

**Investigation and Demonstration of Intervention Strategies to Improve  
Water Quality on Country Club Bayou**



**Final Report**

**by**

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**to**

**Houston Wastewater Program – Research Partner**

**Texas Natural Resources Conservation Commission – Research Partner**

**Wastewater Operations – City of Houston – Research Partner**

**Environmental Institute of Houston – Research Partner**

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## Executive Summary

This research tested the potential effectiveness of various strategies for improving water quality on Country Club Bayou. Pollution of the bayou has been problematic for at least a dozen years. Currently suspected high organic loading in the upstream covered portion of the bayou contributes to observed low dissolved oxygen values, septic odor conditions, and septic (black) color in the bayou water. Attempts at eliminating the sources of organic loading to the bayou have not produced an obvious increase in water quality. Despite repair of numerous sewage leaks and reductions in industrial discharges, septic odor and low dissolved oxygen conditions continue to exist.

The investigation included field monitoring of selected water quality parameters, a series of dye tracer studies, and a computer simulation of water quality to evaluate possible intervention strategies.

The field monitoring indicated that when septic odor conditions are prevalent, DO and BOD levels are significantly different than during non-odor conditions. The elevated BOD indicates that some source (commercial wash water, industrial discharge, etc.) supplies an additional organic load to the bayou that in-turn depresses the DO. Odor likely results when this mixture sits relatively stagnant under the Hughes facility.

The field monitoring also indicated that the mean values of DO and sulfate meet existing or proposed state water quality standards for an unclassified stream. The fecal coliform (FC) values do not. Only about 25% of the FC values measured in this research meet the current standard (2000 cfu/100mL). A *change* in water quality occurs between Evergreen Cemetery and Hughes Street. Between these two locations the DO declines, the ammonia increases, and the BOD declines. The BOD decline is diagnostic because it suggests that the bayou has assimilative capacity and that there is either no source between these two locations or there is significant dilution by some unknown source of water. The changes are greater between these two locations than elsewhere in this study, and this section of bayou corresponds with the stagnant section just described.

A computer model of the water quality of Country Club Bayou was developed to predict the effect of selected intervention strategies developed over the course of the research by the research partners. Based on the model's predictions flow augmentation (one of several strategies) provides improvement in water quality at all flows simulated and is reasonably simple to implement.

In addition to flow augmentations, routine monitoring, continued enforcement (source control), and cleaning of portions of the bayou is recommended for long-term management of water quality on Country Club Bayou. Suggestions for funding the implementation are provided.

## 1. Introduction

### *Problem Statement*

Country Club Bayou, formerly Slaughterhouse Ditch, is located in southeast Houston. The bayou drains from east to west connecting to Brays Bayou. The upper portion of the bayou is conveyed in a concrete channel that was initially placed in the early 1900's. The lower portion of the bayou from the Hughes Street railroad bridge to the confluence with Braes Bayou is open, unlined channel.

Pollution of the bayou has been problematic for at least a dozen years. Currently suspected high nutrient loading somewhere in the covered portion of the bayou contributes to observed low dissolved oxygen values, a septic odor, and septic (black) color. The out-fall from the covered portion of the bayou to the open portion is just upstream of the Hughes Street Bridge. Samples collected at the bridge by the City of Houston Health Department confirm these historical observations.

At times the water at the out-fall just upstream of the Hughes Street Bridge has not meet state water quality standards for unclassified waters. Unclassified waters are waters which are not specifically listed in Appendices A or D of §307.10 of Title:30, Part 1, Chapter 307 of the Texas Administrative Code. Table 1.1 lists some of the relevant standards. While symptomatic treatment is technologically feasible, the purpose of this research is to document an investigation protocol to locate sources of pollution and evaluate possible intervention strategies to mitigate the effects of pollution.

The investigation included field monitoring of selected water quality parameters, a series of dye tracer studies, and a computer simulation of water quality to evaluate possible intervention strategies.

Table 1.1 Selected Water Quality Standards for Unclassified Waters

Parameter	Value	Remarks
Dissolved Oxygen	2.0 mg/L - 24 hr. average 1.5 mg/L - absolute minimum 3.0 mg/L - proposed <sup>1</sup>	
Sulfate	65 mg/L - proposed <sup>1</sup>	
pH	6.5-9.0 - proposed <sup>1</sup>	
Fecal Coliform	200 cfu/100mL 2000 cfu/100mL	Contact recreation Non-contact recreation
Temperature	4°F above ambient 1.5°F above ambient	Fall, Winter, Spring Summer

<sup>1</sup> These values are proposed for Segment 1014 (Buffalo Bayou above tidal) for contact recreation and limited aquatic life use.

See: (<http://www.tnrcc.state.tx.us/water/quality/standards/revisions.html>)

### *Study Area Description*

Figure 1.1 is a portion of a USGS map of the study area based on field survey data from 1915. The map shows the bayou branching upstream of Evergreen Cemetery, with both branches depicted as open ditch. The upper branch runs west towards downtown, stopping near the present day US 59. The lower branch runs southwest towards the University of Houston, stopping somewhere near where the present day Law Center sits. The map suggests that in 1915 most of the bayou was open ditch.



Figure 1.1 USGS Map of Country Club Bayou Area (Circa 1920's)

Figure 1.2 is a portion of a recent USGS map of the same area (different scale) with the present day storm and sanitary sewer network superimposed on the map. Sometime between 1922 and the late 1930's the bayou west of Evergreen Cemetery was covered over – reportedly as part of a WPA project. In 1948 the open portion from Evergreen Cemetery to Hughes Street (the Hughes Tool Complex) was covered.

The present day system map is based on a City of Houston GIMS system map that was cross-checked with the traditional storm sewer project maps located in the basement of 1801 Main Street. The sanitary system is shown as the violet network on the map while the storm sewer system is shown as the green network.



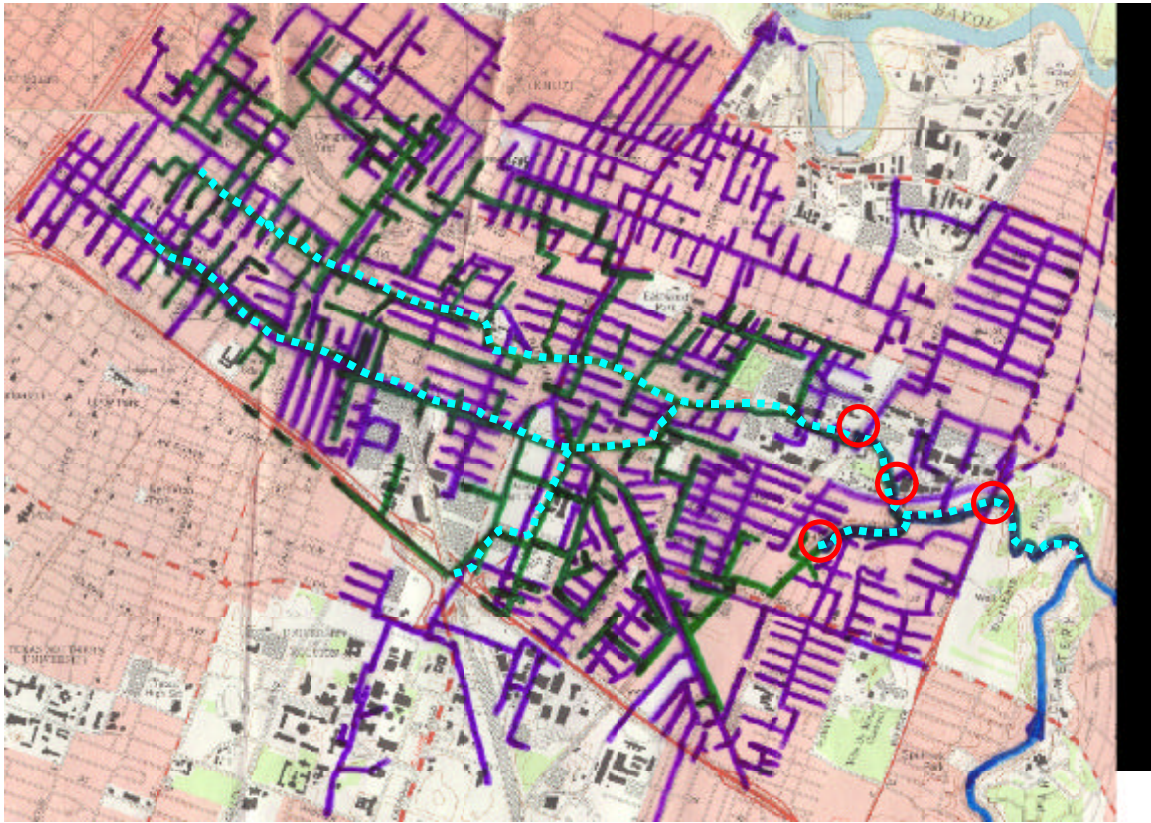


Figure 1.2. USGS Map of Country Club Bayou (Circa 1980's)  
Violet network is sanitary sewer system. Green network is storm sewer system. Red circles indicate approximate locations of photographs in the next section.

The current land-use in the area ranges includes residential, light-industrial, and several large manufacturing facilities. The covered portion of the storm sewer system is owned by the City of Houston, while the open portion appears to be privately owned except for the portion through Wortham Park (COH). There are no known discharge permits issued by any authority for discharge into Country Club Bayou.

### *Bayou Photographs*

The field monitoring effort included occasional photographing at different locations along the open portion of the bayou. The photographs were collected to document the sampling locations in the open portion and document the typical appearance of the bayou. The bayou's appearance is remarkably changeable, especially in the August 1999 photographs collected during a dye study where the water clarity changed from excellent to milky grey in less than 30 hours.

The photographs are arranged first by location then by download date.

*Country Club Bayou at Hughes Street*



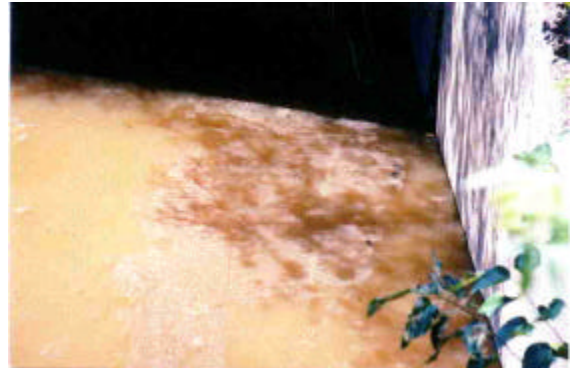
HB&T Railroad Bridge at Hughes Street. Photograph is looking upstream. Sampling location is at upstream side of bridge. March, 1998.



Country Club Bayou Outfall. Upstream of HB&T Railroad Bridge. March, 1998



View from HB&T Railroad Bridge. Observe oily sheen in right side of photograph. March, 1998.



Another view of oil-sheen. March, 1998.



Typical trash appearance during odor episode. Observe black film over much of the material. March, 1998



Typical water appearance after re-aeration in fast moving part of bayou. March, 1998





View looking upstream under HB&T RR Bridge. Trash consists of grocery bags, cans, clothing, paper goods, prophylactics, insulin syringes, fiberglass insulation, shoes, yard waste (leaves). March, 1998



View of sampling location at end of concrete slab in lower left of figure. March, 1998



View under RR Bridge showing trash accumulation in piers of bridge. March, 1998



View just downstream of RR bridge. Velocity measurements are made near this location because water flows in narrow channel in this area. March, 1998



Water in fast-flowing narrow channel beneath RR bridge. Good clarity, black streaks are sediment. May, 1998



Water in fast-flowing narrow channel beneath RR bridge. Good clarity, black streaks are sediment. May, 1998





Spheratolis beneath RR bridge  
May, 1998



Cloudy water at Hughes outfall  
May, 1998



Culvert used to estimate water level for  
field notes  
May, 1998



Clear water at Hughes outfall  
May, 1998



Culvert view  
May, 1998



Cloudy (brown) water  
June, 1998



Cloudy (brown) water  
June, 1998



Milky (grey) water at Hughes outfall  
June, 1998



Milky water at culvert  
June, 1998



Milky water downstream of RR bridge  
June, 1998



Milky water at culvert  
August, 1998



Clear water at culvert  
August, 1998





August 17, 1999 (During dye tracer study)



August 17, 1999 Looking upstream from Hughes Street, water clarity good.



August 17, 1999 (11:00) Bottom visible, good water clarity



August 18, 1999 (16:00) Bottom obscured, poor water clarity, Milky water.



August 17, 1999



August 18, 1999



*Polk and 66th*



Looking upstream from culvert under Polk Street. March 1998



Looking down from culvert  
June, 1998



Looking upstream during milky water event  
June, 1998



Looking upstream after rainfall  
August, 1998



Typical trash (flotables) accumulation  
August, 1998



Looking upstream  
August, 1998



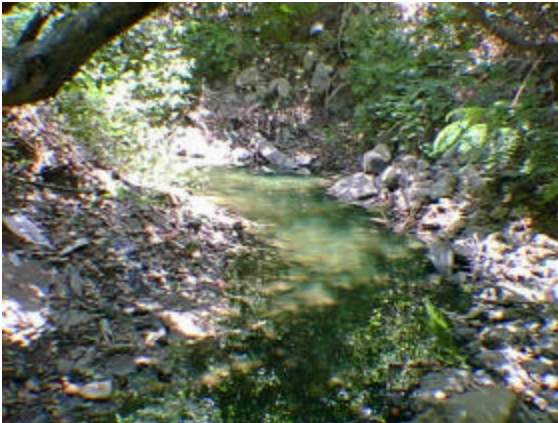
*Yates Gully*



Yates Gully looking upstream behind 901 Hackney  
June, 1998



Yates Gully looking downstream behind 901 Hackney  
June, 1998



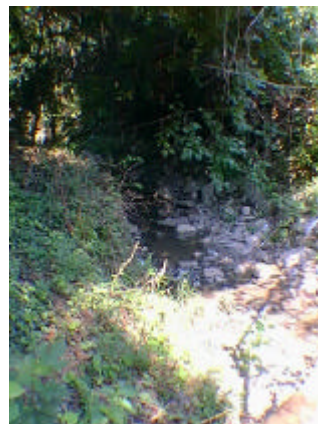
Clear water with slight fluorescent green-yellow color  
June, 1998



Immediately after heavy rainfall  
August, 1998



One week after heavy rainfall  
August, 1998



Downstream of sampling location  
August, 1998

*Wayside Drive*



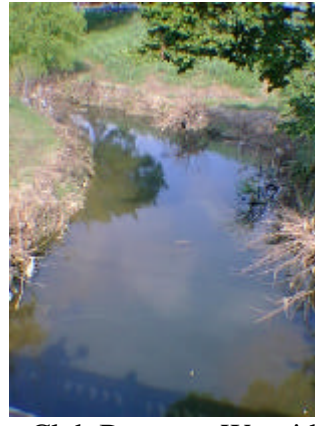
Country Club Bayou at Wayside Drive  
Samples are collected upstream of  
photograph. Flow is from left to right  
August, 1998



Country Club Bayou at Wayside Drive  
Sample collected at this location  
August, 1998



Country Club Bayou at Wayside Drive  
August, 1998



Country Club Bayou at Wayside Drive  
August, 1998



Country Club Bayou at Wayside Drive  
August, 1998



## *Organization*

The remainder of this report is organized into the following sections: Literature Review, Methods, Field Monitoring, Tracer Study, Computer Modeling, Conclusions. The literature review section is a brief review of specific literature used in this research. The methodology section briefly states the purpose and general approach for each of the remaining topics. The field monitoring section describes the details of the field-monitoring program, presents tabular, statistical, and graphical results for the field-monitoring program and interprets these results. The tracer study section describes the details of the tracer tests used to determine time-of-travel for the computer modeling effort. The computer modeling section describes the conceptual hydraulic and water quality models used in this research. The results of calibration and application of the model to evaluate four of five intervention strategies are reported in this section. The conclusion section summarizes the results reported in the previous sections and makes recommendations based on the data collected in this study.

The appendices include the data collected in the various parts of this research, and several reference documents used to support the calculations in this effort.

## 2. Literature Review

The EPA developed and published a user's guide for addressing pollutant inputs into storm water systems (USEPA 1993). This guide identified sanitary wastewater, industrial or commercial pollutant entries, septic tank systems, and vehicle maintenance activities as the most significant potential sources of pollutants into a storm water system. The guide provided a protocol for survey activities to locate and correct non-storm water entries into the storm drainage system. The EPA document focused on systems where direct connection of industrial, and municipal sewers were present and outline corrective techniques that were educational, structural (rehabilitation, disconnection), or administrative (ordinances).

Cleveland et. al. (1993) investigated methods for detection of rainfall induced infiltration into a sanitary sewer system at the Newport Subdivision near Houston, Texas. One of the methods used was chemical analysis of the sanitary wastewater for dilution of key parameters (ammonia). In the subdivision study mapping, chemical analysis, and flow monitoring were used to locate areas in the system where infiltration was most likely. Several of the techniques in this study appeared appropriate for the Country Club Bayou problem.

Most of the activities conducted during the research on Country Club Bayou were based on the EPA guide and the Newport Subdivision Study. Country Club Bayou is unique in that a large portion of the drainage is covered with areas of limited subsurface access so that many of the techniques in the guide can only identify approximate pollutant locations.

One of the proposed intervention strategies is to consider a constructed wetland at Hughes Street to treat the low flows and remove the pollutants that contribute to conditions that create odor episodes. The authors of this report conclude from the literature that at the present time there is not enough knowledge to create an effective constructed wetland on Country Club Bayou. The following materials are presented in support of this conclusion.

The Urban Water Resources Research Council (UWRRC) of ASCE compiled a bibliography on over 800 BMP evaluations. This bibliography was converted into the National Stormwater Best Management Practices (BMP) Database that was funded by cooperative agreement with ASCE and the US EPA. The database was released in the fourth quarter of 1999. The database allows one to search for all BMPs of a particular type and various groups of performance/water quality data. Table 1 lists the results of several searches and the types of water quality data that are currently recorded in the database. The data types were searched as a group, thus the DO/BOD type corresponds to all STORET parameters related to dissolved oxygen or oxygen demand.

Table 1. Wetland Basin BMPs for Stormwater Quality Management  
(source: ASCE National Stormwater BMP Database Version 1.0 June 1999)

Location	BMP Type	Data Type	Number of sites	Number of BMPs
Worldwide	Wetland Basin	All	7	8
Worldwide	Wetland Basin	Solids	7	8
Worldwide	Wetland Basin	Turbidity	3	3
Worldwide	Wetland Basin	Metals	3	4
Worldwide	Wetland Basin	Nitrogen species	3	4
Worldwide	Wetland Basin	DO/BOD	1	1
Worldwide	Wetland Basin	Organics	1	1
Worldwide	Wetland Basin	Temperature	1	1
Worldwide	Wetland Basin	Coliform	0	0
Worldwide	Wetland Basin	Phosphorous	7	8
Worldwide	Wetland Basin	Inorganics	1	1
Texas	Wetland Basin	All	0	0
Texas	Detention Basin	All	2	4
Texas	Biofilter	All	3	3
Texas	Media Filter	All	3	5
Texas	Porous Pavement	All	4	4

Table 1 illustrates that of 800 BMPs categorized only 8 are considered wetland basins. Of the 8 only a couple of sites collected enough data to evaluate water quality enhancement. None of the sites are in Texas. While there is an enormous literature base in constructed wetlands, relatively little is known of their performance when designed to treat stormwater. Table 2. is a listing of the average pollutant removals expressed as percent removal (outflow concentration/inflow concentration). The solids removals are all comparable, on the order of 70%, however the other parameters vary considerably. The limiting nutrient parameters in some cases increase at the outlet, and in one case the total organic carbon, a surrogate for oxygen demanding compounds nearly triples. Admittedly this database excludes many wetlands projects and most stormwater quality management is aimed at solids control because solids are the indicator parameters suggested by the NURP study. Nevertheless, a constructed wetland as a water quality enhancement device needs careful consideration to achieve success.

Komor (1999) reported on a Nutrient and Sediment Control System (NSCS) that can best be described as a four cell system. The four cells are a grass swale at the inlet, a shallow pond, a deep pond, and a shallow swale at the outlet. The purpose of the project was to



reduce N, P, pathogens, and sediments (solids) in agricultural runoff. Runoff for the study was collected from a nearby cow pasture.

Table 2. Removal values for Wetland Basin Stormwater Quality BMPs  
(source: ASCE National Stormwater BMP Database Version 1.0 June 1999)

BMP Name	Parameter Name	Avg Pollutant Removal %
Hidden River Wetland	RESIDUE, TOTAL NONFILTRABLE (MG/L)	68.28
Hidden River Wetland	NITROGEN, TOTAL (MG/L AS N)	-30.44
Hidden River Wetland	NITROGEN, ORGANIC, TOTAL (MG/L AS N)	-53.64
Hidden River Wetland	NITROGEN, AMMONIA, TOTAL (MG/L AS N)	18.52
Hidden River Wetland	NITROGEN, KJELDAHL, TOTAL, (MG/L AS N)	-54.21
Hidden River Wetland	NITRITE PLUS NITRATE, TOTAL 1 DET. (MG/L AS N)	75.51
Hidden River Wetland	PHOSPHORUS, TOTAL (MG/L AS P)	60.69
Hidden River Wetland	PHOSPHORUS, DISSOLVED ORTHOPHOSPHATE (MG/L AS P)	69.92
Hidden River Wetland	CARBON, TOTAL ORGANIC (MG/L AS C)	-191.22
Hidden River Wetland	HARDNESS, TOTAL (MG/L AS CaCO3)	56.55
Hidden River Wetland	CALCIUM (MG/L AS CaCO3)	42.68
Hidden River Wetland	MAGNESIUM, TOTAL (MG/L AS MG)	-123.91
Hidden River Wetland	SODIUM, TOTAL (MG/L AS NA)	-190.34
Hidden River Wetland	POTASSIUM, TOTAL (MG/L AS K)	10.19
Hidden River Wetland	CHLORIDE, TOTAL IN WATER MG/L	-163.03
Hidden River Wetland	SULFATE, TOTAL (MG/L AS SO4)	24.89
Hidden River Wetland	CADMIUM, TOTAL (UG/L AS CD)	26.19
Hidden River Wetland	COPPER, TOTAL (UG/L AS CU)	31.46
Hidden River Wetland	IRON, TOTAL (UG/L AS FE)	-21.98
Hidden River Wetland	LEAD, TOTAL (UG/L AS PB)	24.78
Hidden River Wetland	MANGANESE, TOTAL (UG/L AS MN)	-21.51
Hidden River Wetland	ZINC, TOTAL (UG/L AS ZN)	66.92
Franklin Wetland	RESIDUE, TOTAL NONFILTRABLE (MG/L)	73.8
Franklin Wetland	NITROGEN, TOTAL (MG/L AS N)	3.15
Franklin Wetland	NITROGEN, AMMONIA, TOTAL (MG/L AS N)	2.77
Franklin Wetland	NITROGEN, KJELDAHL, TOTAL, (MG/L AS N)	13.44
Franklin Wetland	PHOSPHORUS, TOTAL (MG/L AS P)	23.41
Franklin Wetland	PHOSPHORUS, DISSOLVED (MG/L AS P)	5.25
Franklin Wetland	PHOSPHORUS, DISSOLVED ORTHOPHOSPHATE (MG/L AS P)	-14.47
Queen Anne's Pond	RESIDUE, TOTAL NONFILTRABLE (MG/L)	72
Queen Anne's Pond	NITROGEN, TOTAL (MG/L AS N)	-46.99
Queen Anne's Pond	NITROGEN, ORGANIC, TOTAL (MG/L AS N)	-43.5
Queen Anne's Pond	NITROGEN, AMMONIA, TOTAL (MG/L AS N)	40.41
Queen Anne's Pond	NITRATE NITROGEN, TOTAL (MG/L AS N)	53.53
Queen Anne's Pond	NITRITE PLUS NITRATE, TOTAL 1 DET. (MG/L AS N)	56.34
Queen Anne's Pond	PHOSPHORUS, TOTAL (MG/L AS P)	34.48
Queen Anne's Pond	PHOSPHORUS, DISSOLVED (MG/L AS P)	53.75
Queen Anne's Pond	PHOSPHORUS, SUSPENDED (MG/L AS P)	12.03
Swift Run Wetland	RESIDUE, TOTAL NONFILTRABLE (MG/L)	75.43
Swift Run Wetland	NITROGEN, KJELDAHL, TOTAL, (MG/L AS N)	-90.52
Swift Run Wetland	PHOSPHORUS, TOTAL (MG/L AS P)	45.37
Swift Run Wetland	IRON, TOTAL (UG/L AS FE)	52.41
Swift Run Wetland	LEAD, TOTAL (UG/L AS PB)	80

The treatment effectiveness was reported as concentration changes between inlet and outlet. The data indicated that the wetland reduced average concentrations of nitrate, suspended ammonia+organics, suspended phosphorous, calcium, potassium, and sulfate, but increased concentrations of ammonia, dissolved phosphorous, and organic nitrogen. There was a slight increase in total solids, but the data appear to display the effects of a large storm (where all the solids washed through the system). These results are consistent with data in the ASCE BMP database suggesting that insufficient knowledge exists for a constructed wetland to confer water quality benefit as well as serve as a storm water BMP.

