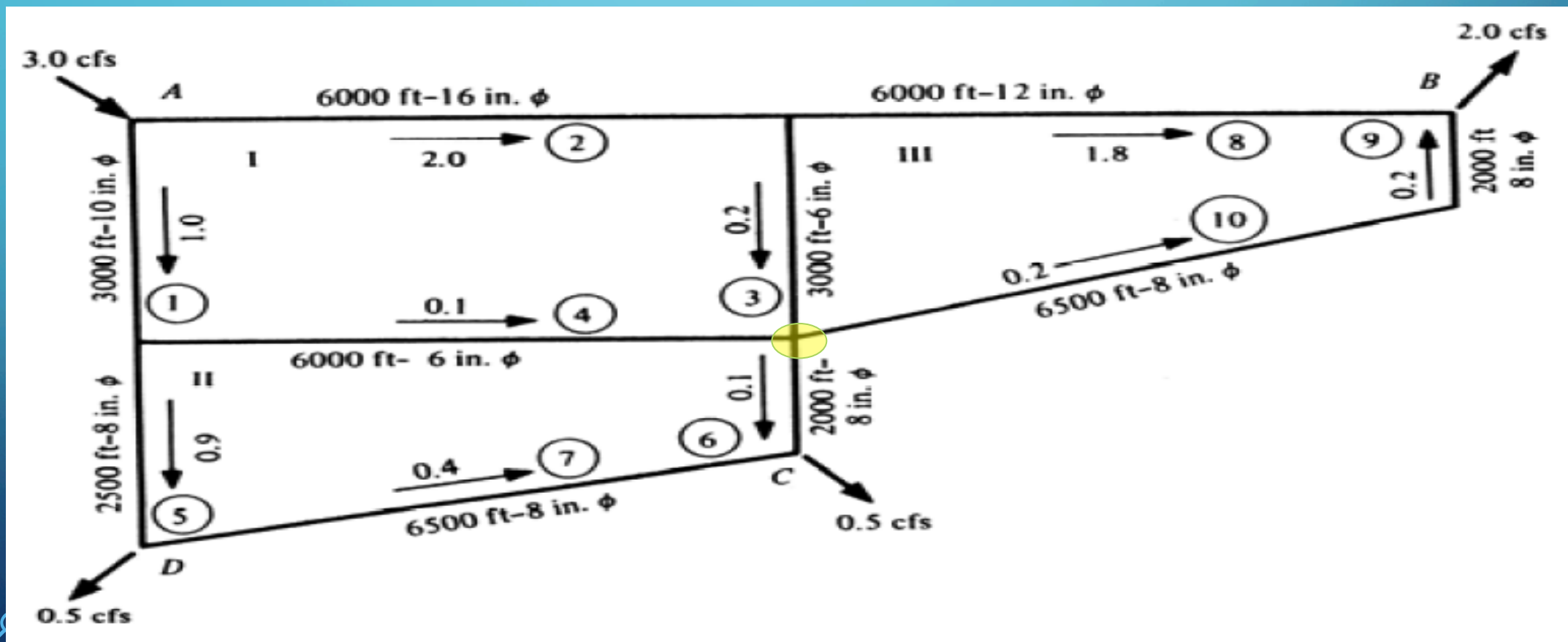
<i>Model</i>	<i>Parameters</i>	<i>Examples</i>
First-Order Decay	$C_L = 0, K_b < 0, n = 1$	Chlorine
First-Order Saturation Growth	$C_L > 0, K_b > 0, n = 1$	Trihalomethanes
Zero-Order Kinetics	$C_L = 0, K_b < 0, n = 0$	Water Age
No Reaction	$C_L = 0, K_b = 0$	Fluoride Tracer

# WALL REACTIONS

- Handled in similar fashion – generally a secondary reaction term based on location in the network

