



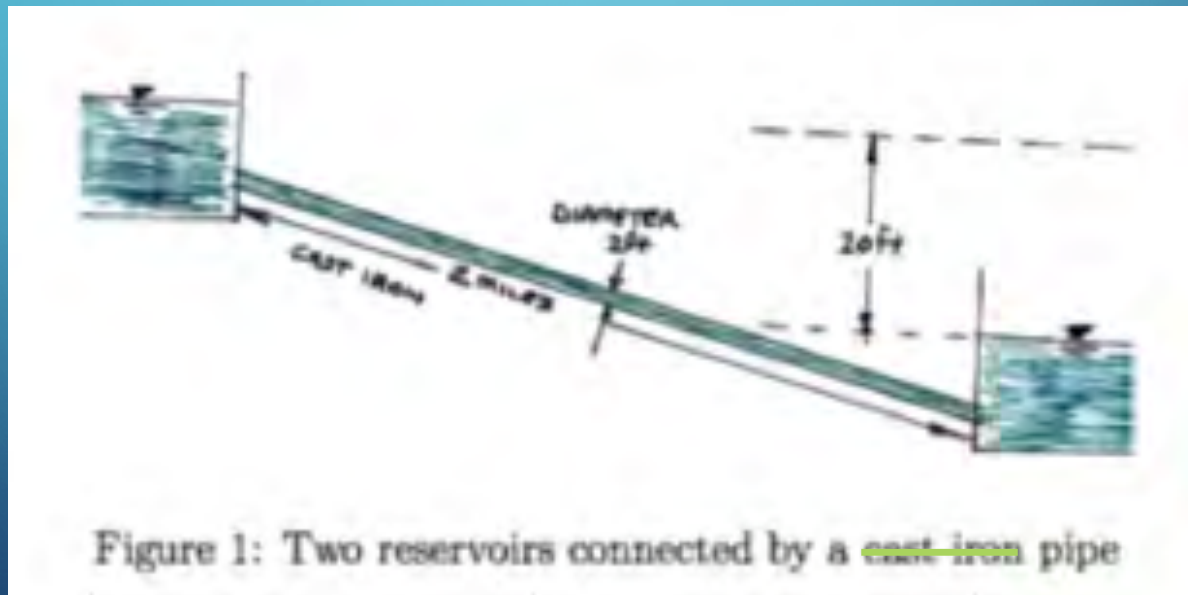
CE 3372 WATER SYSTEMS DESIGN

PIPELINE HYDRAULICS EXAMPLES : PART 1

EXAMPLE 1: USING ON-LINE TOOLS

- Repeat the flow between two reservoirs, but change materials, include entrance and exit losses