### 3. Methodology

#### *General Approach*

The general approach to this research was to conduct field monitoring, tracer studies, and computer modeling to test several possible intervention strategies for Country Club Bayou. The field monitoring and tracer study supported the computer modeling as well as documenting the water quality of the bayou. The computer modeling is principally used to test strategies for potential effectiveness in improving water quality during conditions when odor is likely.

#### *Field Monitoring*

The field-monitoring program was conducted to collect water quality data that could be used to calibrate a computer model of the bayou and to develop a database that could be used to interpret the relative health of the bayou, and any quantifiable cause-effect relationships.

Water samples from Country Club Bayou were collected at locations and days agreed upon by the representatives of the City of Houston and University of Houston. The samples were analyzed for selected water quality parameters using Hach<sup>TM</sup> (Hach Corporation, Loveland CO) and standard methods screening level analytical techniques.

The resulting data are stored in an ACCESS database and filtered using descriptive variables of odor, filaments, before-after, and upstream-downstream at Hughes Street. The filtered data sets were analyzed using two-sample t-tests to determine whether or not the mean value of a particular parameter is different during odor episodes. Such a difference would indicate that some perturbation in the water quality creates conditions where odor is produced. If a difference is not detected then the odor episodes cannot be explained using the particular parameters selected.

#### *Tracer Study*

The purpose of the tracer study was to determine system connectivity and to determine travel times for water parcels in different parts of the Country Club Bayou drainage system. The travel times are used to help calibrate the hydraulic model used to test the different intervention strategies. The travel times also identify portions of the bayou with low flow velocities and near-stagnant conditions where odor can be produced.

The tracer used in the tracer studies is Sodium Fluorescein, an organic dye that is commonly used in tracer studies and medical applications. It is considered conservative although it is strongly adsorbed by alumina. These tracers are detected visually for high concentrations using a fluorometer for low concentrations. The tracers are released at an upstream location, monitored at that location for a short time, then monitored at the downstream location to determine time-of-travel.

### *Computer Modeling*

The purpose of computer modeling of the water quality of Country Club Bayou is to provide a tool to predict the effect of selected intervention strategies developed over the course of the research by the research partners. Several “brainstorming” meetings developed six plausible intervention strategies. Table 3.1 is a list of these six strategies with notations on the author’s perceived complexity, cost, and reliability.

Table 3.1 Intervention Strategies for Country Club Bayou

<b>Strategy</b>	<b>Complexity</b>	<b>Cost</b>	<b>Reliability</b>	<b>Modeled</b>
Channel modification	Complex	High	High	Yes
Mechanical/chemical aeration	Complex	High	Unknown	Yes
Constructed wetland	Complex	Unknown	Unknown	No
Flow augmentation	Simple	Low	High	Yes
Divert low flow to treatment	Simple	Moderate	High	Hydraulics
Source control	Variable	Moderate	Moderate	Indirecty

### *Channel Modification*

The channel modification strategy proposes to narrow the box culvert underneath the Hughes facility by construction of a wall along the length of the channel (could be as simple as sandbags) to force the low dry weather flow to a narrower channel. Because the volumetric rate is unchanged, the flow would travel faster through this reach than at present (assuming the tail-water depth is maintained). The premise for this strategy is that during an odor episode the covered portion under the Hughes facility is thought to act like a large septic tank. The organic load in the storm water uses all the dissolved oxygen in the storm water resulting in anaerobic conditions in much of the water column thus creating the odor conditions. By moving the water faster, one could deliver the storm water to the open portion of the bayou where re-aeration is likely to be greater and more able to accommodate the organic load of an odor episode.

### *Mechanical/Chemical Aeration*

The mechanical/chemical aeration option proposes to install some device or devices that deliver more oxygen to the storm water while in the covered portion. Because the slope is relatively small, a passive system (hydraulic draft tube, etc.) is not considered useable so the mechanical/chemical system would involve some active approach. Based on video logs during non-odor episodes the air in the covered portion is breathable, thus mechanical aeration of the water surface combined with small blowers to move the air in the covered portion could work. Such a concept is the basis for the remark that this strategy is complex.



### *Constructed Wetland*

The constructed wetland strategy is a concept that would use the land between the outfall and Hughes Street Bridge (approximately 400 feet by 100 feet) to place a constructed wetland to enhance the water quality of the storm water from the outfall at Hughes. Based on a literature review, this strategy is considered unlikely to produce the desired water quality improvement in the available space. Generally, wetlands appear to function best for BOD and nutrient removal if retention times are large (on the order of weeks).

### *Flow Augmentation*

The flow augmentation option is a strategy where fire-hydrants would be routinely opened and the water allowed to flow into a storm inlet upstream of the wide covered section and eventually mixing with the water in this section of the bayou. This water would principally supply air and the volumes involved are not modeled to be large enough for dilution. The locations of the releases considered in modeling are upstream and downstream of a point source that is used to represent the pollutant load to the bayou.

### *Diversion*

Diversion of all low dry weather flow to a wastewater treatment plant is also considered as a feasible option. This option would involve the installation of a lift station either at Evergreen Cemetery (Altic junction box) or near the outfall at Hughes Street. This lift station would pump dry weather flow to a nearby sanitary sewer for eventual treatment at a treatment plant. The principal modeling question is what effect would such a diversion have downstream of the outfall.

### *Source Control*

Source control by continued field monitoring and DHHS enforcement activity is a remaining option. Source elimination, in principle, should be able to prevent organic loads that exceed the assimilative capacity of the bayou from entering the bayou. Removal of undocumented/illicit connections to the bayou is technically feasible as such connections are identified. Removal could be as simple as plugging the connections in the covered portion (where worker access is possible). However, because the bayou must function as a storm water drain, surface inlets will always need to connect to the bayou. Protecting these inlets from truck wash water, ice melt from food service, and similar inputs will require continued monitoring and DHHS enforcement activities.

#### 4. Field Monitoring and Data Analysis

##### *Purpose*

The field monitoring program was conducted to collect water quality data that could be used to calibrate a computer model of the bayou and to develop a database that could be used to interpret the relative health of the bayou, and any quantifiable cause-effect relationships.

##### *Sampling Locations*

Water samples from Country Club Bayou were collected at locations and days agreed upon by the representatives of the City of Houston and University of Houston. A team approach was used to extend the analytical capabilities of both organizations. Figure 4.1 is a map of the study area showing the approximate locations of field sampling sites.

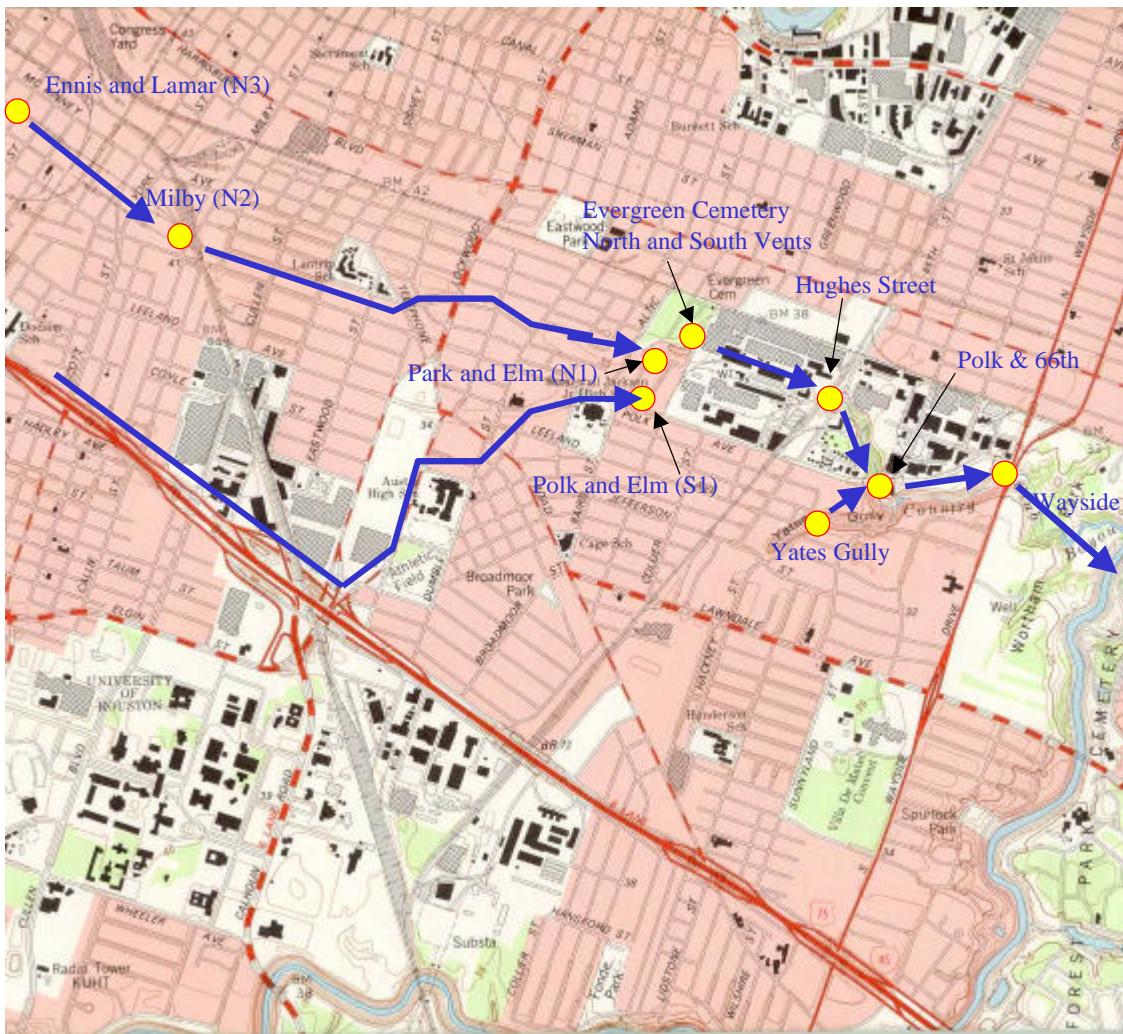


Figure 4.1 Map showing sampling locations

Blue lines are approximate flow alignment of Country Club Bayou

The locations were divided into covered and uncovered locations. The covered locations were at Ennis at Lamar (N3), Polk at Elm (S1), Park at Elm (N1), the North and South vents and Evergreen cemetery. The open locations were Hughes Street under the railroad bridge, 901 Hackney Street (Yates Gully), Polk at 66<sup>th</sup> street, and Wayside Street under the bridge near a Kroger grocery store. The Milby street sampling location (N2) was not sampled by the University of Houston team because traffic redirection is needed. When feasible the UH team did accompany the COH team at this location.

*Parameter Selection*

Figure 4.2 is a screen capture of the data entry form used in this research. On the form are the various hydraulic, water quality, and description parameters that were collected during this research.

ID	Location	Field Visit Date		
111	Hughes Street Bridge	6/11/99		
Remarks				
Weather Conditions	Water Level	Turbid Water	Water Color	Bottom Color
warm, sunny	normal	no	green	grey, green, black
Algae	Filaments	Recent Rainfall	Odor	Water Flowing
yes, green		no	sulfide	slight
Fecal Coliform (cfu/100mL)		BOD (mg/L)		RedoxPotential
pH:		Temp_pH (C)		DO (ppm)
6.889		26.6		1.11
Temp_DO (C)		Sulfide_M8131 (mg/L)		Sulfate_M8051 (mg/L)
27.1		0.021		40
Ferrous_M8146(mg/L)		Ferric (calculated) (mg/L)		Iron-T_M8008 (mg/L)
Iron-T_M8147 (mg/L)		TSS (mg/100ml)		
		0.0004		
Nitrite_M8507 (mg/L)		Nitrite_M8153 (mg/L)		Nitrate_M8171 (mg/L)
				0.4
Ammonia_M8038 (mg/L)		NH3-N COH Lab (mg/L)		TOC (mg/L)
0.58				10.54
Flow Width (feet)		Flow Depth (inches)		Flow Speed (ft/sec)
2.7		4.2		2.51
Discharge (cubic ft/sec)		Discharge (gpm)		Flow Measurement Device
				velocity meter

Figure 4.2 Microsoft ACCESS Data Entry Form



The descriptive parameters were selected based on the early photograph efforts when the changeable nature of the bayou was first apparent. The algae and filaments descriptive parameters were chosen because the COH field team used these observations as indications of possible sewage discharge into the bayou.

The descriptive odor variable was selected because the odor complaints are the driving force behind this entire project. The field sampling team was trained to distinguish between sanitary-type odors (sweet smell), and the “black water” sulfide odor. A hydrogen sulfide safety badge was used to warn the field team to evacuate the outfall if the H<sub>2</sub>S concentration exceeded 25 ppm. The alarm never sounded indicating that even during the field visits with the worst odor, the concentration in air never exceeded this value.

### *Sampling Procedure*

The samples were collected in clean 500-ml jars and placed into a cooler. Two jars per sample site were used. 10-ml of undiluted Nitric acid was added to one of the two jars as a preservative for samples to be tested later for total iron. Water Temperature, dissolved oxygen (DO), oxidation-reduction potential (ORP), and pH were monitored on-site. Velocity, depth, and width of the water stream were also measured at the sites where one could make such measurements. The intended sampling interval was weekly. During poor water quality conditions from the September of 1998 through May 1999, water samples at the Hughes Street were collected twice a week. Water samples were collected at all weather conditions and any unusual site conditions were photographed using a digital camera. During extremely high (storm) flows, water samples were not collected if the samples could not be safely retrieved.

### *Water Sampling and Storage*

At the open sites, the water samples were collected into clean jars directly from the bayou. Such sites included Hughes and Hackney Streets. However, at other open sites, such as Wayside Street and Polk at 66<sup>th</sup> Street such method of sample collection was not used because direct access to the water was impossible (confined entry). In such cases, a clean utility bucket with a synthetic rope tied to its handle was used. The stormwater was retrieved from the center of the water body where the water appeared to be well mixed. The water was then allowed to flow into the bucket for a few seconds and retrieved with as minimal shaking or distraction of water sample as possible. This was done in order to minimize the introduction of air into the sample. The Dissolved Oxygen (DO), temperature, Oxidation-Reduction Potential (ORP) and pH were immediately measured when the bucket was pulled to the surface. DO was always measured first. Any physical properties of the water such as odor, color, turbidity were recorded into the Laboratory Research Notebook. The DO, ORP and pH meters were then rinsed with distilled water for 15 seconds to avoid faulty result values. The samples from each site were then collected into two containers with one of the containers having 10 ml of Nitric acid

present as a preservative. The containers had been previously washed with Alconox® cleaning agent, rinsed with tap water and lastly rinsed with distilled water. A water sample with a preservative was refrigerated and later analyzed for iron using Hach™ iron digestion method.

Samples that were analyzed for non-metallic constituents were stored in 500-ml glass containers that had been washed previously with Alconox®, rinsed with tap water and then rinsed with distilled water. The samples were stored in a portable cooler while on site or in transit, and were refrigerated at the laboratory at 4° C until the samples were ready to be tested. Prior to testing, the samples were allowed to come to room temperature.

The containers were labeled with date and location of the sites prior to collection. The Hach™ pH, DO, and ORP devices were checked and calibrated as necessary before leaving to the collection sites. 10ml of undiluted nitric acid was added to one of the glass container before stormwater was collected. This preservative was used when immediate testing of samples for iron was not feasible.

#### *Laboratory Analysis*

The City of Houston team analyzed samples for BOD and Fecal Coliform (FC) at selected sites because their laboratory has dedicated expertise for these water quality parameters. The University team chose not to conduct FC and BOD because of the strict timing required for success and relatively high cost of expendables. Furthermore these two methods require a fair amount of analyst skill and dedication while the Hach™ methods used for the remaining parameters are relatively simple. Additionally, the city team was able to lift the manhole covers more safely than the university team. Finally, the presence of a city vehicle during sampling reduced the number of confrontations with citizens and business owners. This partnership has been beneficial and four students have been exposed to the limitations and practice of field monitoring.

Refrigerated samples were allowed to warm gently to room temperature prior to analysis. Most samples collected were analyzed for parameters presented in Table 1. Preserved samples were treated with 5 ml of distilled 1:1 hydrochloric acid (HCl) per 100 ml of sample, just before analysis. Near the end of the research parameters were dropped from the analysis as laboratory supplies were exhausted.

The principle technique used for constituent analysis was a colorimetric method using a Hach™ DR/2000 spectrophotometer. For each of the tests, 25 ml of blank sample was compared to 25 ml of sample in which reaction has taken place. Generally, sample preparation consisted of adding and mixing a pre-measured reagent to 25 ml of the sample and allowing time for a reaction to complete and a specific color to develop. The spectrophotometer measured the amount of light of a particular wavelength that passed through the blank sample and considered that measurement as zero, the measurement of a previously reacted sample then was related to that zero measurements. The methods for

analyzing ammonia, sulfide, nitrate MR, involved comparing the reaction in de-ionized water to that in a reacted sample.

The Hach™ methods are generally adaptations of the Standard Methods shown in Table 4.1. Samples whose values were outside the range of the method were diluted using 2:1 or 5:1 volumetric dilutions to reduce the concentrations in the measurement vessels to some value within the instrument range. The concentration in the original sample is obtained by multiplication of the diluted value with the dilution ratio.

The samples were also tested for Total Organic Carbon (TOC) using Shimadzu model TOC 50508 A. Each sample was first filtered using Millipore Millex®-GP gamma sterile 0.22 µm filter unit. If the testing was not performed immediately, the test tube with filtered sample was covered using aluminum foil and refrigerated. Four drops of concentrated hydrochloric acid were added to the filtered sample placed in a 13x100 mm disposable culture tube. The sample was then purged with high purity air for 3-5 minutes. It was then placed in the TOC-5050A analyzer and tested for TOC. The TOC concentration was obtained automatically by the machine by subtracting the Inorganic Carbon from Total Carbon concentration. Prior to testing, a calibration curve was obtained by shifting the intersecting Y-axis of the sample of known value above zero to the origin. A water sample, filtered and purged with purified air was then measured for TOC and the values recorded.

Table 4.1. Analytical Procedures - Method References

Parameter	Range (mg/L)	Hach™ Method # <sup>1</sup>	Standard Methods <sup>2</sup>	USGS Method <sup>3</sup>
Iron, Total	0-3.00	8008	3500-Fe	
Iron, Total	0-1.30	8147	3500-Fe	
Sulfide		3181	4500-S <sup>2-</sup>	
Ferrous ion	0-3.00	8146	3500-Fe	
Ammonia	0-2.5	8038	4500-NH <sub>3</sub>	I-1520-85
Nitrate	0-4.5	8171	4500-NO <sub>3</sub> <sup>-</sup>	
Nitrite	0-150	8153	4500-NO <sub>2</sub> <sup>-</sup>	
Nitrite	0-0.300	8507	4500-NO <sub>2</sub> <sup>-</sup>	
Sulfate	0-70	8051	4500 SO <sub>4</sub> <sup>2-</sup>	I-2823-85
Organic Carbon, Total			5310	
Suspended Solids, Total			2540	
Oxygen, Dissolved	0-10.0	8157	4500-O	
Oxygen Demand, Biochemical <sup>4</sup>		8043	5210	
Coliform, Fecal <sup>4</sup>			9221 or 9222	

<sup>1</sup> Hach Company, 1992

<sup>2</sup> APHA-AWWA-WEF, 1992

<sup>3</sup> Fishman and Freidman, 1989

<sup>4</sup> These parameters were analyzed by the City of Houston



Samples from all of the sites were further tested for Total Suspended Solids (TSS). The sample was shaken to suspend the settled out particles. In this manner, the filtered solids were a very good representation of the field water. For this test, 100 ml of well-mixed sample was measured and filtered using type AP Millipore pre-filters. A vacuum pump was used to filter the solids from the water sample. Prior to testing, each filter was marked and its weight was recorded. After filtering the sample, the filter with residue was placed into the oven at 109°C and allowed to dry for 24 hours. The filter was then allowed to cool completely and its new weight with the residue was measured to the nearest 1/10000 of a gram. Total Suspended Solids weight was calculated by subtracting the weight of the filter from the weight of the filter with residue.

The data were entered into an EXCEL spreadsheet up until September 1999 when the entire database was converted into the ACCESS database program. The ACCESS database is supplied as part of this report.

### *Data Analysis*

The sample data were grouped in 4 different ways to determine if meaningful relationships between the descriptive variables and the measurable parameters were present. At each sample collection site the arithmetic mean value of selected parameters were compared during odor and no-odor conditions; filaments and no-filaments (spheratolis) conditions; before and after conditions. The data were also grouped into a set of upstream and downstream conditions. The comparisons are all made with reference to Hughes Street conditions except where indicated.

If there were a difference in the mean values of f the sample groupes being compared, it is likely that there would also be some difference in the sample variances. A two-sample t-test assuming unequal variances was selected to compare the mean values of the sample groups being tested (Dixon and Massey, 1983).

A t-test on each grouping was performed to test the hypotheses that the mean value of one group was equal to another group (the null hypothesis). A level of significance of 5% was used. This level of significance represents the chance that one will falsely reject the null hypothesis.

In all cases the t-statistic used is

$$t = \frac{X_1 - X_2}{s_p \sqrt{\frac{1}{N_1} + \frac{1}{N_2}}}$$

where  $X_n$  = mean of sample group n,

$N_n$  = number of observations in sample group n,

$$s_p^2 = \frac{(N_1 - 1)s_1^2 + (N_2 - 1)s_2^2}{N_1 + N_2 - 2} = \text{pooled estimate of the population variance,}$$

and  $s_n$  = variance observed in sample  $n$ .

As an example, this data analysis approach can answer the question: Is the DO in the bayou different, on the average, during an odor episode as compared to a non-odor episode?

The purpose of such analysis was to identify specific water quality parameters whose values are either predictive of or responsive to the non-numeric, descriptive conditions. The descriptive conditions were classified in a binary (yes/no) fashion to group the data. Before-after and upstream-downstream analyses were grouped based on dates and locations.

Table 4.2 lists the descriptive variables from the field notes that were analyzed using the statistical test. Table 4.2 also lists the location and time variables analyzed.

Table 4.2 Descriptive Parameters and Perceived Importance

<u>Variable</u>	<u>Analysis Variation</u>	<u>Significance</u>
Odor	Odor vs. No Odor	Complaint generator variable
Water Clarity	Clear vs. Turbid	Indicator of suspended solids
Filaments/Algae	Yes vs. No	Indicator of sanitary discharge
Date	Before vs. After	Determine impact of research
Location	Upstream vs. Downstream	Determine spatial variation

The mechanical procedure for using the descriptive variables is to split the data into two different sets of data at each sampling location using the QUERY feature of the MS ACCESS database management program. One set of data represents samples collected during on particular value of the descriptive condition the other set of data represents the samples collected during the other value of the descriptive condition.