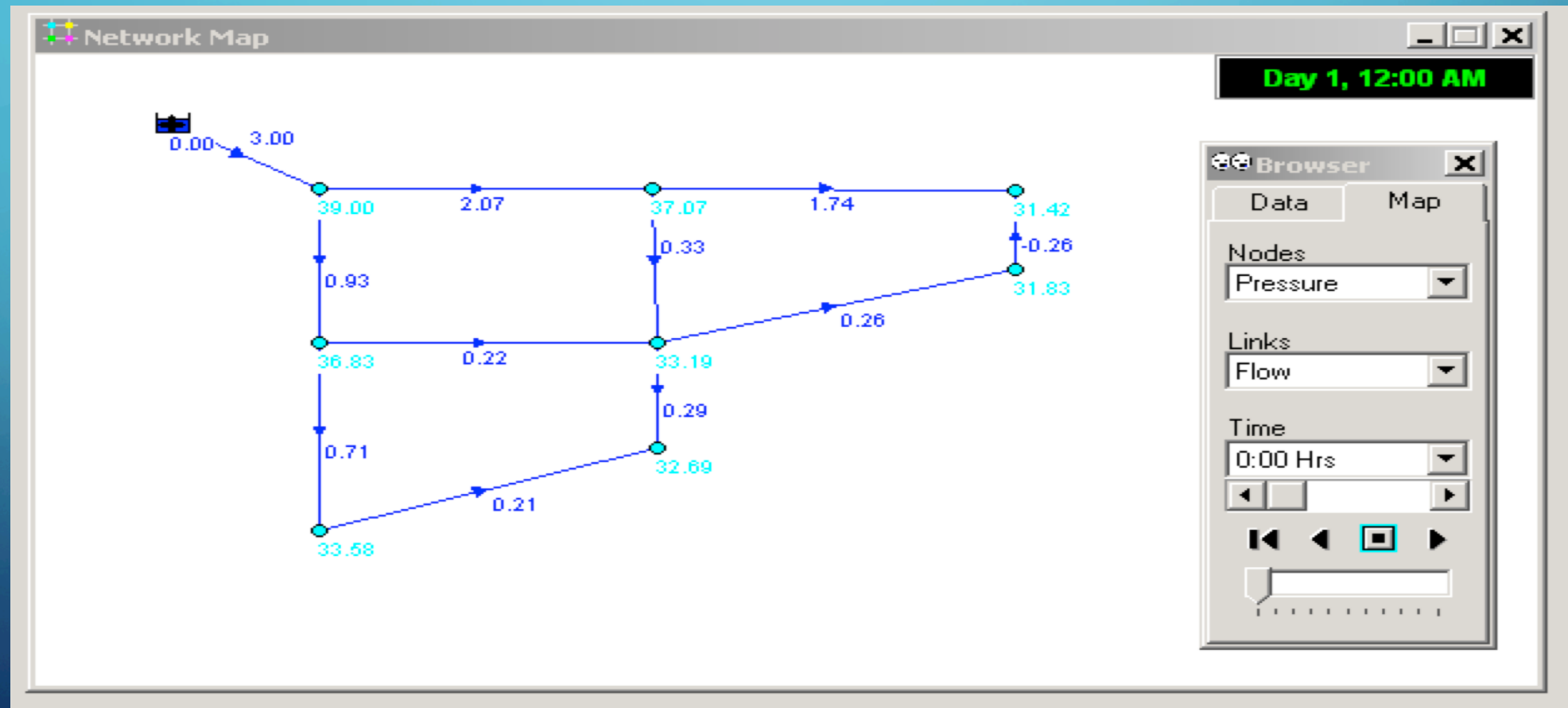
# EXAMPLE

- What is the disinfection residual in the system below if the source water has chloramine at 10 mg/L and the first-order decay mass transfer coefficient ( $K_b$ ) is  $-5$ ?