$$K_{in} = ?$$



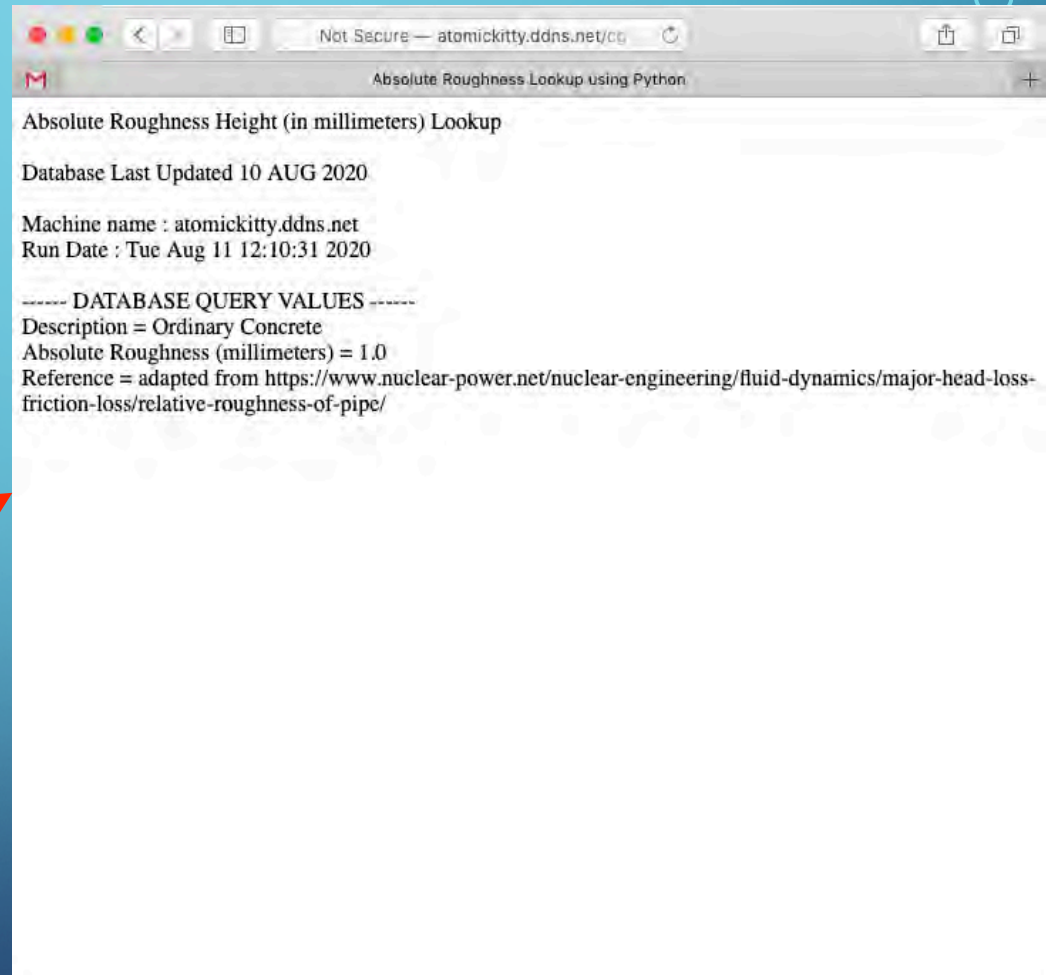
$$K_{exit} = ?$$

Figure 1: Two reservoirs connected by a ~~cast iron pipe~~

concrete

EXAMPLE:

- Determine roughness height for pipeline; $k_s = 1.0\text{mm}$



EXAMPLE:

- Convert millimeters to feet, $k_s = 0.00328$ ft

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Units Converter Tool

Units Converter Tools

Enter value in an input box, result is reported below the box.

Weight (Pounds to ...)

Pounds

Kilograms:

Distance (Feet to ...)

Feet

Meters: 0.001

Centimeters: 0.100

Millimeters: 1.000

Time (Days to ...)

Time

Hours:

Minutes:

EXAMPLE:

- Water Properties



Water Properties (US Customary)
adapted from Table A5 in Elger, Crowe, Roberson 2013. Engineering Fluid Mechanics. Wiley&Sons.

Machine name : theodore-macbookpro.ttu.edu
Run Date : Tue Aug 11 12:19:30 2020

----- INPUT VALUES -----	
Temperature =	50.0 (degrees F)

----- LOOKUP VALUES -----

Density = 1.94 (slugs/ft³)
Specific Weight = 62.4 (lbf/ft³)
Dynamic Viscosity = 2.73e-05 (lbf-s/ft²)
Kinematic Viscosity = 1.41e-05 (ft²/s)
Vapor Pressure = 0.178 (lbf/in²) - absolute

EXAMPLE:

- Minor Loss Coefficients

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atomickitty.ddns.net/documents/university-courses/ce-3372/1-Lessons/Lesson03/

municipal water distribution, stormwater collection, and wastewater collection systems. Oral and written presentations. (Communication Intensive)

Estimation of Water Demand

Lesson Description

Notes

[Lesson Notes](#) Instructor notes used in face-to-face presentation.

[Estimating Water Demand](#) R. S. Gupta 20XX. Chapter 2 in Hydrologic and Hydraulics Systems.

[USGS Circular 1200](#) Solley, W. B., Pierce, R. R., and Perlman, H. A. (1998). Estimated Use of Water in the United States in 1995. U.S. Geological Survey Circular 1200.

[Hydraulics of Pipelines and Pipe Networks](#) Wurbs, R.A., and James, W. P. (2002) Water Resources Engineering, Prentice Hall; pp.130-156; and 156-198.

[TAC-290d](#) Texas Administrative Code. Chapter 290, Subchapter D. Rules and Regulations for Public Water Systems










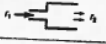

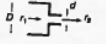

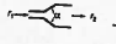
[Water Distribution Systems](#) "Water Distribution Systems" in Land Development Handbook, Ed. S.O. Dewberry, Dewberry Inc., McGraw-Hill

Video Links

Data Links

TABLE 25.13 Table of Local Loss Coefficients

Use the equation $h_v = kv^2/2g$ unless otherwise indicated. Energy loss E_L equals h_v head loss in feet.