Each of the data groupings are reproduced in Appendix IV. In the appendix the data are ordered by grouping and then location. All the t-tests were performed using the Excel statistical analysis packages included with Microsoft Excel. The results of the individual t-tests are suppressed, but the determination as to whether two mean values are different is indicated by **Bold** typesetting.

*Results: Odor versus No-Odor*

Table 4.3 is a listing of the results for the analysis for odor versus no-odor conditions.

Table 4.3 Mean values for all location during odor and no-odor conditions

		DO	Ammonia	Sulfate	Fecal Coliform	BOD
Ennis and Lamar (N3)	Odor	4.59	<b>2.44</b>	57.14	114,470	
	No Odor	5.33	<b>1.73</b>	48.86	31,883	
Park and Elm (N1)	Odor	5.34	1.05	52.78	107,477	<i>100.00</i>
	No Odor	5.81	0.72	48.87	164,806	<i>12.00</i>
Polk and Elm (S1)	Odor	4.95	0.55	62.59	17,043	<b>39.25</b>
	No Odor	5.34	0.66	55.57	15,462	<b>9.62</b>
Evergreen, North and South Vents	Odor	4.73	<b>0.98</b>	53.57	59,944	<b>64.06</b>
	No Odor	4.73	<b>0.72</b>	46.77	182,356	<b>10.49</b>
Hughes Street	Odor	<b>2.29</b>	0.88	<b>52.68</b>	<b>285,550</b>	<b>23.25</b>
	No Odor	<b>3.64</b>	0.71	<b>39.68</b>	<b>149,721</b>	<b>6.87</b>
Polk and 66th	Odor	<b>3.65</b>	<b>0.93</b>	<b>52.49</b>	20,244	<b>74.10</b>
	No Odor	<b>4.79</b>	<b>0.73</b>	<b>43.38</b>	171,827	<b>8.98</b>
Yates Gully	Odor	4.15	0.88	<b>55.55</b>		
	No Odor	4.77	0.67	<b>45.44</b>		
Wayside Drive	Odor	4.82	0.80	<b>51.37</b>		
	No Odor	4.40	0.79	<b>42.35</b>		

Differences in **Bold** values are statistically significant at p=0.05

Differences in *Italic* are statistically significant at p=0.10

These analysis indicates that when odor conditions are observed at Hughes Street, the DO at Hughes Street is 1 mg/L lower, on average, than when odor is not observed. The DO at Polk and 66th exhibits a similar pattern. Upstream of Hughes, and downstream of Polk and 66th, the DO values are the same, regardless of whether odor is observed at Hughes or not. One interpretation of these results for the downstream portion is that the Wayside drive site receives water from both the main branch and Yates Gully. The dilution effect of Yates Gully water along with natural re-aeration can explain the relative recovery of DO values in the bayou. An interpretation for the upstream portion is that the process that exerts an oxygen demand on the water occurs between Evergreen and Hughes, even though the oxygen demanding compounds can come from upstream. The time-of-travel of water in the bayou between Evergreen and Hughes is about 18 hours of ½mile as compared to 7 hours for nearly twice the distance upstream of this location.

The BOD at all locations where data were collected was always greater during odor episodes than during no-odor episodes. This result supports the concept that one cause of the odor condition is some upstream source of organic load that exerts an oxygen demand



on the water. This increased oxygen demand thus lowers the DO in the stream, and creates conditions where odor is produced.

Only at Hughes are the FC numbers significantly higher during odor conditions. This result suggests that during odor conditions when the organic load is higher that either the source of the load contains elevated FC numbers or the organic load creates conditions where the FC organisms thrive. Intuition suggests both explanations are reasonable.

*Results: Filaments versus No-Filaments*

Table 4.4 is a listing of the results for the analysis for filaments versus no-filaments conditions. Filaments are used as a visual indicator of pollutants that are likely to have come from sanitary sewerage leaks into the bayou.

Table 4.4 Mean values for all location during filaments and no-filaments conditions

		DO	Ammonia	Sulfate	Fecal Coliform	BOD
Ennis and Lamar (N3)	Filaments	<b>4.49</b>	<b>2.59</b>	<b>59.83</b>	59,935	
	No Filaments	<b>7.74</b>	<b>1.36</b>	<b>41.92</b>	189,000	
Park and Elm (N1)	Filaments	<b>4.79</b>	<b>1.17</b>	<i>55.59</i>	230,267	90
	No Filaments	<b>7.43</b>	<b>0.53</b>	<i>39.48</i>	60,800	13
Polk and Elm (S1)	Filaments	5.53	<b>0.66</b>	<b>66.53</b>	11,940	25.74
	No Filaments	5.91	<b>0.49</b>	<b>51.25</b>	36,000	<b>420.75</b>
Evergreen,North and South Vents	Filaments	5.32	1.12	<b>58.61</b>	162,186	38.14
	No Filaments	5.00	0.55	<b>39.21</b>	114,248	87.47
Hughes Street	Filaments	<b>2.47</b>	<b>0.92</b>	<b>55.67</b>	<b>303,089</b>	15.86
	No Filaments	<b>3.94</b>	<b>0.51</b>	<b>30.29</b>	<b>11,162</b>	10.92
Polk and 66th	Filaments	4.03	0.92	<b>57.86</b>		
	No Filaments	4.61	0.84	<b>39.67</b>		
Yates Gully	Filaments	4.98	0.89	<i>53.27</i>		
	No Filaments	6.42	0.57	<i>41.88</i>		
Wayside Drive	Filaments	4.67	0.85	<b>58.20</b>		
	No Filaments	5.18	0.85	<b>45.13</b>		

Differences in **Bold** values are statistically significant at p=0.05

Differences in *Italic* are statistically significant at p=0.10

Bold *Italic* indicates a suspicious value (data series contained one very high value)

These analysis indicates that when filaments are observed at Hughes Street, the DO at Hughes Street is 1 mg/L lower, on average, than when filaments are not observed. The DO at Park and Elm exhibits a similar pattern, but Evergreen, and Polk and Elm do not exhibit such a pattern. One interpretation of these results is that the source of pollutants that cause visible filaments at Hughes enters the system near or upstream of Park and Elm. Although the analysis does not show a significant difference in the mean values at Evergreen, the water at Evergreen is conceptualized to represent a mixture of Polk and

Elm flow and Park and Elm flow (combination of North and South branch flows). The south branch exhibits uninteresting behavior in this particular grouping.

The BOD at all locations was not significantly different in this particular grouping, although the numerical values were near the mean value for odor conditions.

Only at Hughes are the FC numbers significantly higher during filament conditions. This result suggests that during filament conditions the pollutant sources contain elevated FC numbers and a BOD load similar to the loads experienced during odor conditions.

*Results: Before-After*

Table 4.5 is a listing of the results for the analysis for before versus after conditions. The only location with more than two years of data was Hughes. The data were grouped by year (1998 and 1999) except at Hughes where all data prior to 1998 were grouped into a separate group. All the groups in this analysis include data during odor and non-odor conditions. Therefore these results only represent relatively long-term changes (if any).

Table 4.5 Mean values for all locations Before-After analysis

		DO	Ammonia	Sulfate	Fecal Coliform	BOD
Ennis and Lamar (N3)	Mean_98	5.71	1.45	44.50	48,594	
	Mean_99	4.71	2.35	56.90	98,518	149.1
Park and Elm (N1)	Mean_98	6.34	0.44	50.38	<b>21,650</b>	<b>14.70</b>
	Mean_99	5.00	1.28	52.79	<b>145,827</b>	<b>58.29</b>
Polk and Elm (S1)	Mean_98	6.00	0.62	58.23	<b>5,561</b>	9.09
	Mean_99	4.77	0.58	59.74	<b>15,974</b>	85.24
Evergreen, North and South Vents	Mean_98	4.77	0.60	52.90	134,839	<b>14.49</b>
	Mean_99	4.65	0.92	49.40	136,396	<b>43.04</b>
Hughes Street	Mean_97		1.39		<b>329,703</b>	<b>16.84</b>
	Mean_98	3.05	1.45	46.67	<b>236,995</b>	<b>9.81</b>
	Mean_99	2.79	0.76	45.80	<b>83,918</b>	12.38
Polk and 66th	Mean_98	3.88	0.91	47.63		
	Mean_99	3.80	0.92	48.13		
Yates Gully	Mean_98	4.72	0.71	50.25		
	Mean_99	4.58	0.79	44.97		
Wayside Drive	Mean_98	3.66	0.96	55.93		
	Mean_99	4.47	0.78	49.69		

The DO, ammonia, and sulfate at all locations was unchanged during the entire research period. At Hughes, the FC numbers declined from a pre-research mean of 330,000 to 83,000. Upstream of Hughes, the FC numbers were unchanged or increased from 1998 to 1999. These increases upstream of Evergreen (N1 and S1 locations) are not evident in

the data at Evergreen suggesting some mixing process in the stagnant section of the bayou from Evergreen to Hughes.

*Results: Upstream-Downstream*

Data in this analysis were grouped into one upstream group and one downstream group. Park and Elm N1, Polk and Elm S1 and Evergreen Cemetery were taken as a single group. Hughes Street, and Polk and 66th were taken as the downstream group. The choice of the downstream grouping was based on exploratory analysis that indicated that the behavior of the water quality at these two location was similar. The purpose of combined grouping is to increase the size of the data series in each group to produce a large enough data set for meaningful upstream-downstream analysis. The dates are ignored in this analysis so the results reflect upstream versus downstream behavior over the entire research period.

Table 4.6 is a listing of the results for the analysis for upstream-downstream analysis.

Table 4.6 Mean values for Upstream-Downstream analysis

		DO	Ammonia	Sulfate	Fecal Coliform	BOD
All Data	Upstream	<b>5.20</b>	<b>0.70</b>	<b>55.50</b>	<b>68,122</b>	<b>48.16</b>
	Downstream	<b>3.28</b>	<b>1.13</b>	<b>46.89</b>	<b>257,015</b>	<b>12.77</b>
Mean_98	Upstream	<b>5.56</b>	<b>0.56</b>	<b>53.70</b>	<b>47,431</b>	<u>12.66</u>
	Downstream	<b>3.35</b>	<b>1.34</b>	<b>47.02</b>	<b>293,456</b>	<u>12.88</u>
Mean_99	Upstream	<b>4.86</b>	<u>0.85</u>	<b>57.19</b>	<u>80,608</u>	<b>72.30</b>
	Downstream	<b>3.21</b>	<u>0.81</u>	<b>46.78</b>	<u>83,918</u>	<b>12.38</b>
No Odor	Upstream	<b>5.16</b>	<b>0.68</b>	<b>52.12</b>	<u>106,725</u>	<b>57.81</b>
	Downstream	<b>3.97</b>	<b>1.29</b>	<b>40.66</b>	<u>149,721</u>	<b>6.87</b>
Odor	Upstream	<b>5.31</b>	<u>0.77</u>	<u>58.39</u>	<b>60,458</b>	<b>65.00</b>
	Downstream	<b>2.69</b>	<u>0.99</u>	<u>53.22</u>	<b>355,502</b>	<b>22.64</b>

Differences in **Bold** values are significant at p=0.05

Underline pairs represent values that are not different at p=0.05

The DO values decrease moving downstream as expected. The bayou’s oxygen demand and low re-aeration capabilities in the covered portion are one explanation of this decline.

The BOD values also decrease moving downstream, except in 1998. These results suggest that the bayou has some assimilative capacity for organic materials entering from runoff.

All the upstream FC values regardless of grouping are within one standard deviation of the mean value for all upstream data except for the No Odor condition when the upstream value is nearly two standard deviations higher. This result means that for all practical purposes the FC values in the upstream locations are the same for Mean\_98, Mean\_99,

and Odor groupings. The downstream FC values are always greater than the upstream value except for the Mean\_99 grouping and the No Odor grouping where the values are the same (downstream is same as upstream). These results can be interpreted to suggest that during No Odor conditions the FC values are unchanged as one moves downstream, but during odor conditions the FC increases moving downstream.

Further comparison of the Odor and No Odor grouping shows that the upstream DO, FC, and BOD are all about the same value, but downstream during odor conditions, the FC triples. This result suggests that the odor conditions are either caused by some input between Evergreen and Hughes or by some process difference between Evergreen and Hughes. The 1999 FC difference is negligible suggesting that the input or process involved has changed. Because a natural process change is unlikely, one can conclude that the input character into this section of the bayou has changed since 1998.

This section of bayou is particularly interesting because it is very slow moving, and although it has a free surface it is not directly open to the air. Based on video tapes, the air in this section of bayou is breathable (workers were shown in the bayou without supplied air) during no-odor conditions.

*Results: Quarterly Data Summaries*

The data sets were grouped into quarterly time blocks for a summary analysis of selected water quality parameters. Statistical tests are not performed on the data presented in this section. The quarterly bar charts use the following conventions. Q1 represents data collected in January, February, or March. Q2 is data collected in April, May, or June. Q3 is data collected in July, August, or September. Q4 is data collected in October, November, or December.

Figure 4.3 is a bar chart relating the mean DO value when odor was reported in the field notes and when no odor was reported, grouped quarterly, at the downstream location group. The mean values during the first five quarters of the research are remarkably consistent, with the odor conditions being producing a 1mg/L reduction in DO at Hughes Street Bridge.

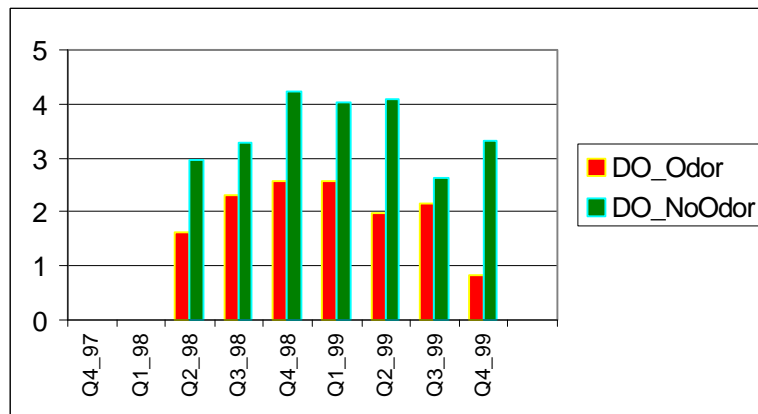




Figure 4.3 Mean DO values when odor was observed (not observed) grouped quarterly

Figure 4.4 is a bar chart relating the mean ammonia value when odor was reported in the field notes and when no odor was reported, grouped quarterly, at the downstream location group. The mean values are about the same regardless of whether odor is reported or not, except for the first quarter of 1998. This result suggests that ammonia is not a useful indicator or predictor of odor conditions.

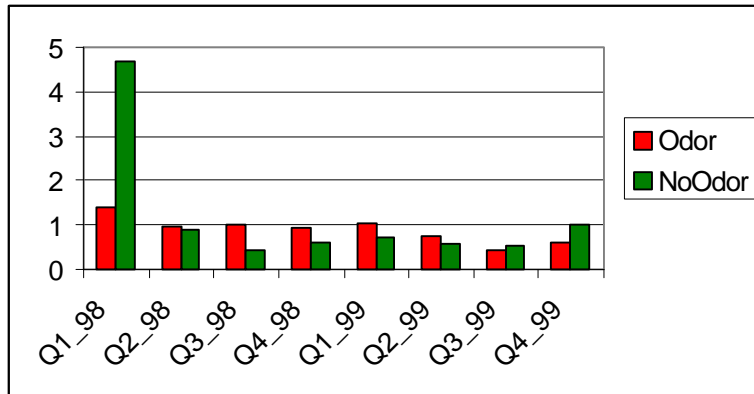


Figure 4.4 Mean Ammonia values when odor was observed (not observed) grouped quarterly.

Figure 4.5 is a bar chart relating the mean sulfate value when odor was reported in the field notes and when no odor was reported, grouped quarterly, at the downstream location group. Although the differences in the mean values are significant, this parameter is not considered useful as a predictor/indicator of odor conditions because it had a wide range of values and the upstream-downstream behavior was inconsistent.

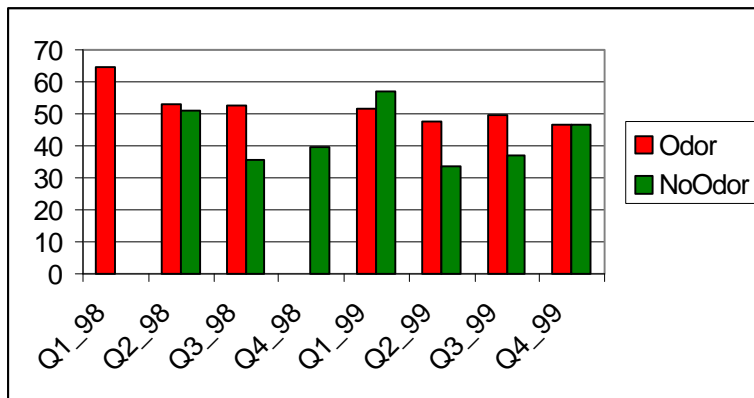


Figure 4.5 Mean Sulfate values when odor was observed (not observed) grouped quarterly

Figure 4.6 is a bar chart relating the mean FC values when odor was reported in the field notes and when no odor was reported, grouped quarterly, at the downstream location group. The mean values are about the same regardless of whether odor is reported or not. The last three quarters indicate a downward trend in the value of FC.

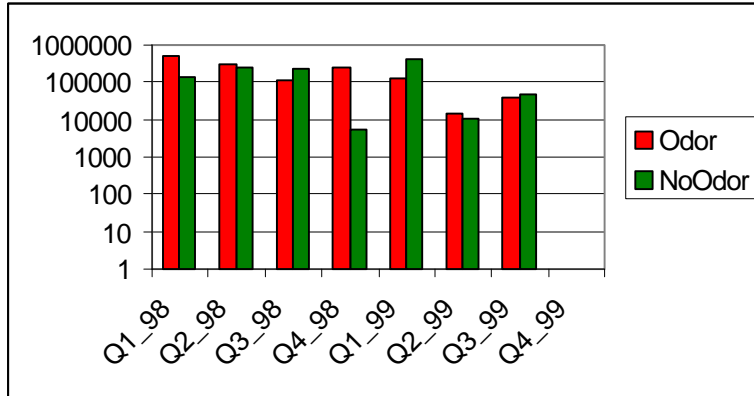


Figure 4.6 Mean FC values when odor was observed (not observed) grouped quarterly

Figure 4.7 is a bar chart relating the mean BOD value when odor was reported in the field notes and when no odor was reported, grouped quarterly, at the downstream location group. The mean values are much higher when odor is reported indicating that one cause of the odor is an organic load that elevates the BOD values. BOD appears to be a good predictor of odor conditions, (with DO responding in a downward fashion as the oxygen demand uses DO in the water column).

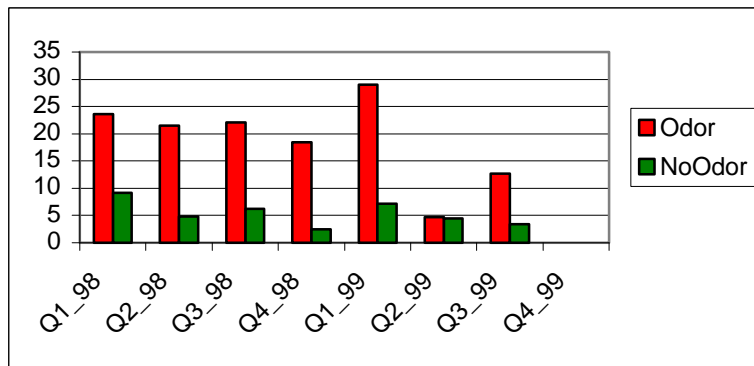


Figure 4.7 Mean BOD values when odor was observed (not observed) grouped quarterly

Figure 4.8 is a bar chart relating the fraction of field visits when odor was reported in the field notes and the fraction of field visits when no odor was reported, grouped quarterly. The odor fraction is plotted as the left bar, while the no-odor fraction is plotted as the right bar. The ideal condition is the right bar at 100% and the left bar absent. The number of field visits in each quarter is different, but the minimum number during the study period was six visits where data were collected. This minimum number is roughly a visit once every two weeks. Most of the quarters reflect weekly visits. The third quarter of 1999 and fourth quarter of 1999 had a relatively low fraction of observed odor, suggesting that the water quality during this period was acceptable.

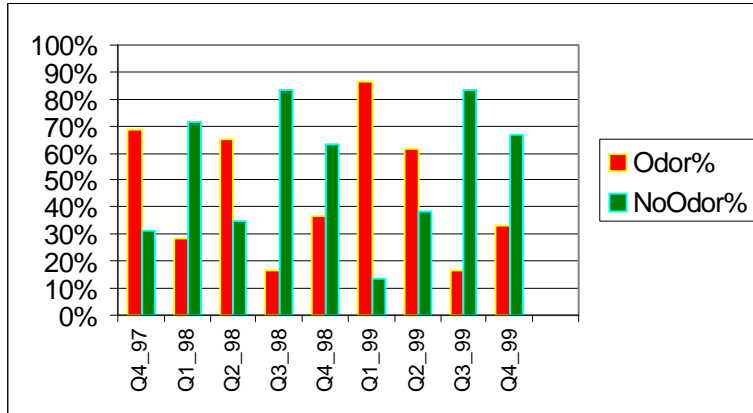


Figure 4.8 Fraction of field visits when odor was observed grouped quarterly

These summary statistics display two useful characteristics. The first is that the frequency of odor reports is lowest in the third quarter of each year studied (July-September) and highest in the first quarter of each year (January -March). The frequency of odor occurrences is about the same for quarters two and four. The question as to why the number of observed odor incidents is smallest in the third quarter is unanswered.

The second useful characteristic is that the DO at Hughes during no-odor conditions is about 1 mg/L higher than the DO regardless of quarter suggesting that the cause of the odor conditions is something that exerts an oxygen demand on the bayou system. The third quarter is again anomalous in that the difference in DO between the two conditions is the smallest and the value of DO is smallest.

The third useful characteristic is that BOD is significantly higher during odor conditions - consistent with the DO behavior above.

*Summary*

During odor conditions DO and BOD are significantly different than during non-odor conditions. The elevated BOD indicates that some source (sewage, industrial discharge, etc.) supplies an additional organic load to the bayou that in-turn depresses the DO. Odor likely results when this mixture sits relatively stagnant under the Hughes facility. The sediment in this area exerts an oxygen demand as well and because the water is moving slowly vertical mixing does not occur. The sediment is postulated to become anaerobic and sulfate reducing bacteria in this zone produce sulfide that contributes to the odor.

The mean value of DO and Sulfate meets existing or proposed state water quality standards for an unclassified stream. The FC values do not. Only about 25% of the FC values meet the current standard (2000 cfu/100mL). However, many samples were collected when the bayou water quality appeared poor (especially the older data) so this poor performance may be from a biased sampling protocol.

Based on the upstream-downstream data differences a *change* in water quality occurs between Evergreen Cemetery and Hughes Street. The upstream-downstream analysis supports the concept that the source of BOD is upstream of Evergreen Cemetery.



## 5. Tracer Study

### *Purpose*

The purpose of the tracer test is to determine connectivity and average time-of-travel (residence time) in different sections of Country Club Bayou and to estimate the degree of mixing. The tracer tests were conducted in response to a series of tests reported by the City of Houston team. The COH tests were conducted in February 1999. Table 5.1 lists the results reported by the COH team in a project meeting at HDHHS on March 23, 1999.

Table 5.1 Time of Travel COH Tests

<u>Deployment Location</u>	<u>Distance to Hughes</u>	<u>Time-of-travel</u>
North Vent	2250 feet	1 day
South Vent	2250 feet	3 days
Polk and Elm (S1)	3000 ft	2 days, North tunnel
Park and Elm (N1)	2800 ft	2 days, North tunnel

The University of Houston experiments were conducted as independent confirmation of the visual tracer tests performed by the city team.