# EXAMPLE

- What is the disinfection residual in the system below if the source water has chloramine at 10 mg/L and the first-order decay mass transfer coefficient ( $K_b$ ) is -5



# EXAMPLE

The screenshot displays a software interface for a network simulation. The main window is titled "Network Map" and shows a network of nodes and edges. The nodes are represented by green circles with numerical values: 0.00, 3.00, 39.00, 37.07, 31.42, 36.83, 33.19, 31.83, 33.58, and 32.69. The edges are blue lines with arrows and numerical values: 2.07, 1.74, 0.93, 0.33, 0.22, 0.26, 0.71, 0.29, and 0.21. A red circle highlights the node with value 37.07. In the top right corner, a black box displays "Day 1, 12:00 AM" in green text, also circled in red. To the right, a "Browser" panel shows a list of options: "Hydraulics", "Quality", "Reactions", "Times", and "Energy". The "Reactions" option is highlighted with a blue background. Below the browser panel, a "Reactions Options" dialog box is open, showing a table of properties and values. The "Bulk Reaction Order" is set to "1" and the "Global Bulk Coeff." is set to "-5", both of which are circled in red. A black text box in the bottom right corner provides instructions: "Select Data/Options/Reactions Set Bulk Coefficient; Reaction Order".

Property	Value
Bulk Reaction Order	1
Wall Reaction Order	First
Global Bulk Coeff.	-5
Global Wall Coeff.	0
Limiting Concentration	0
Wall Coeff. Correlation	0

Select  
Data/Options/Reactions  
Set Bulk Coefficient; Reaction Order



# EXAMPLE

The screenshot displays a software interface for a network simulation. At the top left, a window titled "Network Map" shows a network of nodes and pipes. The nodes are represented by green circles with numerical values: 0.00, 39.00, 37.07, 31.42, 36.83, 33.19, 31.83, 33.58, and 32.69. Pipes are represented by blue lines with arrows and numerical values: 3.00, 2.07, 1.74, 0.93, 0.33, 0.26, 0.22, 0.71, 0.29, and 0.21. A traffic light icon is next to the 0.00 node. The top right corner of the "Network Map" window shows "Day 1, 12:00 AM".

To the right of the "Network Map" window is a "Browser" panel with two tabs: "Data" and "Map". The "Data" tab is active, and a list of options is shown: "Hydraulics", "Quality", "Reactions", "Times" (highlighted), and "Energy".

At the bottom of the interface is a "Times Options" dialog box. It contains a table with the following data:

Property	Hrs:Min
Total Duration	24
Hydraulic Time Step	1:00
Quality Time Step	0:05
Pattern Time Step	1:00
Pattern Start Time	0:00
Reporting Time Step	1:00
Report Start Time	0:00

The value "24" in the "Total Duration" row is circled in red.

A black text box at the bottom right contains the following instructions:

Select  
Data/Options/Times  
Set a total simulation duration (how many  
Hydraulic Time Steps to Take

# EXAMPLE

The screenshot displays a hydraulic modeling software interface. On the left, a 'Network Map' window shows a network of pipes and nodes with flow directions and head values. A 'Reservoir 9' window is open, showing a table of properties. On the right, a 'Source Editor for Node 9' dialog is open, with 'Source Quality' set to 10 and 'Source Type' set to 'Concentration'. A black callout box with white text provides instructions: 'Select Data/Reservoirs/...' and 'Set a source quality and source type'.

**Network Map**

**Day 1, 12:00 AM**

**Browser**

Data | Map

Reservoirs

9

**Reservoir 9**

Property	Value
*Reservoir ID	9
X-Coordinate	-2321.43
Y-Coordinate	8898.81
Description	
Tag	
*Total Head	90
Head Pattern	
Initial Quality	
Source Quality	10
Net Inflow	-3.00
Elevation	90.00
Pressure	0.00
Quality	0.00

**Source Editor for Node 9**

Source Quality: 10

Time Pattern:

Source Type:

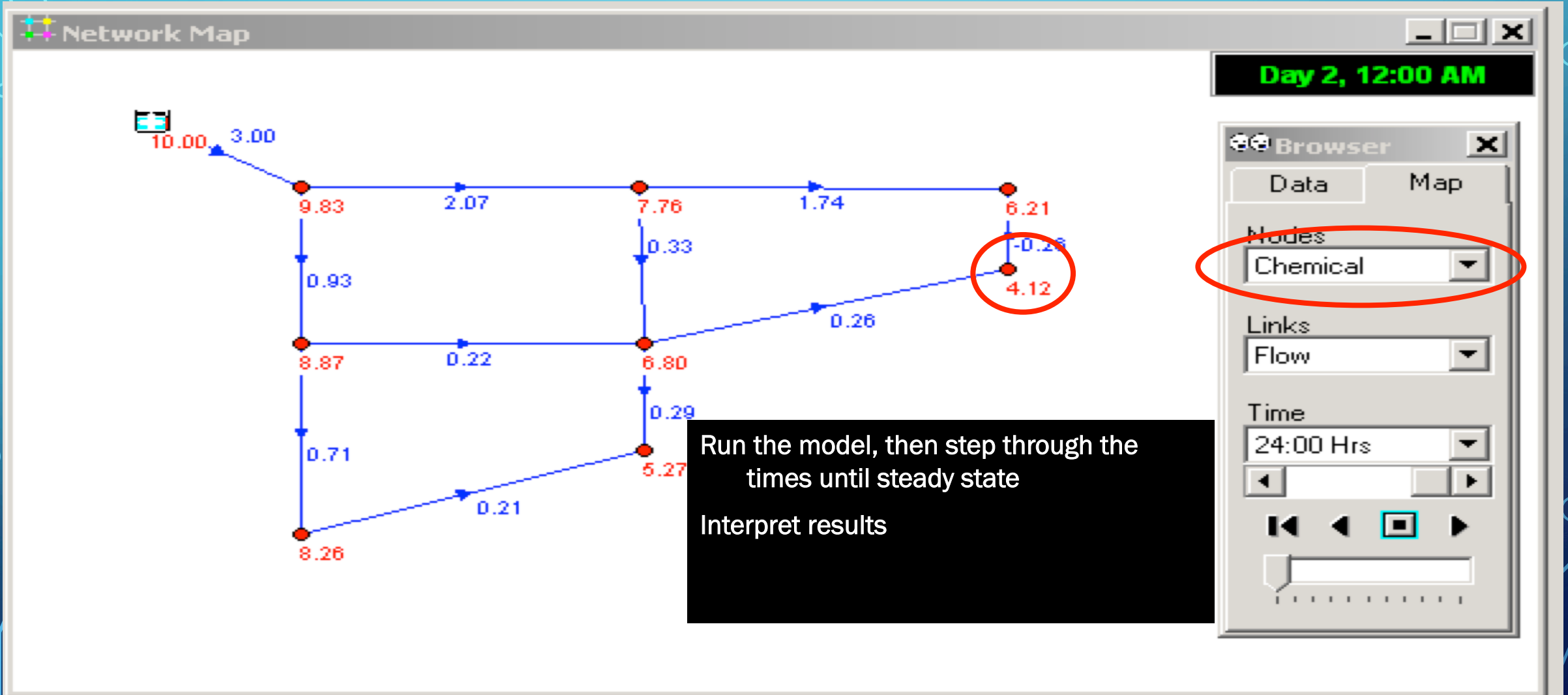
- Concentration
- Mass Booster
- Setpoint Booster
- Flow Paced Booster