Perpendicular square entrance:  $k = 0.50$ if edge is sharp		Check valves:  Swing type $k = 2.5$ when fully open Ball type $k = 70.0$ Lift type $k = 12.0$																				
Perpendicular rounded entrance:  <table border="1"> <tr> <td>$R/d = 0.05$</td> <td>0.1</td> <td>0.2</td> <td>0.3</td> <td>0.4</td> </tr> <tr> <td>$k = 0.25$</td> <td>0.17</td> <td>0.08</td> <td>0.05</td> <td>0.04</td> </tr> </table>		$R/d = 0.05$	0.1	0.2	0.3	0.4	$k = 0.25$	0.17	0.08	0.05	0.04	Angle valve:  $k = 5.0$ if fully open										
$R/d = 0.05$	0.1	0.2	0.3	0.4																		
$k = 0.25$	0.17	0.08	0.05	0.04																		
Additional loss due to skewed entrance:  $k = 0.505 + 0.303 \sin \alpha + 0.226 \sin^2 \alpha$		Segment gate in rectangular conduit:  $k = 0.3 + 1.3 [(V/n)]^2$ where $n = \phi/\phi_0$ = the rate of opening with respect to the central angle																				
Strainer bucket:  $k = 10$ with foot valve $k = 5.5$ without foot valve		Sluice gate in rectangular conduit:  $k = 0.3 + 1.9 [(V/n) - n]^2$ where $n = h/H$.																				
Standard tee, entrance to minor line:  $k = 1.8$		Sudden expansion:  $E_L = (1 - \frac{v_2}{v_1})^2 \frac{v_1^2}{2g}$ or $E_L = (\frac{v_1}{v_2} - 1)^2 \frac{v_2^2}{2g}$																				
Confluent outlet:  <table border="1"> <tr> <td>$d/D = 0.5$</td> <td>0.6</td> <td>0.8</td> <td>0.9</td> </tr> <tr> <td>$k = 5.5$</td> <td>4</td> <td>2.55</td> <td>1.1</td> </tr> </table>		$d/D = 0.5$	0.6	0.8	0.9	$k = 5.5$	4	2.55	1.1	Sudden contraction:  <table border="1"> <tr> <td>$(d/D)^2 = 0.01$</td> <td>0.1</td> <td>0.2</td> <td>0.4</td> <td>0.6</td> <td>0.8</td> </tr> <tr> <td>$k = 0.5$</td> <td>0.5</td> <td>0.42</td> <td>0.33</td> <td>0.25</td> <td>0.15</td> </tr> </table> use v_2 in Equation 13.13	$(d/D)^2 = 0.01$	0.1	0.2	0.4	0.6	0.8	$k = 0.5$	0.5	0.42	0.33	0.25	0.15
$d/D = 0.5$	0.6	0.8	0.9																			
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$(d/D)^2 = 0.01$	0.1	0.2	0.4	0.6	0.8																	
$k = 0.5$	0.5	0.42	0.33	0.25	0.15																	
Exit from pipe into reservoir:  $k = 1.0$		Diffusor:  $E_L = k(v_1^2 - v_2^2)/2g$ <table border="1"> <tr> <td>$\alpha^2 = 20$</td> <td>40</td> <td>60</td> <td>80</td> </tr> <tr> <td>$k = 0.20$</td> <td>0.028</td> <td>0.32</td> <td>0.35</td> </tr> </table>	$\alpha^2 = 20$	40	60	80	$k = 0.20$	0.028	0.32	0.35												
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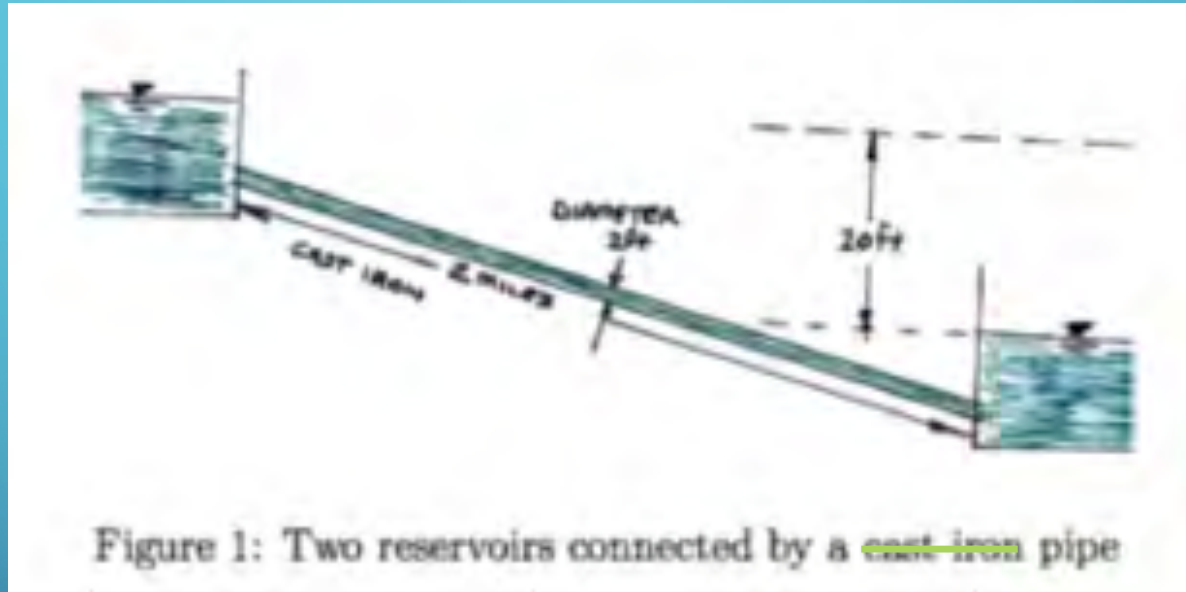
$$K_{in} = 0.5$$