### *Methods*

The tracer used in the tracer studies is Sodium Fluorescein, an organic dye that is commonly used in tracer studies and medical applications. It is considered conservative although it is known to be strongly adsorbed by alumina (a component of clay). These tracers are detected visually for high concentrations and using a fluorometer for low concentrations. The tracers are released at an upstream location, monitored at that location for a short time, then monitored at the downstream location to determine time-of-travel.

The tracer concentration curve is interpreted by fitting an advection-dispersion model. The purpose of the model is to infer peak arrival time and dispersion characteristics of the tracer. Prior to deployment the analyst estimates the system volume and flow rate to calculate a hydraulic retention time. The release is scheduled so that the pulse arrives at the downstream location over a convenient period. Generally travel times less than 8 hours or greater than 20 hours are ideal because nighttime sample collection is avoided.

The volume estimate is also used to determine the tracer dose. In these studies the dose was designed so that if the tracer is uniformly mixed into the entire system volume the concentration is 0.1 to 1.0 mg/Kg (mg/L). These concentrations are barely visible. The choice of a concentration that is barely visible has advantages in terms of public perception of impact – although the tracers are considered non-toxic, the color is nearly identical to glycol-based anti-freeze and large concentrations in the water can cause concern among the public.

### *Results*

Figure 5.1 is a map of the area for the three experiments. In the first study a fluorescein dye tracer is released at the North Vent access shaft and concentration is monitored at the outfall near Hughes Street Bridge. The flow-line distance from the North Vest to the Railroad Bridge was estimated as 2,250 feet using the storm sewer division map of the area (Sheet 209B; 12-1-1980)

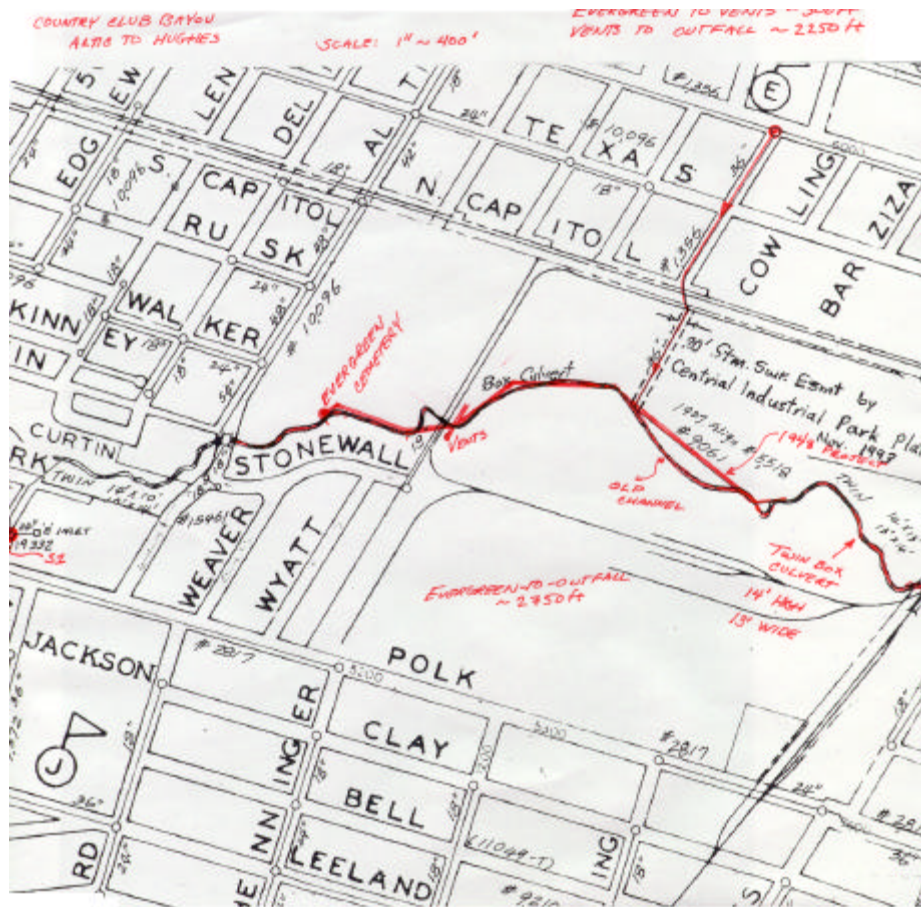


Figure 5.1. Map of Tracer-Test Area for Tracer Study

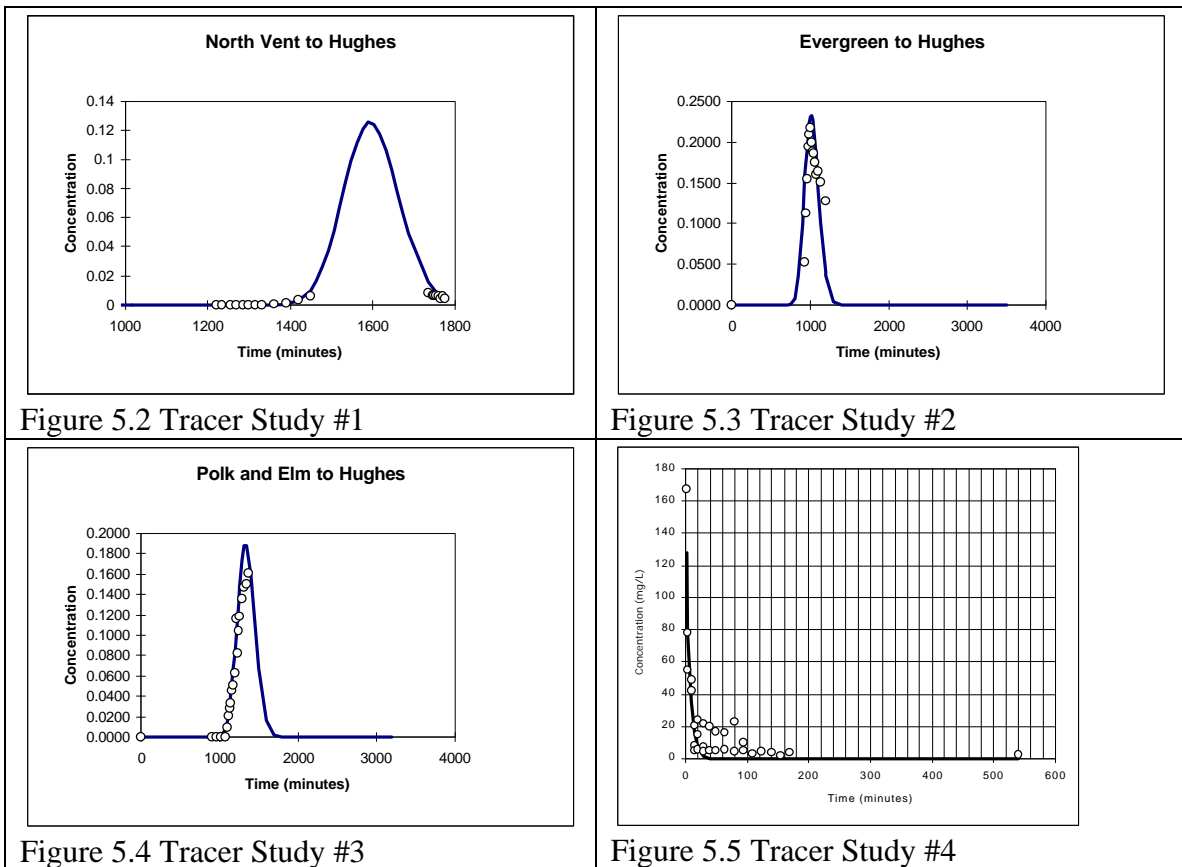
flow width is assumed to be a uniform 13 feet based on the construction drawings for the 1948 storm sewer project in the area when the entire Hughes site was covered (Drawing 9061). The maximum low flow depth is 3.25 feet based on the same drawing. In the drawing openings with a bottom sill of 3.25 feet are shown every 200 feet. If the flow depth is above 3.25 feet, then there would be potential for mixing between the two culverts, otherwise flow is in separate channels.

The stagnant water volume is estimated as the product of this flow depth, the flow length and the flow width. The stagnant water volume on 5/6/1999 was 2796 cubic meters. 277 grams of dye is required to label this mass of water at 0.1 mg/Kg. At a flow depth of

one-tenth the maximum (about 3 inches) only 27 grams of dye is required to label the entire water mass.

The tracer mass used in this first experiment was 20 grams (20,000mg), about one-tenth to mass required to label the entire volume. This mass was selected because the water is known to be moving relatively slowly and mixing should be small. We assumed we could label and detect one-minute of flow and used this volume to calculate the mass of tracer required. The field-measured depths on 5/6/99 are 2.33 feet at the North Vent, and 2.5 feet at the outfall. The discharge at the outfall is 520 gpm. The expected hydraulic retention time is 18.4 hours.

The outfall location had a sulfide odor characteristic of a poor water quality episode and the water was cloudy. The bottom of the bayou was black-brown. Spheratolis was visible downstream of the outfall in the faster flowing portion between the railroad bridge and Hughes Street.



The North Vent water was clear with some decaying plant matter (juniper leaves) and dirt. The odor was unremarkable. Dye was placed at 9:50 am on 5/6/99. Two samples were collected at 10:00 and 10:05 am at the North Vent, to be sure dye was diluting and moving downstream. Later dye studies will monitor the release location for several hours. The field team left the Hughes property at 10:05 am at the request of the Baker-

Hughes plant manager. The downstream location was monitored hourly for 9 hours. No dye was visible after 9 hours. The location was monitored the next morning for 4 hours, and the afternoon for 2 hours.

Figures 5.2 through 5.5 are plots of the concentration histories for all the tracer studies. The markers are the observations, the solid line is the fitted model. The parameters used to fit the model were used to estimate discharge and determine a mixing length. A large mixing length indicates that the system is functioning like a mixed reactor, while a small length indicates plug-flow type behavior. The mixing lengths are relatively small suggesting that the covered portion of the bayou is similar to a plug flow system.

Table 5.2 is a summary listing the results of the tracer studies.

Table 5.2 Time of Travel UH Tests

<u>Release</u>	<u>Recovery</u>	<u>Distance</u>	<u>Travel time</u>	<u>Discharge<sup>1</sup></u>	<u>Discharge<sup>2</sup></u>	<u>Mixing Length</u>
North Vent	Hughes	2250 feet	1.1 day	520 gpm	342 gpm	2 feet
Evergreen	Hughes	2750 feet	0.7 days	1694 gpm	660 gpm	11 feet
Polk and Elm	Hughes	3750 feet	0.9 days	2500 gpm	630 gpm	13 feet
Ennis	Milby	2400 feet	0.3 days	N/A		N/A
Hughes	Polk and 66th	1450 feet	0.2 days	600 gpm	598 gpm	N/A
Ennis	Ennis	1 feet	N/A	N/A	2.5 gpm <sup>3</sup>	N/A

<sup>1</sup>Discharge measured at Hughes Street using method in Appendix I

<sup>2</sup>Discharge calculated from travel time

<sup>3</sup>Discharge estimated by dilution

Appendix III contains the data collected in the tracer tests and the interpretation calculation spreadsheets.

### *Interpretation*

The travel time in the covered portion is on the order of several hours upstream of the junction box at Altic. The covered portion from Altic to Hughes has a remarkably long travel time for the linear distances involved. The large width of this section of bayou allows relatively small changes in depth to store large volumes of flow and thus upstream flow changes are passed through this section undetected. Even the highest flow event had a travel time through this portion of the system on the order of one day. This long travel time in the section between Altic and Hughes may contribute to odor conditions because the water in this portion of the bayou is essentially stagnant.



## 6. Computer Modeling

### Hydraulic Model

Hydraulic modeling is used to predict flow velocities for use in a water quality model that considers the advection of dissolved compounds as the principal transport mechanism. The gradually varied flow equation is used (same equation as HEC-2; WSPRO, etc.). The idea in the hydraulic modeling is to construct a model that produces realistic travel time when field measured flow depths are supplied.

Figure 6.1 is a sketch defining the terms used in the hydraulic calculations. The spreadsheet implementation uses the following difference equations for calculating flow depth.

$$y_i = y_{i-1} + \Delta x(S_o - S_f) \quad 6.1$$

$$y_i = y_i + \frac{Q^2}{2gA_i^2} \quad 6.2$$

where  $h_i$  is the flow depth in the section,  $S_o$  and  $S_f$  are the bottom and friction slope, respectively, and  $\Delta x$  is the length of the section.

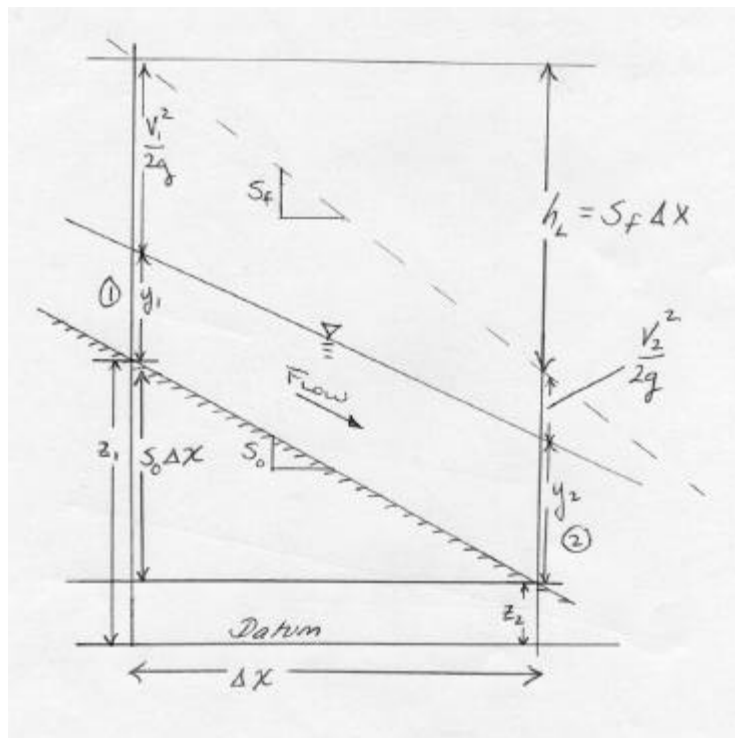


Figure 6.1 Sketch of variables used in hydraulic calculations

The difference between these two equations is minimized by the choice of  $h_i$  for each section using the SOLVER feature in Excel. When the minimum is found for all sections the result is a backwater curve that satisfies the gradually varied flow equation. Figure 6.2 is an example spreadsheet that implements the method.

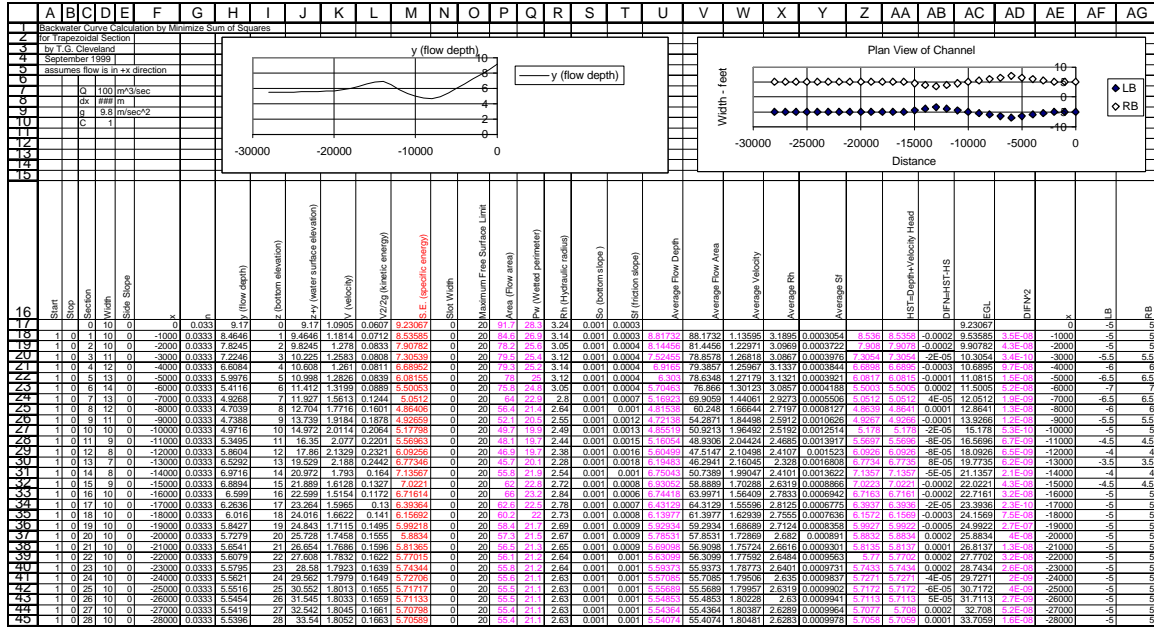


Figure 6.2 Hydraulic Model Spreadsheet

The cross sectional areas are determined from cross-sectional geometry inferred from a USGS topographic map. These sections are individually analyzed to produce a depth-area function that is used in the spreadsheet to relate flow depth to cross sectional area. The USGS map and the sections are attached in Appendix V.

After considerable experimentation the channel sections were changed to a simple rectangular geometry and only the width was adjusted. This change was made because the low flow depths required were on the order of 3-6 feet and we did not have actual bathymetry from the USGS maps. Additionally in early September 1999 the channel passing by the METRO Polk Garage was modified. As of November 10, 1999 the construction is still in-progress. The work will extend the box culvert under Polk Street several hundred feet to create an additional section of covered bayou.

*Roughness Coefficients*

Figure 6.3 is a table of roughness coefficients for use in open channel flow modeling.

TABLE 8.2 Roughness Coefficients for Open Channels

Description of Channel	<i>n</i>
Exceptionally smooth, straight surfaces; enameled or glazed coating; glass; lucite; brass	0.009
Very well planed and fitted lumber boards; smooth metal; pure cement plaster; smooth tar or paint coating	0.010
Planed lumber; smoothed mortar (1 sand) without projections, in straight alignment	0.011
Carefully fitted but unplanned boards; steel troweled concrete, in straight alignment	0.012
Reasonably straight, clean, smooth surfaces without projections; good boards; carefully built brick wall; wood troweled concrete; smooth, dressed ashlar	0.013
Good wood, metal, or concrete surfaces with some curvature, very small projections, slight moss or algae growth or gravel deposition; shot concrete surfaced with troweled mortar	0.014
Rough brick; medium quality cut stone surface; wood with algae or moss growth; rough concrete; riveted steel	0.015
Very smooth and straight earth channels, free from growth; stone rubble set in cement; shot, untroweled concrete; deteriorated brick wall; exceptionally well excavated and surfaced channel cut in natural rock	0.017
Well-built earth channels covered with thick, uniform silt deposits; metal flumes with excessive curvature, large projections, accumulated debris	0.018
Smooth, well-packed earth; rough stone walls; channels excavated in solid, soft rock; little curving channels in solid loess, gravel, or clay with silt deposits, free from growth and in average condition; deteriorating uneven metal flume with curvatures and debris; very large canals in good condition	0.020
Small, human-made earth channels in well-kept condition; straight natural streams with rather clean, uniform bottoms without pools and flow barriers, cavings, and scours of the banks	0.025
Ditches; below-average human-made channels with scattered cobbles in bed	0.028
Well-maintained large floodway; unkept artificial channels with scours, slides, considerable aquatic growth; natural stream with good alignment and fairly constant cross-section	0.030
Permanent alluvial rivers with moderate changes in cross section, average stage; slightly curving intermittent streams in very good condition	0.033
Small, deteriorated artificial channels, half choked with aquatic growth; winding river with clean bed, but with pools and shallows	0.035
Irregularly curving permanent alluvial stream with smooth bed; straight natural channels with uneven bottom, sand bars, dunes, few rocks and underwater ditches; lower section of mountainous streams with well-developed channel with sediment deposits; intermittent streams in good condition; rather deteriorated artificial channels, with moss and reeds, rocks, and slides	0.040
Artificial earth channels partially obstructed with debris, roots, and weeds; irregularly meandering rivers with partly grown-in or rocky bed; developed flood plains with high grass and bushes	0.067
Mountain ravines; fully ingrown small artificial channel; flat flood plains crossed by deep ditches (slow flow)	0.080
Mountain creeks with waterfalls and steep ravines; very irregular flood plains; weedy and sluggish natural channels obstructed with trees	0.10
Very rough mountain creeks; swampy, heavily vegetated rivers with logs and driftwood on the bottom; flood plain forest with pools	0.133
Mudflows; very dense flood plain forests; watershed slopes	0.22

Figure 6.3. Roughness coefficients for open channels  
(from: Simon A.L., and Korom, S.F., 1997. *Hydraulics 4ed.*, Prentice Hall, Ohio. 443 p.)

The portion of the bayou from Hughes Street to the confluence with Braes Bayou was modeled with a roughness coefficient of ranging from 0.035 at the confluence to 0.06 for the section between Polk Street and the outfall at Hughes Street. The later value is near the upper range of values listed in the table. The justification for selecting this value is that the bayou meanders in this location and has a relatively rough, uneven bed.

This hydraulic model was used to develop flow-depth-velocity data for the water quality model. In developing these data a range of discharges was chosen an the hydraulic model

used to compute the depth with matching conditions forced at the downstream portion of the model. The average depth for each section in the hydraulic model was saved and a linear regression model was used to develop a discharge-depth power-law relationship for use in the water quality model.

### *Water Quality Model*

Figure 6.4 is a map of the QUAL2E model used in evaluating water quality impacts on Country Club Bayou from the five alternative intervention measures. The blocks represent the approximate location of the element centroids in the QUAL2E computer program and the numbers are the element numbers used in the program. Two reaches, upstream from block 1 are displayed in the figure as a single block labeled 20+. These reaches represent the portion of the bayou from Ennis and Lamar to Altic.

The conceptual model for the basin assumes that the upstream portion of the bayou can be approximated as a water source (headwater) element in the program and the entire south branch is simulated in the same fashion. The input, if any, at Yates Gully is simulated as a point source of water with associated water quality constituents.

The sub-basin model simulates dissolved oxygen (DO) and BOD using flow values independently computed using a hydraulics model. This separation is necessary because the hydraulic component included in QUAL2E could not produce the backwater-type flow regime observed in Country Club bayou.

### *Installation, Testing, and Calibration*

The program was installed from the US EPA website. After downloading the program a series of simulations using EPA supplied input files was performed to test the installation. Table 6.1 lists the more significant test simulations, the purpose of the test, and important remarks. The last three test simulations used the geometry depicted in Figure 6.4.

Table 6.1 Installation and Testing Simulations

Simulation	Purpose	Remarks	Data File
T001	Verify QUAL2E install	Use EPA supplied files	WRKSHOP1.DAT
T002	Disable DAM feature	Modify EPA file	NOT SAVED
T003	Disable Algae	Modify EPA file	NOT SAVED
T004	CCBayou geometry	Modify EPA file – rename	NOT SAVED
T005	Test element sequence		NOT SAVED
T006	Test line-plot		NOT SAVED
T007	Sub-model geometry	Junction element sequence OK	NOT SAVED
T008	Plot commands	Plot sequence OK	NOT SAVED
T009	Point loads	Load at Vents – Impact visible	NOT SAVED
T010	Correct dimensions	River km actual dimensions	BAYOU1.DAT

Calibration for No-Odor conditions was accomplished by trial-and-error. Various input parameters in the QUALE input files were changed and the simulation output compared



to the mean values observed in the field monitoring study. The goal in these cases was to force the predicted DO and BOD to fall within a prescribed calibration range based on the data analysis.