Buttons: OK, Cancel, Help

**Select Data/Reservoirs/...**

**Set a source quality and source type**

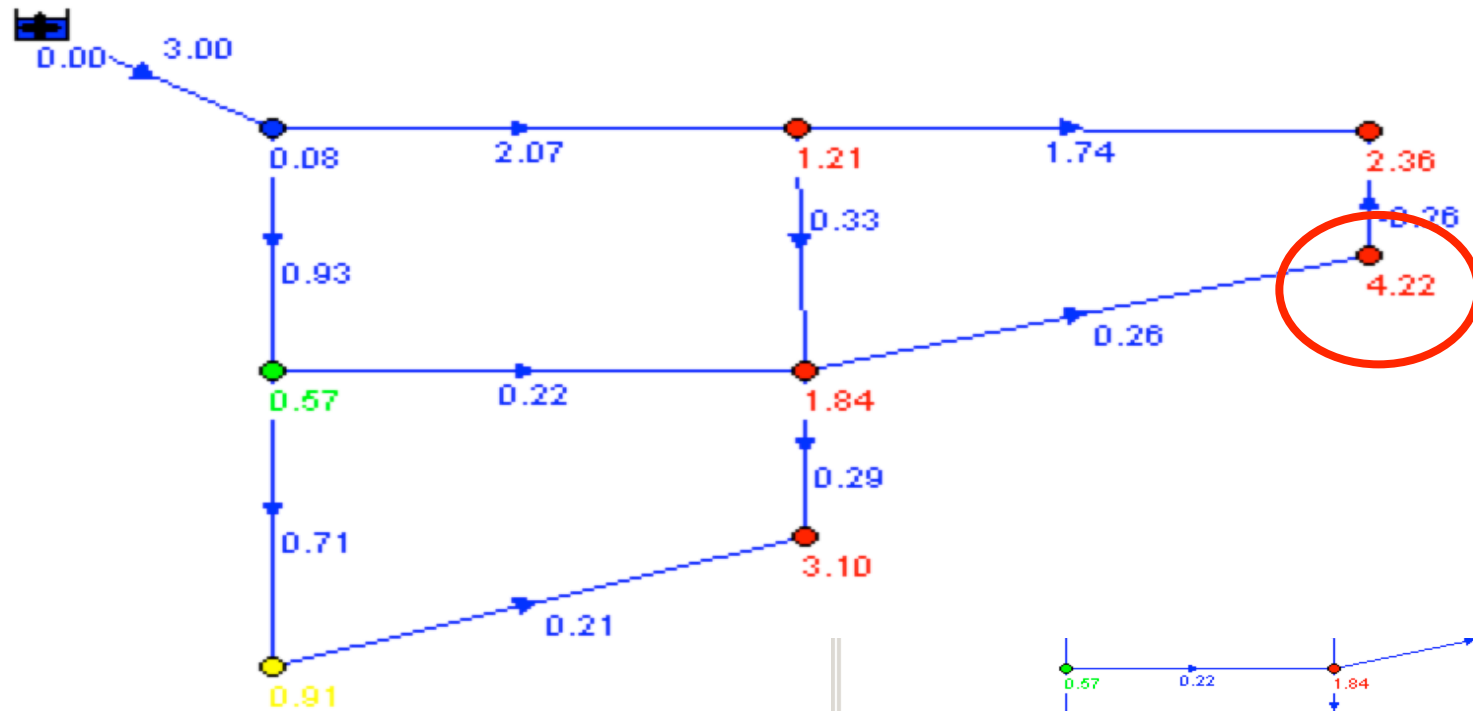
# EXAMPLE



# ADDITIONAL CONCEPTS

- A “tracer” can be used to estimate water age in the system (its treated as a different constituent)
- Use Zero-Order reaction with  $K_b = 1$ ; resulting “concentration” is water age in Hydraulic Time Steps
- Multiple sources can be used to estimate mixing in a system (homework)
- Intrusions of contaminants can be modeled (inject a dose at a node, and see where it arrives).

# EXAMPLE



Day 2, 12:00 AM

Browser

Data Map

Nodes

Age

Links

Flow

Time

24:00 Hrs

Reactions

Times

Energy

Run the model, then step through the times until steady state

Interpret results

Reactions Options

Property	Value
Bulk Reaction Order	0
Wall Reaction Order	First
Global Bulk Coeff.	1
Global Wall Coeff.	0
Limiting Concentration	0
Wall Coeff. Correlation	0

# NEXT TIME

- Open Channel Flow
  - Uniform flow
  - Gradually Varied Flow
  - Hydraulic Elements