$$K_{exit} = 1.0$$

EXAMPLE:

- Collect the various pieces of information:

$$K_{in} = 0.5$$



$$K_{exit} = 1.0$$

Concrete: $k_s = 0.00328$ ft

Kinematic Viscosity = 1.41×10^{-5} (ft²/s)

- Apply the JupyterLab tool, or online tool.

EXAMPLE:

- Online tool:

Discharge Between Two Reservoirs (US Customary Units)

Pipeline connecting two reservoirs. Pool elevations are Z_1 and Z_2 . Pipeline length is L , diameter is D , sand roughness height is k_s . Pipeline can be analyzed with entrance and exit loss coefficients (K_i and K_e). Pipeline can be analyzed with 2 fitting (K_f) loss coefficients. Calculator solves for flow rate in the pipeline.

Uses Jain equation to make initial flow estimate, then Newton's method to refine the estimate.

[Detailed Explanation](#) (Under Construction)

Pipeline Parameters	Fittings Parameters
Pool Elevation (Z_1): 20	Inlet Loss (K_i): 0.5
Pool Elevation (Z_2): 0	Exit Loss (K_e): 1.0
Pipeline Length (L): 10560	Fitting Loss (K_f): 0
Pipeline Diameter (D): 2	Fitting Loss (K_f): 0
Sand Roughness Height (k_s): 0.00328	
Kinematic Viscosity (ν): 1.41e-5	
Gravitational Acceleration (g): 32.2	

Submit

Use zero fitting values to ignore minor losses

Machine Name : theodore-macbookpro.ttu.edu
Run Date : Tue Aug 11 12:37:16 2020
COMMAND TO RUN : /usr/bin/Rscript 2QReservoir.R

Return Code : 0

INPUT VALUES		
Pool Elevation 1 (Z_1) =	20.0	feet
Pool Elevation 2 (Z_2) =	0.0	feet
Pipeline Length (L) =	10560.0	feet
Pipeline Diameter (D) =	2.0	feet
Sand Roughness Height (k_s) =	0.00328	feet
Kinematic Viscosity (ν) =	1.41e-05	feet ² /second
Gravitational Acceleration (g) =	32.2	feet/second ²
Inlet Loss Coefficient (K_i) =	0.5	
Outlet Loss Coefficient (K_e) =	1.0	
Fitting Loss Coefficient (K_f) =	0.0	
Fitting Loss Coefficient (K_f) =	0.0	

COMPUTED RESULT		
Discharge =	10.21029	ft ³ /sec
Friction Factor =	0.02281027	
EGL Slope =	0.001893939	ft/ft

EXAMPLE:

- JupyterLab Notebook:
 - Nearly same result, Notebook did not account for entrance and exit losses

```
# Get head loss, use a simple error trap
yes=0
while yes == 0:
    xnow = input("Enter head loss \n")
    try:
        head_loss = float(xnow)
        yes = 1
    except:
        print ("Value should be numeric, try again \n")

Enter Pipe Diameter
2
Enter Pipe Length
10560
Enter Pipe Roughness Height
0.00328
Enter liquid viscosity
1.41e-05
Enter gravitational acceleration constant (unit system appropriate)
32.2
Enter head loss
20

# now perform computation and construct output
discharge = jainQ(pipe_diameter,pipe_length,roughness,viscosity,
# Echo inputs, and outputs
print ("Pipe Diameter : ", pipe_diameter)
print ("Pipe Length : ", pipe_length)
print ("Pipe Roughness Height : ", roughness)
print ("Liquid Viscosity : ", viscosity)
print ("Gravitational acceleration constant : ",gravity)
print ("Head loss : ",head_loss)
print ("Discharge : ",discharge)

Pipe Diameter : 2.0
Pipe Length : 10560.0
Pipe Roughness Height : 0.00328
Liquid Viscosity : 1.41e-05
Gravitational acceleration constant : 32.2
Head loss : 20.0
Discharge : 10.303917146034022
```