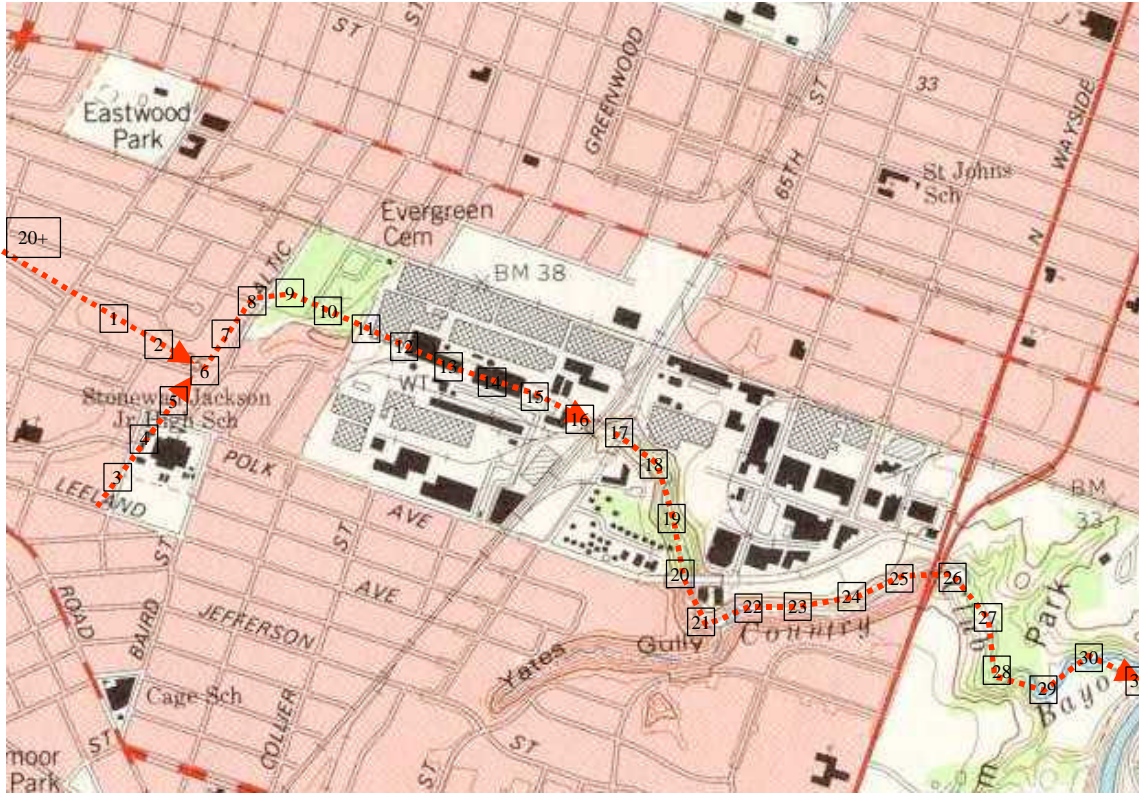


Figure 6.4. Diagram of QUAL2E sub-basin model for water quality prediction on Country Club Bayou.

A series of hydraulic simulations was used to determine appropriate hydraulic coefficients for the test runs. The coefficients were determined by fitting the mean flow depths and mean flow velocities to power-law stage-discharge and velocity-discharge relationships. These relationships are the models used in QUAL2E when the functional hydraulics representation is used. Table 6.2 lists the more significant simulations, the purpose, and meaningful remarks. The last five of these simulations involved testing the effect of flow variation on the calibration.

Table 6.2 Calibration Simulations Set#1

Simulation	Purpose	Remarks	Data File
C001	Calibrate flows		BAYOU2.DAT
C002	DO,BOD Inputs	Try to get BOD,DO at Hughes	CCB_CR1.DAT
C003	DO,BOD Inputs		CCB_CR2.DAT
C004	BOD point load	5% Flow BOD=10	CCB_CR3.DAT
C005	Increase BOD point	5% Flow BOD=30	CCB_CR4.DAT

C006	Increase BOD point	5% Flow BOD=50;Up loads =1	CCB_CR5.DAT
C007	Adjust SOD	0% Flow 0 BOD	CCB_CR6.DAT
C008	Adjust SOD rates	0% Flow; 6.5 BOD;DO up 6.0	CCB_CR7.DAT
C009	Adjust geometry	Add reach for Ennis and Lamar	CCB_CR8.DAT
C010	Refine calibration	DO Ok, BOD low	CCB001.DAT
C011	Refine calibration	DO Ok, BOD Ok	CCB002.DAT
C012	Fix flow coefficients	Adjust from 5 hydraulics runs	NOT SAVED
H001	Lowest flow	Calibrate to fit DO,BOD	CCBH01.DAT
H002	Mean-SD Flow	Calibrate to fit DO,BOD	CCBH02.DAT
H003	Mean Flow	Calibrate to fit DO,BOD	CCBH03.DAT
H004	Mean+SD Flow	Calibrate to fit DO,BOD	CCBH04.DAT
H005	High Flow	Calibrate to fit DO,BOD	CCBH05.DAT

The effect of flow variation was significant so the five simulations using flows distributed about the mean flow were used in further calibration exercises. The values in the mean flow simulation were used and the range of validity of the calibration was tested. Table 6.3 lists the results of this second set of calibration exercises.

Table 6.3 Calibration Set#2: All input values are held constant except the flow.

Flow	DO at Hughes	BOD at Hughes	DO at Wayside
Q = 145 gpm (lowest)	3.86	1.42*	5.02
Q = 286 gpm (Mean-SD)	3.54	3.22*	4.88
Q = 370 gpm (Tracer test)	3.44	4.08*	4.80
Q = 520 gpm (Tracer test)	3.37	5.18	4.67
Q = 855 gpm (Mean)	3.29	6.63*	4.48
Q = 1424 gpm (Mean+SD)	3.43	7.92*	4.36
Q = 2800 gpm (Highest)	3.63	8.91*	4.36
Target Ranges <sup>1</sup>	3.24 – 3.96	5.04-6.16	4.32-5.28

<sup>1</sup> Target ranges are the No Odor mean values +/- 10%

\* These values are outside prescribed target range

This set of simulations indicated that the calibration is valid (in terms of matching observed mean values) only for a single flow rate. Trial-and-error sensitivity analysis suggested that either re-aeration parameters or BOD loading could be changed to try to satisfy the prescribed calibration target values. Changing the re-aeration values was not selected because the purpose of the model is to test effects of different loading and different hydraulic changes on the water quality of the bayou. Instead the BOD loading was varied in an attempt to establish a useful range of validity of the model.

The approach to extend the range of usefulness of the model was to vary the BOD loading as the flow rate changes. The arbitrarily selected prescribed BOD range was 6.0 to 14.0 mg/L. The values in each program execution values were adjusted to try to meet all the target values. Table 6.4 shows the results of this series of simulations.

Table 6.4 Calibration Set #3.

Flow	DO at Hughes	BOD at Hughes	DO at Wayside	Upstream BOD
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Q = 145 gpm (lowest)	3.01*	3.64*	4.99	15.5*
Q = 286 gpm (Mean-SD)	3.21*	4.82*	4.79	15.0*
Q = 370 gpm (Tracer test)	3.24	5.10	4.73	12.5
Q = 520 gpm (Tracer test)	3.37	5.18	4.67	10.0
Q = 855 gpm (Mean)	3.41	5.97	4.54	9.5
Q = 1424 gpm (Mean+SD)	3.76	5.54	4.59	7.0
Q = 2800 gpm (Highest)	3.96	5.35	4.68	6.0
Target Ranges <sup>1</sup>	3.24 – 3.96	5.04-6.16	4.32-5.28	8.5-10.5

<sup>1</sup> Target ranges are the No Odor mean values +/- 10%

\* These values are outside prescribed target range

From this series of calibration exercises the baseline No-Odor condition was represented using the higher tracer test values with an upstream BOD concentration set in a range defined by the observed mean value to approximately twice the observed mean value at Hughes Street Bridge.

Odor conditions are simulated by adding point loads near the suspected locations in the study area (Near 3100 Lamar upstream of Altic). The selection of the input location is based on a theme-map constructed from the HDHHS and Field reports. The locations of reported discharges were determined by GPS and then adjusted manually to create an EXCEL overlay on a map image. Multiple locations were shifted slightly so that when plotted, these locations appeared to have “depth”. Figure 6.5 is a copy of the map created by this process.



Figure 6.5 Theme Map for Country Club Bayou

Visual inspection shows two areas that are likely source areas. The first is the area far upstream near N3 and the other is the area just upstream of Evergreen Cemetery. The field data do not suggest one area is more likely because the change in water quality parameters is observed only after the water has traversed the section beneath the Hughes Street facility.

Odor calibration was performed in a similar as the No-Odor cases fashion. The No-Odor cases were modified to include a point load with flow set to a 10% of the total flow and the BOD adjusted by trial and error until a set of target values was matched at Hughes Street. Table 6.5 shows the results of this series of simulations.

Table 6.5 Calibration Set #4.

Flow	DO at Hughes	BOD at Hughes	DO at Wayside	Point BOD
Q = 370 gpm (Tracer test)	1.06	15.43	4.03	99.0
Q = 520 gpm (Tracer test)	1.49	14.52	3.95	99.0
Q = 855 gpm (Mean)	0.74	21.30	3.88	99.0
Q = 1424 gpm (Mean+SD)	1.45	21.20	3.36	99.0
Q = 2800 gpm (Highest)	0.79	19.42	3.29	99.0
Target Ranges <sup>1</sup>	<2.5	>15.0	>3.0	

<sup>1</sup> Target ranges are arbitrary

\* These values are outside prescribed target range

The two sets represented by calibration sets #3 and #4 are used as the models to compare various water quality enhancement strategies.

#### *Channel Modification Simulations*

The channel modification simulation assumed that the section underneath the Hughes Facility is reduced to a width of 3 feet. The hydraulic model was used with this modification to produce a set of depth-velocity values for use in the QUAL2E model.

The result for this set of simulations is shown in Figures 6.6-6.9. On each figure the base (Odor) case is plotted in blue with the modified case plotted in green. The red arrow corresponds to a point source located near Milby that represents all the unknown upstream inputs of BOD into the model. The grey arrow simply indicates the outfall location in the model. Flow is from left to right in the figures. The leftmost location is at Ennis and Lamar while the rightmost location is at Wayside Drive. The number on the figure (e.g. C370D) refers to the flowrate in gallons per minute. The letter codes correspond to various simulation types.

The only types of simulations plotted in this report are for odor cases.

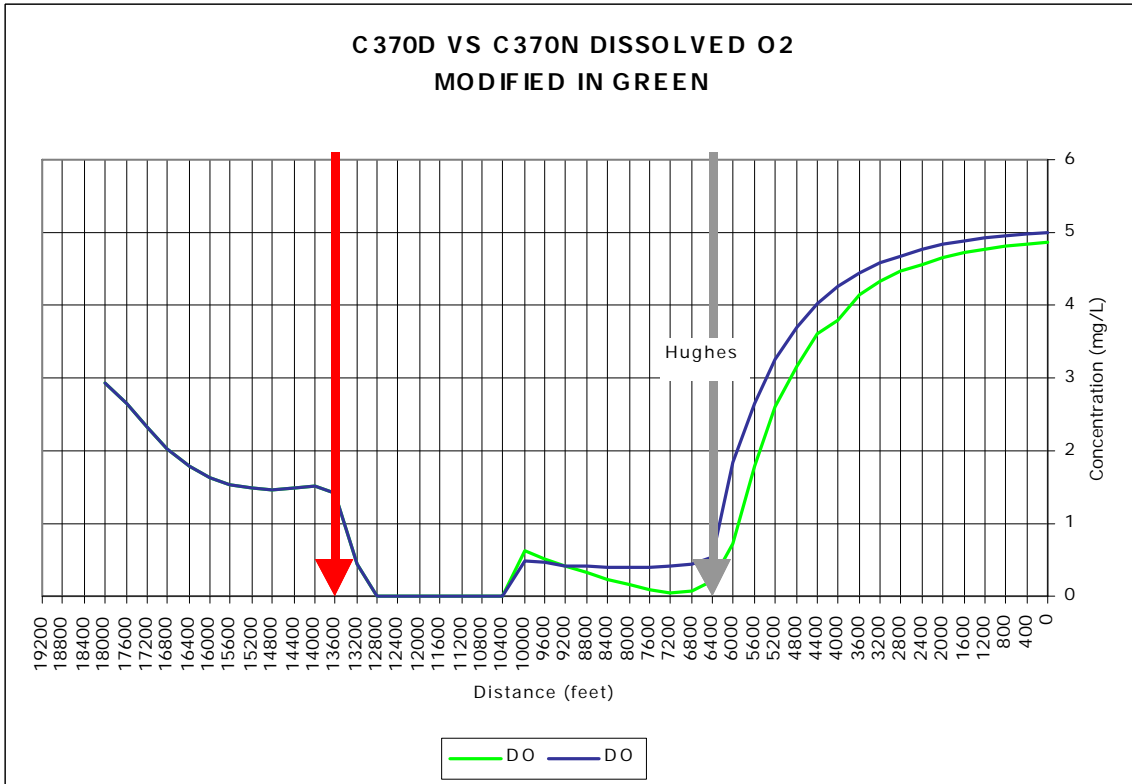


Figure 6.6. DO in system at Q=370GPM

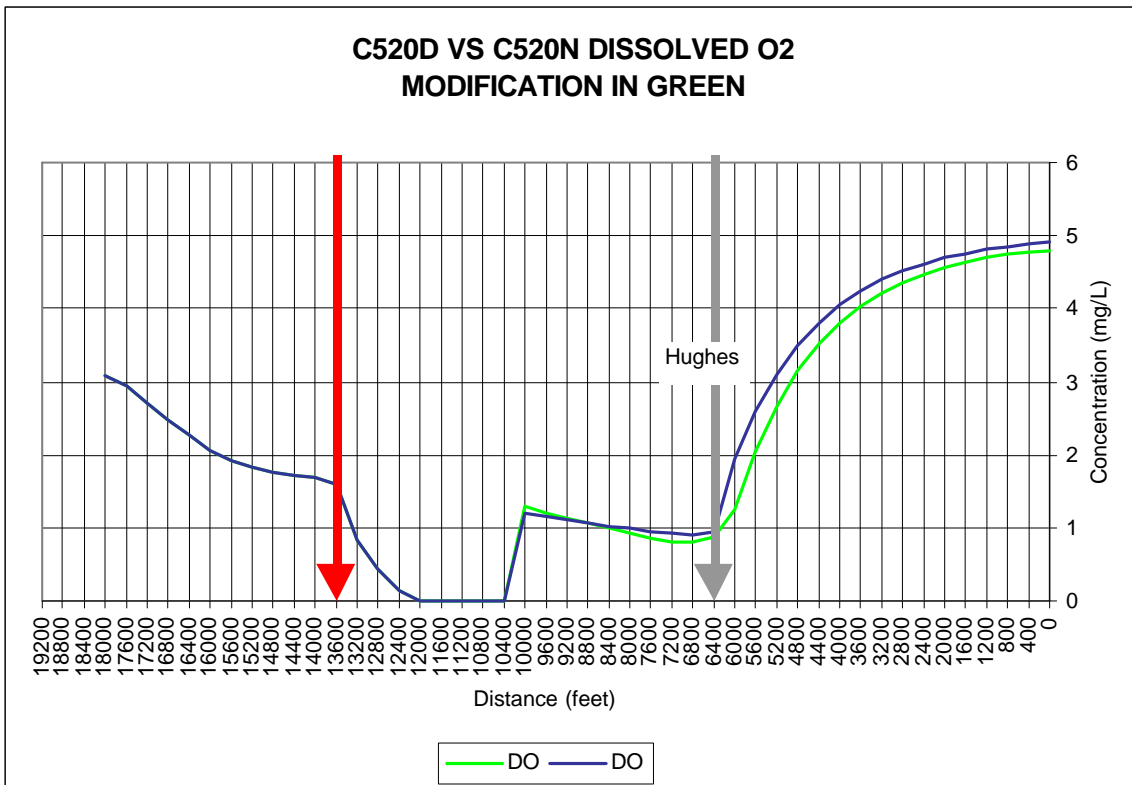


Figure 6.7. DO in system at Q=520GPM



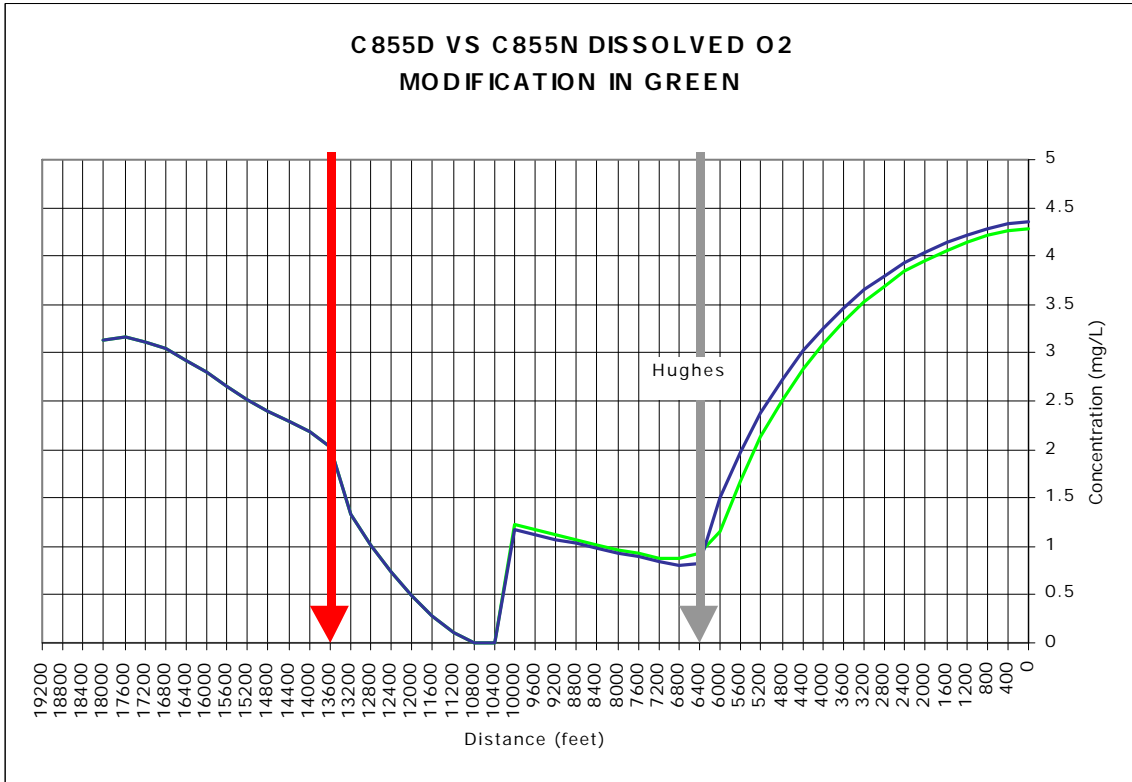


Figure 6.8. DO in system at Q=855GPM

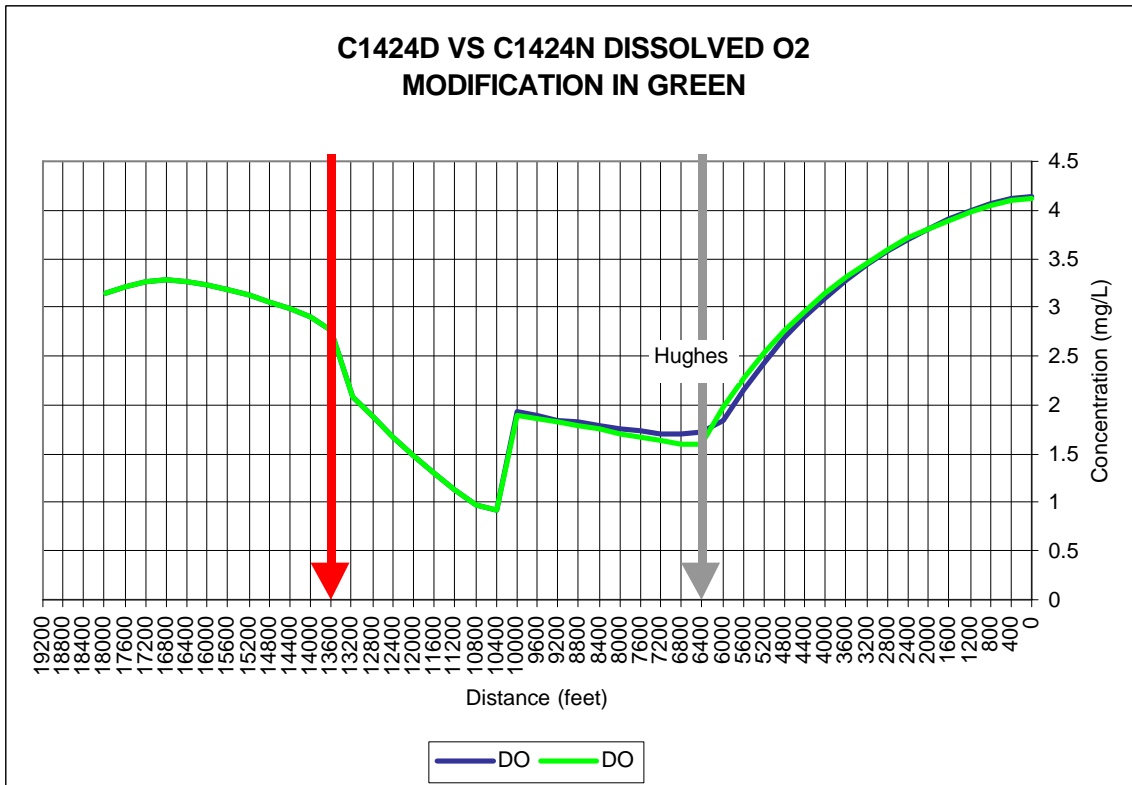


Figure 6.9. DO in system at Q=1424GPM

At the lowest flow the model predicts that a narrower channel actually makes the DO situation worse. Even at high flow the model predicts negligible improvement using the narrower channel, thus based on this model this strategy is not expected to be effective.

*Scheduled Flow Augmentation Simulations*

The scheduled flow augmentation strategy assumed that the flows were increased by 64 gpm at selected locations by release of water from a fire-hydrant. Two locations were studied with the model. The first location is upstream of the Altic street junction box, but downstream of the point load used to simulate the effect up upstream BOD loading in the drainage area. The second location is upstream of the point source. The two different locations were selected to determine if augmentation in the upper end of the drainage area produces a more improved water quality than augmentation in the lower (downstream) end of the watershed.

Figures 6.10-6.13 are plots comparing the odor baseline case with the augmented case at different system flow rates. Because the flow augmentation rate is constant the greatest impact occurs at the lowest system flow rate.

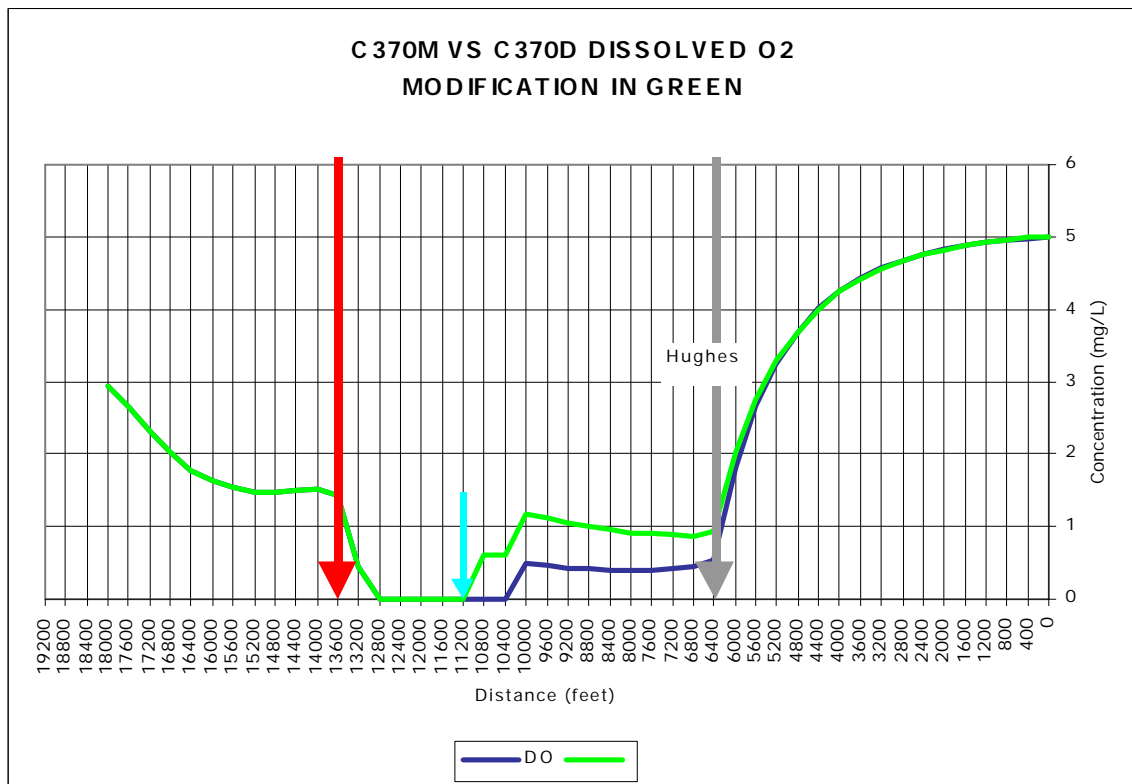


Figure 6.10. DO at Q=370GPM; Flow Augmentation near Altic St. Junction Box

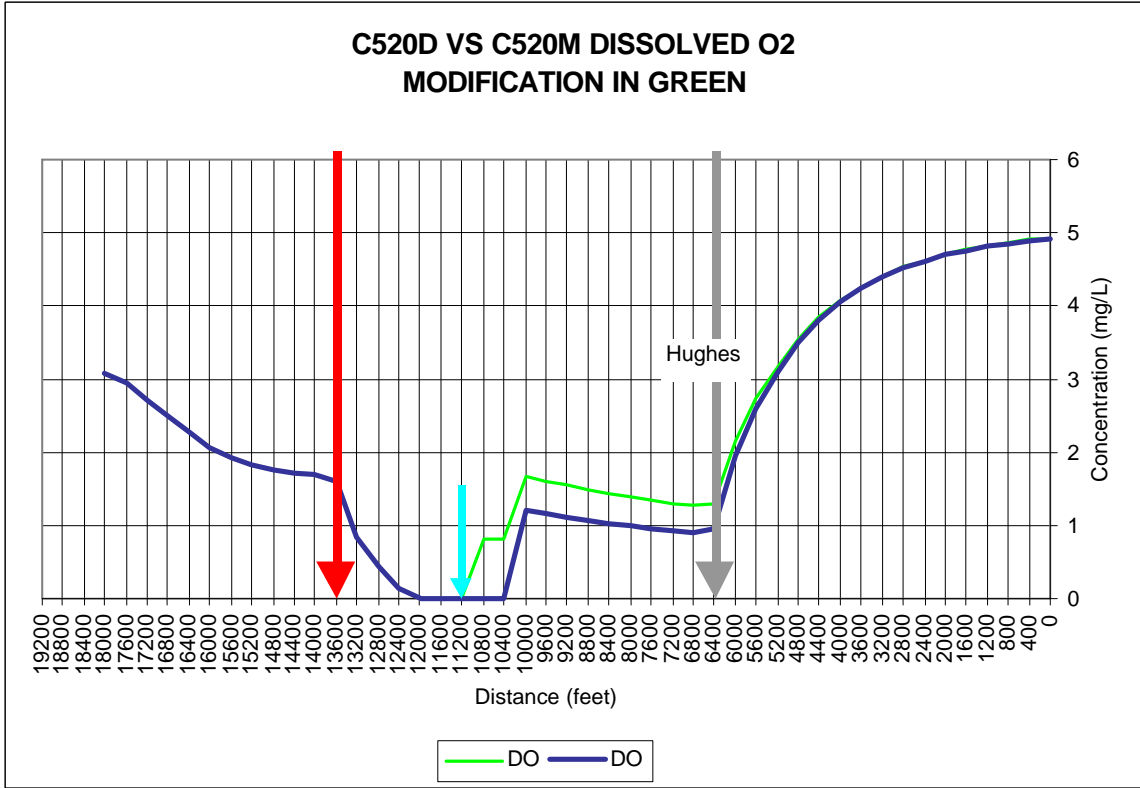


Figure 6.11. DO at Q=520GPM; Flow Augmentation near Altic St. Junction Box

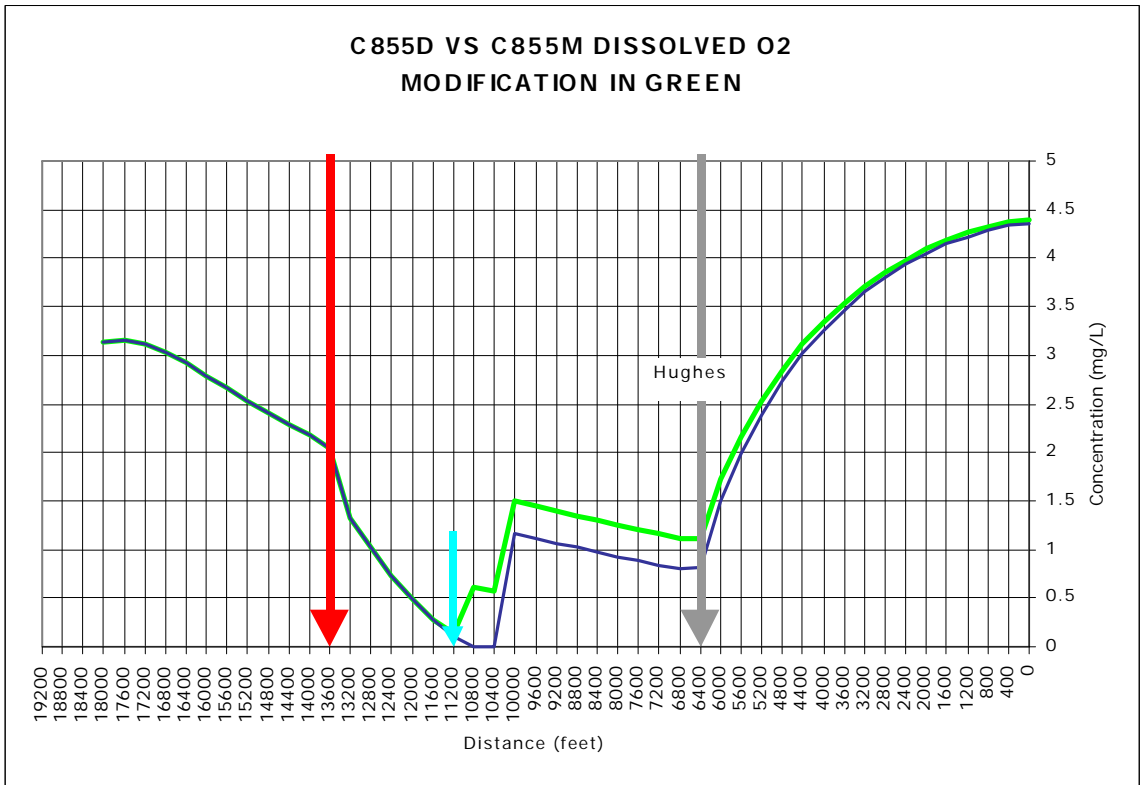


Figure 6.12. DO at Q=855GPM; Flow Augmentation near Altic St. Junction Box

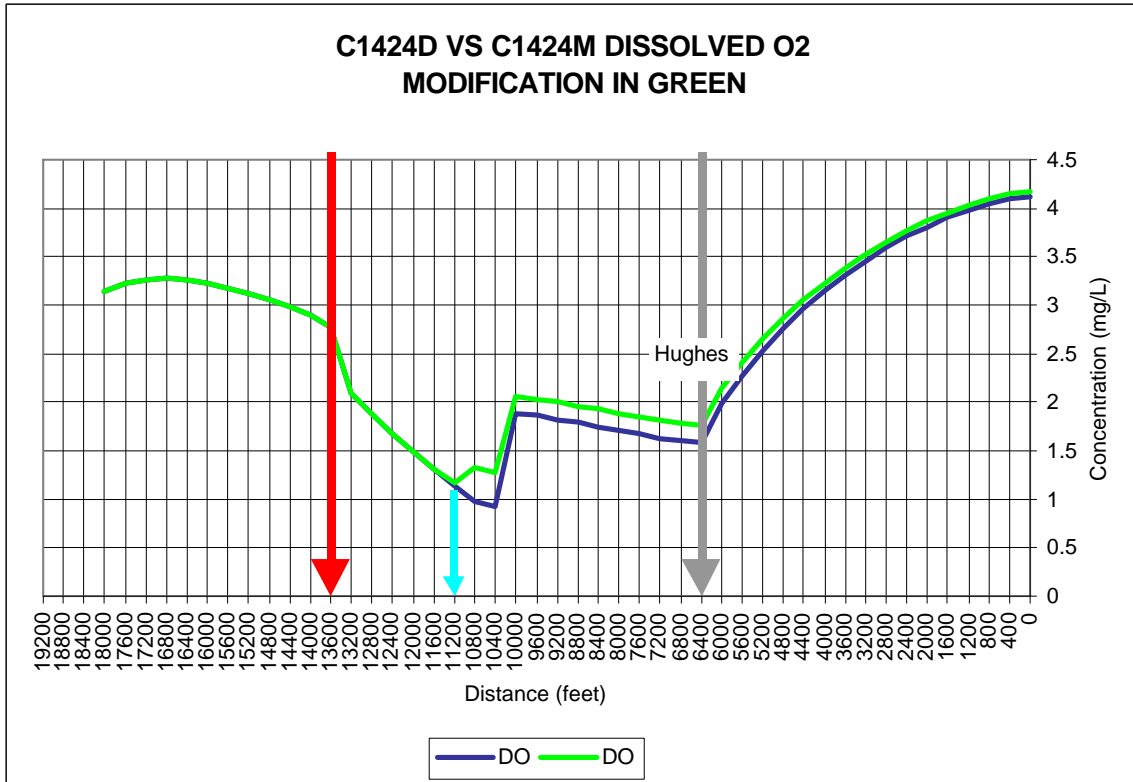


Figure 6.13. DO at Q=1424GPM; Flow Augmentation near Altic St. Junction Box

In all cases the model predicts that the addition of a small volume (4-20% of the total flow) of high quality (DO =5.5 mg/L) water improves the water quality at the Hughes Street outfall.

Figures 6.14-6.17 are a set of plots that present a similar set of simulations except the location of the flow augmentation has been moved upstream of the point source. These simulations represent flow augmentation near the upstream end of the drainage area, rather than just before the outfall.

Like the previous set of simulations the model predicts that the addition of a small volume (4-20% of the total flow) of high quality (DO =5.5 mg/L) water improves the water quality at the Hughes Street outfall. The amount of improvement is slightly less than the previous set of cases, however the results suggest that the location of the flow augmentation is unimportant (as long as it is upstream of the outfall). This insensitivity to input location is beneficial because multiple hydrants upstream of the outfall could be used.

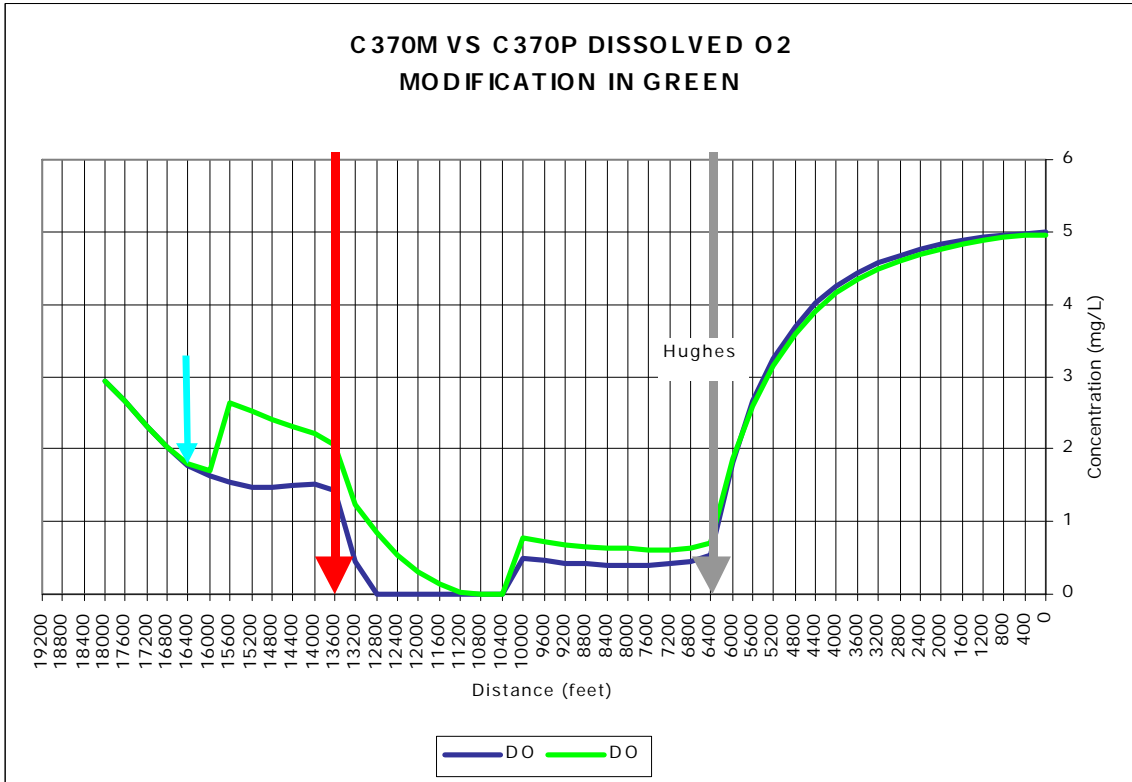


Figure 6.14. DO at Q=370GPM; Flow Augmentation between Ennis and Milby.

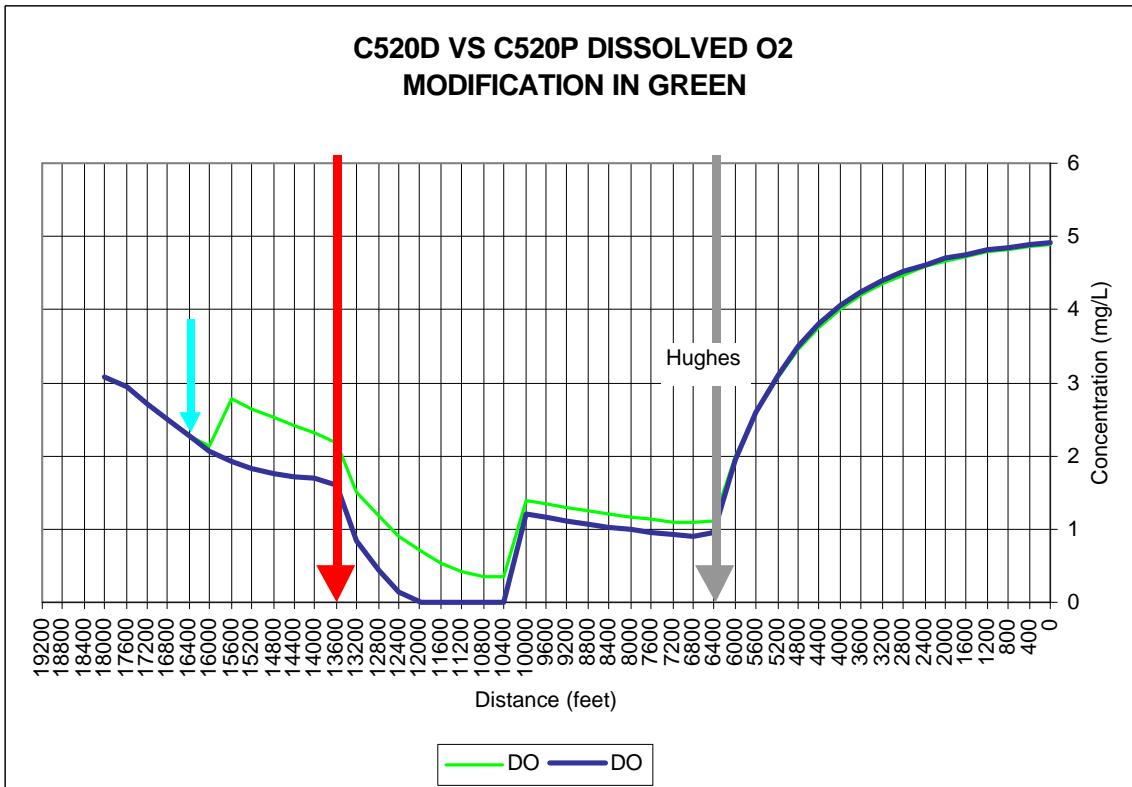


Figure 6.15. DO at Q=520GPM; Flow Augmentation between Ennis and Milby.



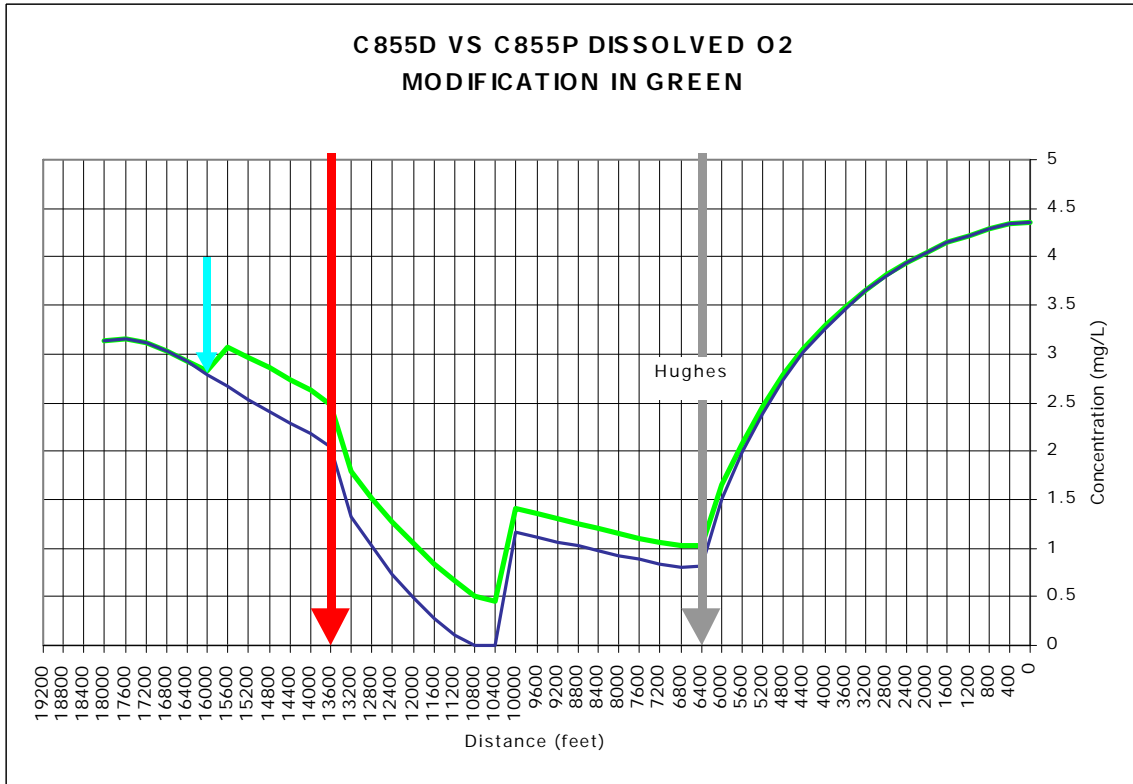


Figure 6.16. DO at Q=855GPM; Flow Augmentation between Ennis and Milby.

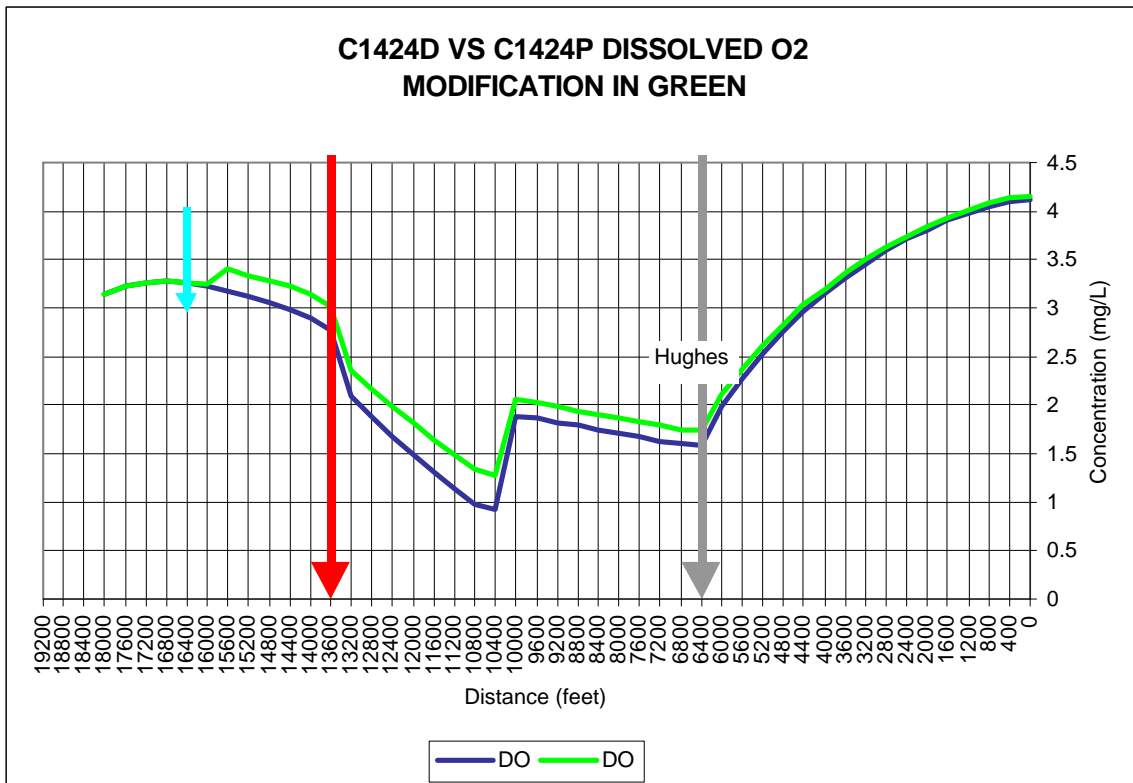


Figure 6.17. DO at Q=1424GPM; Flow Augmentation between Ennis and Milby.

*Source Control (Reduced Input Loads) Simulations*

Source control was simulated as fractional reductions in BOD loads at the point source without changing any other input in the computer model. Only the Q=855 gpm case is presented as the trends for this case would be consistent with trends for each modeled system flow rate.

The simulations used the odor baseline and the no-odor baseline as the two extremes. The no-odor case represents complete source control (100% reduction in input load). Figure 6.18 is a plot of these two extremes with two intermediate cases (50% and 75% reduction).

Assuming continued efforts achieve a 50% reduction in source magnitudes, one expects far more water quality benefit than either channel modification (negligible) or flow augmentation alone.

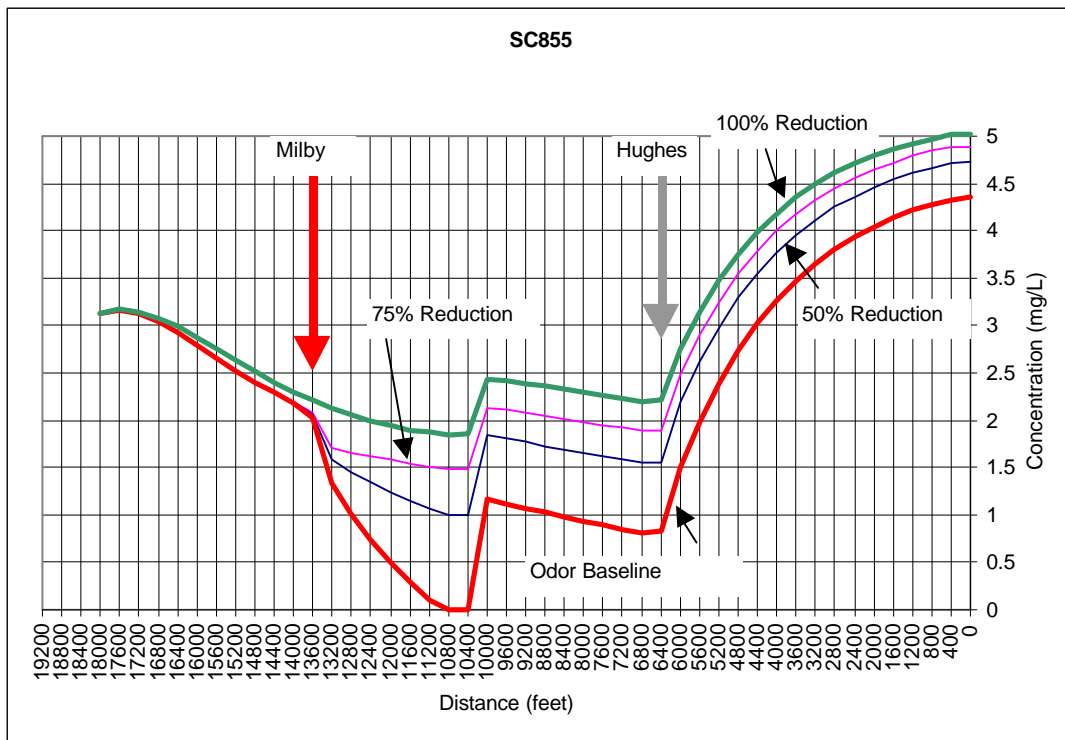


Figure 6.18 DO at Q=855 gpm; Various source load reductions.

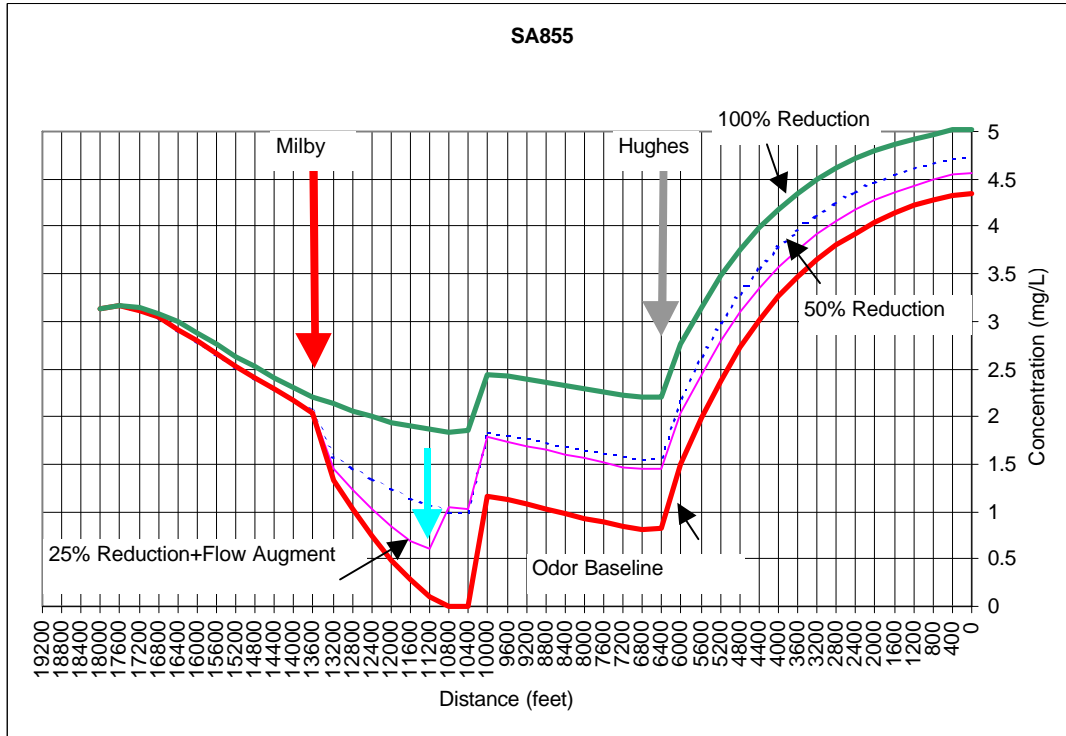


Figure 6.19 DO at Q=855 gpm; 25% Reduction in Combination with 64 gpm Flow Augmentation.

A reduction in source magnitude of 50% using field monitoring and enforcement may not be realistic.

An second set of simulations studied the effect if only 25% source control is achieved in combination with flow augmentation. Figure 6.19 plots the simulated water quality in this scenario. The result is that flow augmentation in combination with 25% source reduction can achieve nearly the same effect as 50% source reduction.

*Re-aeration (Modify Re-aeration Coefficient in Covered Portion) Simulations*

This strategy is modeled assuming that some efforts to improve mixing in the covered portion between the Altic St. junction box and the outfall, increases the re-aeration in this reach. The increased re-aeration is modeled by assuming that the re-aeration coefficient is increased by 10% and 50%, respectively. Figure 6.20 is a plot comparing the Q=855 gpm baseline case and the increased re-aeration case.

The 10% increase in the Hughes Facility section improves the water quality at the outfall while the 50% increase produces a dramatic increase. In both cases although the DO in the water is predicted to be higher, the BOD loading is unchanged and the sediment oxygen demand is the same so that downstream of the outfall the conditions approach the baseline conditions.

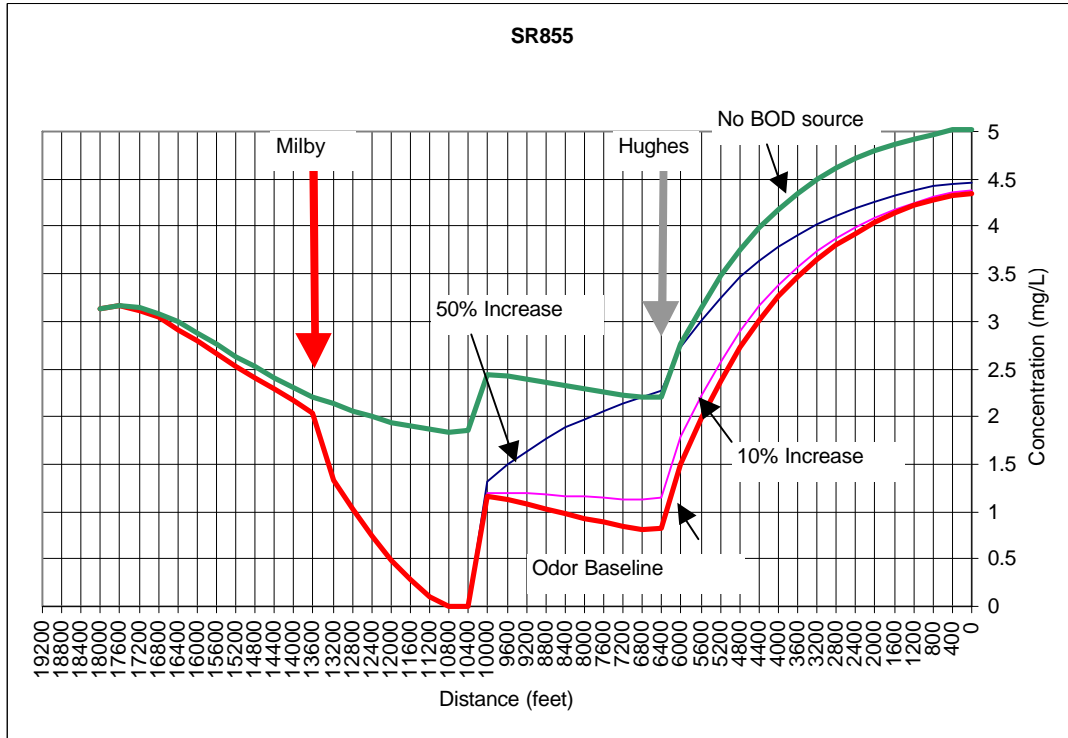


Figure 6.20 DO at Q=855 gpm; Increased re-aeration is section upstream of Hughes.

*Summary*

The computer model was used to evaluate the feasibility of several intervention strategies that could be applied on Country Club Bayou. Table 6.6 lists the predicted values of DO at the Hughes Street Outfall for the different intervention strategies.

Table 6.6 Water quality predictions for various strategies.

Discharge (gpm)	DO(mg/L) at Hughes									
	Base Case	Channel Modification	Flow Augment at Altic	Flow Augment at Milby	100% Source Reduce	75% Source Reduce	50% Source Reduce	25% Source Reduce + Augment at Alt	5% Increase in reaeration	50% Increase in reaeration
370	0.54	0.21	0.93	0.7						
520	0.96	0.89	1.3	1.11						
855	0.82	0.92	1.12	1.02	2.21	1.91	1.56	1.44	1.15	2.28
1424	1.59	1.72	1.77	1.74						
2800	0.77	0.87	1.08	0.98						

Based on the computer predictions the channel modification option only marginally improves water quality (measured as DO at Hughes), and would be costly to implement. Flow augmentation at either location provides a good improvement in water quality at all flows simulated. The insensitivity of location means that many different hydrants can be identified if this strategy is implemented.

Source control is predicted to have the most improvement, however it is not possible to predict how much source control can be achieved by vigorous monitoring and enforcement. A combination of a lower degree of source control and flow augmentation can produce results similar to a more rigorous source control alone.

Artificial addition of air into the system under the Hughes facility can also achieve good results, however it is not possible to predict what a reasonable estimate of aeration increase can be achieved using the computational tools in this research.



## 7. Conclusions and Recommendations

### *Conclusions*

This research tested strategies and evaluated potential effectiveness in improving water quality on Country Club Bayou. Pollution of the bayou has been problematic for at least a dozen years. Currently suspected high organic loading in the upstream covered portion of the bayou contributes to observed low dissolved oxygen values, septic odor conditions, and septic (black) color in the bayou water. Attempts at eliminating the sources of organic loading to the bayou have not produced an obvious increase in water quality. Despite repair of numerous sewage leaks and reductions in industrial discharges, septic odor and low dissolved oxygen conditions continue to exist. Investigation of other options for improving water quality became necessary, thus providing impetus for this project.

The investigation included field monitoring of selected water quality parameters, a series of dye tracer studies, and a computer simulation of water quality to evaluate possible intervention strategies.

The field-monitoring program was conducted to collect water quality data that could be used to calibrate a computer model of the bayou and to develop a database that could be used to interpret the relative health of the bayou, and any quantifiable cause-effect relationships.

The hydraulic model was difficult to calibrate because simulated channel slopes much smaller than expected were needed to fit the observed flow depths. The presence of debris in the covered portion underneath the Hughes facility is documented in video surveys. The presence of these “sand bars” contributes to the poor flow conditions beneath the Hughes facility and cleaning of this section of storm sewer should be considered.

When septic odor conditions are prevalent, DO and BOD levels are significantly different than during non-odor conditions. The elevated BOD indicates that some source (commercial wash water, industrial discharge, etc.) supplies an additional organic load to the bayou that in-turn depresses the DO. Odor likely results when this mixture sits relatively stagnant under the Hughes facility. The sediment in this covered reach exerts an oxygen demand as well and because the water is moving slowly, vertical mixing does not occur. The sediment is postulated to become anaerobic and sulfate reducing bacteria in this zone produce sulfide that contributes to the odor.

The mean values of DO and sulfate meets existing or proposed state water quality standards for an unclassified stream. The fecal coliform (FC) values do not. Only about 25% of the FC values measured in this research meet the current standard (2000 cfu/100mL).

Based on a series of tracer studies the travel time in the covered portion is on the order of several hours upstream of the junction box at Altic Street. The covered portion from Altic St. to Hughes has a remarkably long travel time for the distances involved (on the order of one day). The large width of this section of bayou (box culverts) allows relatively small changes in depth to store large volumes of flow and thus upstream flow changes are passed through this section undetected. Even the highest flow event measured during this study had a travel time through this portion of the system on the order of one day. This stagnant water in the section between Altic and Hughes contributes to odor conditions.

A *change* in water quality occurs between Evergreen Cemetery and Hughes Street. Between these two locations the DO declines, the ammonia increases, and the BOD declines. The BOD decline is diagnostic because it suggests that the bayou has assimilative capacity and that there is either no source between these two location or there is significant dilution by some unknown source of water. These changes are greater between these two locations than elsewhere in this study, and this section of bayou corresponds with the stagnant section just described.

A computer model of the water quality of Country Club Bayou was developed to predict the effect of selected intervention strategies developed over the course of the research by the research partners. Several “brainstorming” meetings developed six plausible intervention strategies. Table 7.1 is a list of these six strategies with notations on the author’s perceived complexity, estimated cost, and reliability.

Table 7.1 Intervention Strategies for Country Club Bayou

Strategy	Complexity	Cost <sup>1</sup>	Reliability	Modeled
Channel modification	Complex	\$500,000 <sup>a</sup>	Low	Yes
Mechanical/chemical aeration	Complex	High	Unknown	Yes
Constructed wetland	Complex	Unknown	Unknown	No
Flow augmentation	Simple	\$185,000 <sup>b</sup>	High	Yes
Divert low flow to treatment	Simple	\$350,000 <sup>c</sup>	High	No
Source control	Moderate	\$250,000 <sup>d</sup>	Moderate	Indirectly

<sup>1</sup>Cost estimate basis: Costs are totals for five years of service.

a) Channel modification: 0.5 miles of underground construction. Assume cost is 1 million dollars/mile. Cost of channel modification is estimated to be at least \$500,000, excluding maintenance.

b) Flow augmentation: Assume water for augmentation costs \$2.00/1000 gallons. Then the estimated cost for five years of continuous augmentation is about \$185,000.

c) Diversion to treatment: Assume installation of a lift station to pump 1000 gpm into the sanitary system is \$100,000. Assume diversion operating costs are \$0.10/1000 gallons. Estimated cost for five years is \$350,000.

d) Source control: Assume that the University expenditures represent one-half the costs that would be incurred by the City of Houston (monitoring, enforcement, follow-up) The estimated annual cost to maintain the level of effort at that of September 1999 is \$50,000/year.

Based on the model’s predictions, the channel modification option only marginally improves water quality (measured as DO at Hughes), and would be costly to implement. Flow augmentation at either location provides improvement in water quality at all flows simulated. The insensitivity of location means that many different hydrants can be

identified if this strategy is implemented. Figure 7.1 identifies several possible locations along the bayou alignment (not shown) that could be used for flow augmentation.

It is difficult to predict how much improvement in water quality can be achieved by source control. Vigorous monitoring and enforcement have achieved limited success in the past. A combination of a lower degree of source control and flow augmentation can produce results similar to a more rigorous source control alone. Nevertheless, continued source control will reduce organic loading to the bayou and is recommended.

Source control by the methods assumed in this research (monitoring, enforcement, and follow-up) is unlikely to be able to achieve a high-percentage source reduction. If structural methods were employed (packing undocumented lateral connections to the storm sewer) a higher reduction could be achieved. However, even with structural controls, there will still be inputs to the storm system containing pollutants, some of these intentional because of archaic or ignorant business practices. These remaining inputs will still require some level of monitoring and enforcement to control.

Artificial addition of air into the system under the Hughes facility may also improve water quality, however it is not possible to accurately predict the improvement using the computational tools in this research. The approach used in this work was to adjust a re-aeration parameter, which is, at best, a crude surrogate for actual simulation of artificial addition of air.



Figure 7.1 GIMS map of possible hydrant locations for flow augmentation.  
(Blue lines are approximate alignment of Bayou)

Flow augmentation is predicted to be effective at relatively small volumes (70 gpm). Figure 7.1 is a map of candidate hydrants for flow augmentation near the downstream portion of the covered section. The computer program is a steady-state program so the duration of augmentation on an as-needed basis cannot be evaluated. Based on the travel

time study, one could expect that the bayou would respond to augmentation within a day, and during the augmentation period, an effort to locate and eliminate the pollutant source could be employed. Thus, augmentation may only be needed for symptomatic treatment only a few days during any particular odor episode.

*Recommendations*

Routine monitoring continued enforcement (source control), flow augmentations, and cleaning of portions of the bayou are recommended for long-term management of water quality on Country Club Bayou.

Routine water quality monitoring should be conducted monthly at Evergreen Cemetery, Hughes Street Bridge, and at Wayside Drive. Department of Health and Human Services (DHHS) personnel could conduct this monitoring because DHHS has the authority to conduct enforcement activities and currently is a responsible agency for water quality monitoring in the City of Houston.

The water should be analyzed in the field for temperature, pH, dissolved oxygen, ammonia and sulfide. The last three parameters require special equipment, but the equipment is rugged and reasonably inexpensive. Water samples should be collected and analyzed for CBOD and Fecal Coliform. Depressed DO or elevated sulfide or elevated ammonia should be investigated immediately to identify possible sources. Table 7.2 lists the water quality measures and alert values based on data collected in this research. The alert values represent the mean values from data collected during odor episodes, or estimates of values indicative of odor conditions. If alert values are exceeded, flow augmentation and source control procedures should be implemented as described below.

Table 7.2 Water Quality Alert Values

Measurement	Value	Recommended action
Dissolved Oxygen <sup>1</sup>	<1.0 mg/L	search for pollutant source; consider flow augmentation
Ammonia <sup>1</sup>	>1.5 mg/L	search for pollutant source
Sulfide <sup>1</sup>	>0.15 mg/L	search for pollutant source; rotten egg odor should be present; bottom sediment should be black; consider flow augmentation.
Spheratolis (filaments) <sup>1</sup>	visible	search for pollutant (sanitary) source
Fecal Coliform	>200 <sup>2</sup> >10,000 <sup>3</sup>	begin search for sanitary source; industrial source possible.
CBOD	>15 mg/L <sup>3</sup>	begin search of pollutant source

<sup>1</sup>Field measurement; assumes instrument(s) available

<sup>2</sup>State value

<sup>3</sup>Practical value based on data collected in this project.

Because CBOD and Fecal Coliform require laboratory analysis, immediate investigation is not possible. Elevated CBOD and Fecal Coliform should be investigated in a follow-up visit to attempt to identify possible sources of these pollutant indicators.

Non-routine monitoring in response to complaints should collect these same data and consider the same response procedures. Non-routine monitoring events should include samples at the routine monitoring locations. These extra samples can be analyzed to develop updated alert values for triggering an aggressive investigation.

Current enforcement activities should continue. These activities include education of the various industries in the watershed that may engage in practices that carry wash water and process water to the storm sewer rather than to the sanitary collection system. Past examples of known negligent business practices in the Country Club Bayou watershed include: ice melts from a fish processor, wash water from a dairy; wash water from a bulk sweetener processor, wash water from a commercial bus operator, and sanitary sewer leaks. When the DHHS monitoring personnel identify a source they can immediately contact the owner/operator in an attempt to remove the source. If the source is a sanitary system excursion, then the Customer Response Center (CRC) (713-837-0600) should be contacted so that the excursion can be repaired. Follow-up of identified sources is required to confirm that the sources are removed and excursions repaired. In the event sources are not removed, the DHHS can follow existing protocol to use citations and other administrative tools to achieve compliance.

Flow augmentation supplements this monitoring and enforcement effort by providing a tool to address low water quality by a short-term intervention. When a contaminant source is identified that has degraded the water quality, an augmentation release can be used to temporarily improve the water quality while the source is being eliminated. When flow augmentation is used, the water should be released near the location where the alert level water quality parameter was detected. The release amount and duration should follow the routine release protocol below. The bayou segment between the release location and the alert location should be inspected to be sure that there is not a pollutant source between the release point and the sampling point, otherwise the source will be undetectable. Investigation should proceed upstream of the release point until the source can be located, or the source appears to have stopped.

In addition to symptomatic flow augmentation releases, a scheduled approach is recommended to release augmentation water after a prescribed period without rainfall to serve as an artificial “storm” event. This “artificial storm” will keep the bayou water refreshed with high quality water regardless of the current water quality of the bayou. The actual waiting period and release duration would best be determined by trial and error. A suggested protocol to start is listed.

1. Release rate: 70 gpm
2. Duration: 3 days, maximum
3. Frequency: every 15 days without measurable rainfall



4. Monitoring: DO at Hughes, should observe increase after 24 hours.

This protocol should be adjusted by trial and error to determine the most effective release frequency and duration. For example, more frequent, higher flow-rate, or longer duration releases should be attempted if the suggested release protocol results in continued water quality and odor problems. Less frequent, lower flow-rate, or shorter duration releases could be tested if the suggested approach is successful. The release frequency may vary seasonally. If trial-and-error indicate continuous release is necessary, then more aggressive source control is indicated.

One final recommendation is related to the physical structure of the drainage system itself. A cleaning of the storm sewer between the Altic St. Junction Box and Hughes Street bridge is recommended to remove the blockages to flow that may contribute to stagnation in this area.

#### *Implementation Funding*

Currently funding sources appear to exist for some of the recommendations, including the following:

- Investigations by DHHS (ongoing)
- Routine water quality monitoring (DHHS through agreement with HGAC)
- Investigation and repair of sanitary sewer excursions (ongoing via City of Houston maintenance quadrants)

No funding sources are identified for flow augmentation or for cleaning the storm sewer between Altic Street and Hughes Street. None of the parties in this research project including the City of Houston's Wastewater Operations Division, The DHHS, and the TNRCC demonstrate obvious responsibility for funding these recommendations. A cooperative approach will be necessary for any funding applied to address Country Club Bayou's long term water quality management.

An alternative that appears to be gaining favor in recent regulatory documents is the watershed approach to water quality. The complexity of the responsibilities within the City's current structure for even this small area lends some credence to the watershed concept where the entire services are integrated instead of separated along historical disciplinary lines. An integrated service could probably charge an assessment to fund water quality management based on past and current expenditures within the watershed to address water quality problems. This assessment would create an economic incentive to reduce source contributions to lower the assessment rate as the magnitude and frequency of low water quality events is reduced. Non-government organizations could be funded to provide the educational services to reduce the source loads and ultimately reduce the need for the assessment. Such arrangements are beyond the scope of this research and the author's expertise but could be considered in planning the implementation.

## 8. References

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## Appendix – I Discharge Measurements in Open Portion of Country Club Bayou.

The measurement of discharge is accomplished by measuring flow velocity near the surface of the water, and measurement or estimation of flow depth and flow width. Simple channel geometry is assumed for making discharge calculations.

A correction factor is applied to the velocity measurement to approximate the mean section velocity. The correction factor was determined by graphical analysis of the velocity distributions for the trapezoidal and the shallow ditch in Figure A.1 below.

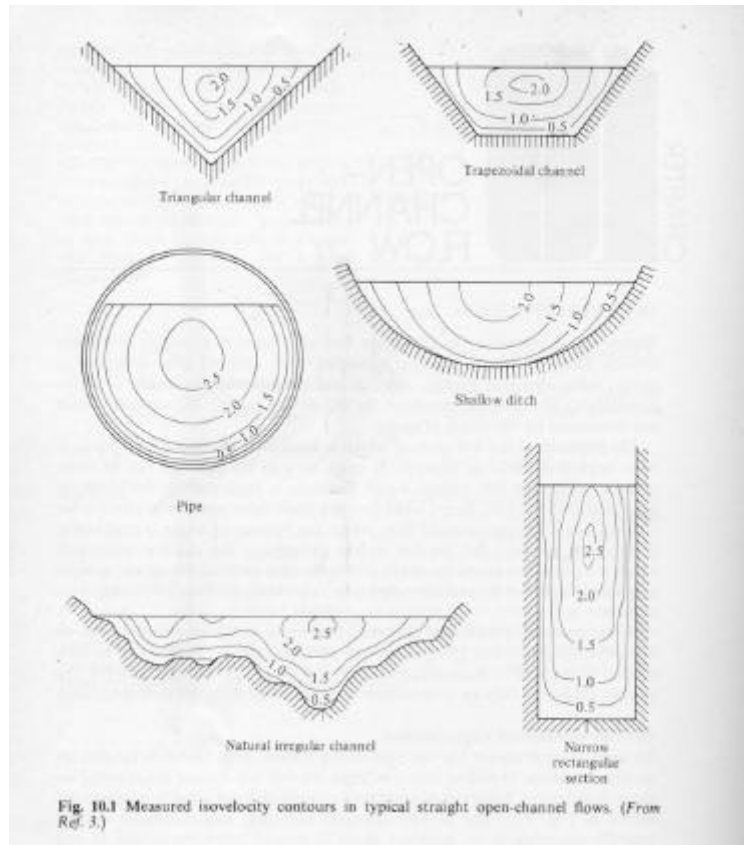


Fig. 10.1 Measured isovelocity contours in typical straight open-channel flows. (From Ref. 3.)

Figure A.1 Velocity Distributions in Open Channels  
(from F.M. White, 1979. Fluid Mechanics, McGraw Hill, New York p598)

The graphical analysis is performed as follows. The velocity distribution in Figure A.1 was transferred to graph paper and the areas between the iso-velocity contours were determined by counting the number of squares in the inter-contour panels. The product of the lower iso-velocity value and the panel area represents the proportion of discharge in the channel contributed by the panel. These discharges are summed for all the panels, then the ratio of this sum and the total area of all panels represents the equivalent mean section velocity. The ratio of this mean velocity and the maximum velocity near the centerline of flow is the correction factor used to convert maximum (measured) velocity to mean section velocity for computation of discharge from the field data. Figures A.2

and A.3 show this graphical approach applied to a trapezoidal and a shallow ditch section. The other sections were not analyzed because they are not relevant to application on Country Club Bayou.

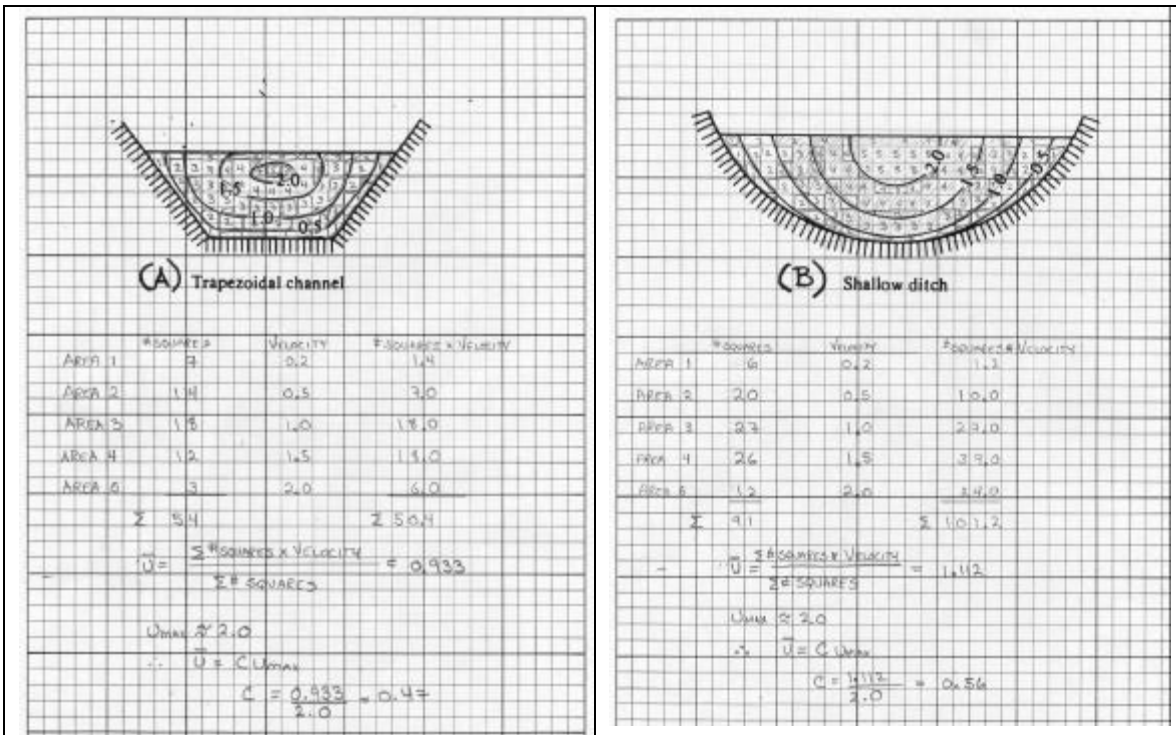


Figure N.2 Graphical Analysis for a Trapezoidal Channel Section.

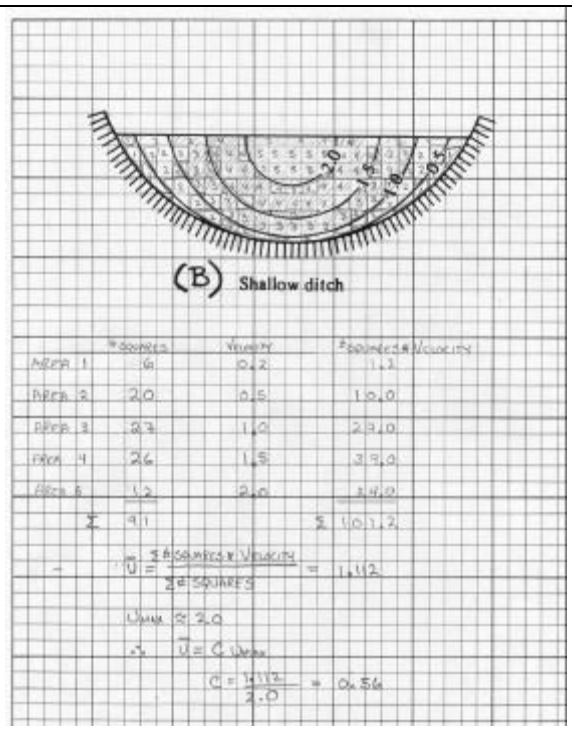


Figure N.3 Graphical Analysis for a Shallow Ditch Section.

The correction factors from these two sections range from 0.47 to 0.56. The value selected based on analyst judgement for the Country Club Bayou flow measurements is 0.52. This correction factor allows a single velocity measurement at the fastest portion of the flow field to provide an estimate of total section discharge.

Using this correction factor, the procedure to measure flow in Country Club Bayou is the following:

- 1) Determine flow section depth and width. Record these values and the associated units (feet, inches, etc.) The simplest geometry is rectangular, and all calculations in the field notebook assumed this section geometry. The product of these two measurements is the flow area,  $A$ .  $A = d \times w$
- 2) Measure velocity in the center of the flow field by either the velocity meter or using a drift tracer. Record the measurement as feet per seconds (e.g. using a three foot floating ruler and a drift tracer, one might record 3 feet / 14 seconds - the conversion to feet per second can be done in the office).

3) Compute discharge as the product of velocity, depth, and width. This computation will produce a flow rate in cubic feet per second.  $Q = U \times A = U \times d \times w$

4) Multiply the discharge by the correction factor.  $Q_c = C \times Q = 0.52 \times Q$

5) Convert the corrected discharge to gallons per minute.

$$Q_{gpm} = Q_{c-cfs} \times \frac{7.48 \text{ gal}}{\text{cu. ft.}} \times \frac{60 \text{ sec}}{\text{min}}$$

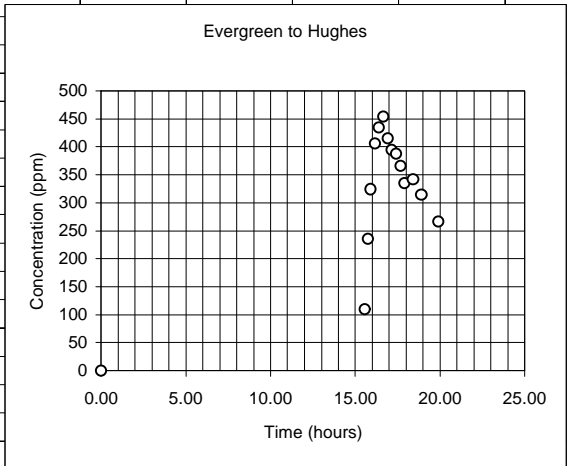
## Appendix II Tracer Studies

### Part 1: Data for various tracer studies

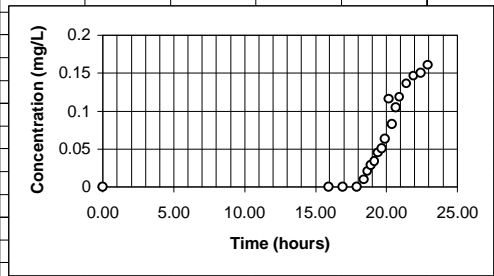
	A	B	C	D	E	F	G	H	I	J	K						
1	Tracer Study #1 North Vents to Hughes Street																
2	Date:																
3	Time (minutes)	SampleID	Reading	Concentration (mg/L)													
4	0.0001	H Back	0	0													
5	67	H 1057	0	0													
6	126	H 1156	1	0.0002													
7	156	H 1226	0	0													
8	217	H 1327	0	0													
9	249	H 1359	0	0													
10	306	H 1456	0	0													
11	365	H 1555	1	0.0002													
12	425	H 1655	0	0													
13	461	H 1731	1	0.0002													
14	491	H 1801	0	0													
15	505	H 1815	0	0													
16	520	H 1830	1	0.0002													
17	535	H 1845	0	0													
18	550	H 1900	0	0													
19	1220	H 5/7 0610	1	0.0002													
20	1235	H 0625	0	0													
21	1255	H 0645	1	0.0002													
22	1270	H 0700	0	0													
23	1285	H 0715	1	0.0002													
24	1300	H 0730	0	0													
25	1315	H 0745	0	0													
26	1330	H 0800	1	0.0002													
27	1360	H 0830	3	0.0006													
28	1390	H 0900	7	0.0014													
29	1420	H 0930	18	0.0036													
30	1450	H 1000	32	0.0064													
31	1735	H 1445	40	0.008													
32	1745	H 1450	33	0.0066													
33	1750	H 1455	33	0.0066													
34	1755	H 1500	34	0.0068													
35	1760	H 1505	29	0.0058													
36	1765	H 1510	23	0.0046													
37	1770	H 1515	30	0.006													
38	1775	H 1520	24	0.0048													
39																	



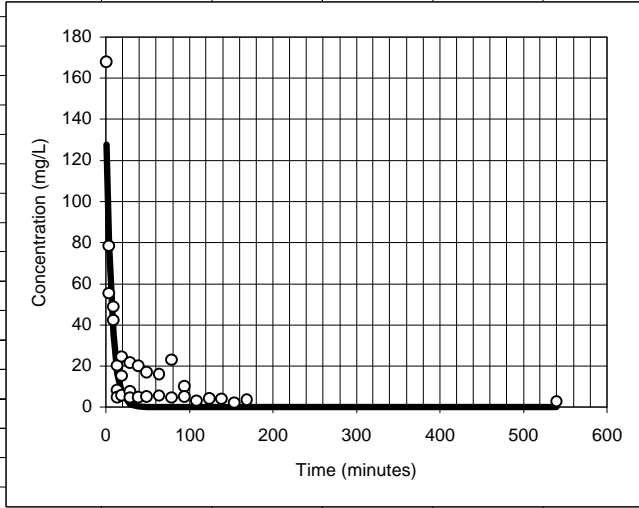
	A	B	C	D	E	F	G	H	I	J
1	Tracer Study #2 Evergreen to Hughes Street									
2	Date:									
3	Time	time (hrs)	reading	Conc (ppm)						
4	8/16/99 16:05	0.00	0	0.0000						
5	8/17/99 7:40	15.58	109	0.0523						
6	8/17/99 7:50	15.75	235	0.1128						
7	8/17/99 8:00	15.92	324	0.1555						
8	8/17/99 8:15	16.17	406	0.1949						
9	8/17/99 8:30	16.42	435	0.2088						
10	8/17/99 8:45	16.67	454	0.2179						
11	8/17/99 9:00	16.92	415	0.1992						
12	8/17/99 9:15	17.17	394	0.1891						
13	8/17/99 9:30	17.42	387	0.1858						
14	8/17/99 9:45	17.67	366	0.1757						
15	8/17/99 10:00	17.92	335	0.1608						
16	8/17/99 10:30	18.42	342	0.1642						
17	8/17/99 11:00	18.92	314	0.1507						
18	8/17/99 12:00	19.92	266	0.1277						
19										
20	blank	0.00								
21	0.48ppm std	1000.00								
22										



	A	B	C	D	E	F	G	H	I	J	K	L
1	Tracer Study #3 Polk and Elm to Hughes Street						Discharge at Hughes					
2	Date:	8/19/99										
3	Time	reading	Sample ID	Time	conc (ppm)	Time	v	w	d	Q (gpm-un	Q (gpm)	
4	8/19/99 16:05	0		0.00	0	800	5.88	3.1	0.54	4417.592	2297.148	
5	8/20/99 8:00	0		15.92	0	900	5.88	3.1	0.56	4581.207	2382.228	
6	8/20/99 9:00	0		16.92	0	1000	6.07	3.1	0.58	4898.14	2547.033	
7	8/20/99 10:00	0		17.92	0	1100	6.03	3.1	0.62	5201.439	2704.748	
8	8/20/99 10:30	20		18.42	0.0096	1200	6.48	3.1	0.64	5769.916	3000.357	
9	8/20/99 10:45	43		18.67	0.02064	1300	6.13	3.14	0.66	5701.471	2964.765	
10	8/20/99 11:00	59		18.92	0.02832							
11	8/20/99 11:15	70		19.17	0.0336							
12	8/20/99 11:30	94		19.42	0.04512							
13	8/20/99 11:45	106		19.67	0.05088							
14	8/20/99 12:00	132		19.92	0.06336							
15	8/20/99 12:15	242		20.17	0.11616							
16	8/20/99 12:30	172		20.42	0.08256							
17	8/20/99 12:45	218		20.67	0.10464							
18	8/20/99 13:00	247		20.92	0.11856							
19	8/20/99 13:30	284		21.42	0.13632							
20	8/20/99 14:00	305		21.92	0.1464							
21	8/20/99 14:30	313		22.42	0.15024							
22	8/20/99 15:00	335		22.92	0.1608							
23												



	A	B	C	D	E	F	G	H	I	J
1	Tracer Study #4 Dilution at Ennis and Lamar									
2	Date:									
3	Co	150 mg/L								
4	U	0.16 ft/min		Q (cfm)	0.32 Q(gpm)		2.3936			
5	Depth	0.5 ft								
6	Width	4 ft								
7		Time	C(mg/L)	C(model)						
8	1056	1	167.8795	127.8216						
9	1100	4	55.36413	79.09386						
10	1100	4	78.31362	79.09386						
11	1105	9	48.85116	35.53916						
12	1105	9	42.2828	35.53916						
13	1110	14	20.17437	15.96878						
14	1110	14	8.106345	15.96878						
15	1110	14	4.711465	15.96878						
16	1115	19	24.49553	7.175233						
17	1115	19	15.17734	7.175233						
18	1115	19	5.603195	7.175233						
19	1125	29	21.65771	1.448655						
20	1125	29	7.625067	1.448655						
21	1125	29	4.540498	1.448655						
22	1135	39	19.96782	0.292478						
23	1135	39	4.876759	0.292478						
24	1145	49	16.96126	0.05905						
25	1145	49	5.09434	0.05905						
26	1200	64	16.05137	0.005357						
27	1200	64	5.553237	0.005357						
28	1215	79	23.01395	0.000486						
29	1215	79	4.560278	0.000486						
30	1230	94	9.931295	4.41E-05						
31	1230	94	5.093933	4.41E-05						
32	1245	109	3.047482	4E-06						
33	1300	124	4.232686	3.63E-07						
34	1315	139	3.873174	3.29E-08						
35	1330	154	2.075615	2.99E-09						
36	1345	169	3.586728	2.71E-10						
37	1956	540	2.785625	4.5E-36						



Visual tracer tests

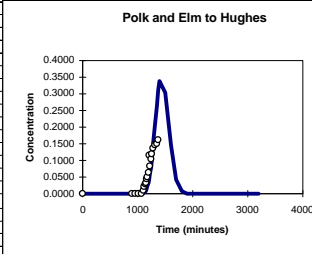
Release	Recovery	Time	
Hughes - 07:15 u=4.03 ft/sec W=2.9 ft D=0.22 ft	Polk- 12:00	4.75 hours	Visual only
Ennis Same as study#4	N2	~7 hours	Visual only

## Part 2. Interpretation Calculations

	A	B	C	D	E	F	G	H	I	J	K	L	M	N	O	P	Q	R			
1	Country Club Bayou Tracer Test Model																				
2	Advection and Dispersion																				
3	North Vent to Hughes Street Bridge																				
4	Tracer mass 20 grams																				
5	Depth 2.5 ft																				
6	Width 13 ft																				
7	Length 1 ft																				
8	Labeled Volume 0.94674556 cu.meters.																				
9	Label Concentration 21.125 mg/L																				
10	Transport Parameters																				
11	Co 21.125 Tracer Concentration (mg/liter)																				
12	U 1.41 Fluid Velocity (feet/min)																				
13	L 2 Mixing Length (feet)																				
14	D 2.82 Dispersion Coefficient (ft <sup>2</sup> /min)																				
15	t 1 Pulse Length (min)																				
16	Computed Constants																				
17	y/D 0.5																				
18																					
19																					
20																					
21																					
22																					
23																					
24	Intermediate Calculations																				
25	Simulation Parameters																				
26	(1)	(2)	(3)	(4)	(5)	(6)	(7)	(1)	(2)	(3)	(4)	(5)	(6)								
27	z (feet)	C(x,t) Field Data	v <sub>vt</sub>	v <sub>vt</sub>	SD <sub>vt</sub>	ERFCCJ (1)/(3)	ERFCCJ (2)/(3)	EXP <sub>vt</sub> (x,v,D)	C(x,t) (mg/m <sup>3</sup> )	(meters)	L (years)	v <sub>vt</sub>	v <sub>vt</sub>	SD <sub>vt</sub>	#D1/D0	#D1/D0	ERFCCJ (1)/(3)	ERFCCJ (2)/(3)	EXP <sub>vt</sub> (x,v,D)		
28	2250	0.0001	0	2249.99986	2250.00014	0.03358571	0	0	2.6881E+43	0	2250	0	2250	0	2250	2156.94	2343.06	27.2851608	0	0	2.6881E+43
29	2250	67	0	2155.53	2344.47	27.4910895	0	0	2.6881E+43	0	2250	66	2156.94	2343.06	27.2851608	0	0	2.6881E+43	0	0	2.6881E+43
30	2250	126	0.0002	2072.34	2427.66	37.6998674	0	0	2.6881E+43	0	2250	125	2073.75	2426.25	37.5499667	0	0	2.6881E+43	0	0	2.6881E+43
31	2250	156	0	2030.04	2469.96	41.9485399	0	0	2.6881E+43	0	2250	155	2031.45	2468.55	41.8138733	0	0	2.6881E+43	0	0	2.6881E+43
32	2250	217	0	1944.03	2555.97	48.4748421	0	0	2.6881E+43	0	2250	216	1945.44	2554.56	48.3307131	0	0	2.6881E+43	0	0	2.6881E+43
33	2250	249	0	1898.91	2601.09	52.9073554	0	0	2.6881E+43	0	2250	248	1900.32	2599.68	52.800831	0	0	2.6881E+43	0	0	2.6881E+43
34	2250	306	0	1818.54	2681.46	58.751	0	0	2.6881E+43	0	2250	305	1819.95	2680.05	58.6549231	0	0	2.6881E+43	0	0	2.6881E+43
35	2250	365	0.0002	1735.35	2764.65	64.1654112	0	0	2.6881E+43	0	2250	364	1736.76	2763.24	64.0774531	0	0	2.6881E+43	0	0	2.6881E+43
36	2250	425	0	1650.75	2849.25	69.2387175	0	0	2.6881E+43	0	2250	424	1652.16	2847.84	69.1572122	0	0	2.6881E+43	0	0	2.6881E+43
37	2250	461	0.0002	1590.99	2930.01	73.115802	0	0	2.6881E+43	0	2250	460	1601.44	2898.56	72.0333258	0	0	2.6881E+43	0	0	2.6881E+43
38	2250	491	0	1557.69	2942.31	74.4209648	0	0	2.6881E+43	0	2250	490	1559.1	2940.9	74.3451141	0	0	2.6881E+43	0	0	2.6881E+43
39	2250	505	0	1537.95	2962.05	75.474499	0	0	2.6881E+43	0	2250	504	1539.36	2960.64	75.3997347	0	0	2.6881E+43	0	0	2.6881E+43
40	2250	520	0.0002	1516.68	2983.2	76.5872052	0	0	2.6881E+43	0	2250	519	1518.21	2981.79	76.5135282	0	0	2.6881E+43	0	0	2.6881E+43
41	2250	535	0	1495.65	3004.35	77.6839757	0	0	2.6881E+43	0	2250	534	1497.06	3002.94	77.6113394	0	0	2.6881E+43	0	0	2.6881E+43
42	2250	569	0	1474.5	3025.5	78.7854797	0	0	2.6881E+43	0	2250	568	1475.91	3024.09	78.6838371	0	0	2.6881E+43	0	0	2.6881E+43
43	2250	580	0	1432.2	3067.8	80.8851037	0	0	2.6881E+43	0	2250	579	1433.61	3066.39	80.8153451	0	0	2.6881E+43	0	0	2.6881E+43
44	2250	625	0	1368.75	3131.25	83.9642781	0	0	2.6881E+43	0	2250	624	1370.16	3129.84	83.8970798	0	0	2.6881E+43	0	0	2.6881E+43
45	2250	670	0	1305.3	3194.7	86.9344581	0	0	2.6881E+43	0	2250	669	1306.71	3193.29	86.8695574	0	0	2.6881E+43	0	0	2.6881E+43
46	2250	730	0	1220.7	3279.3	90.7435948	0	0	2.6881E+43	0	2250	729	1222.11	3277.89	90.6814204	0	0	2.6881E+43	0	0	2.6881E+43
47	2250	790	0	1136.1	3363.9	94.3991525	0	0	2.6881E+43	0	2250	789	1137.51	3362.49	94.3393873	0	0	2.6881E+43	0	0	2.6881E+43
48	2250	850	0	1051.5	3448.5	97.9183333	0	0	2.6881E+43	0	2250	849	1052.91	3447.09	97.8607173	0	0	2.6881E+43	0	0	2.6881E+43
49	2250	910	0	966.9	3533.1	101.315349	0	0	2.6881E+43	0	2250	909	968.31	3531.69	101.259666	0	0	2.6881E+43	0	0	2.6881E+43
50	2250	970	0	882.3	3617.7	104.602103	0	0	2.6881E+43	0	2250	969	883.71	3616.29	104.548171	0	0	2.6881E+43	0	0	2.6881E+43
51	2250	1015	0	818.85	3681.15	107.000935	0	0	2.6881E+43	0	2250	1014	820.26	3679.74	106.948212	0	0	2.6881E+43	0	0	2.6881E+43
52	2250	1230	0.0002	529.8	3970.2	117.308946	1.6924E-10	0	2.6881E+43	2.16E-10	2250	1219	531.21	3968.79	117.261758	1.4884E-10	0	0	2.6881E+43		
53	2250	1235	0	508.65	3991.35	118.02881	1.097E-09	0	2.6881E+43	1.32E-09	2250	1234	510.06	3989.94	117.981015	9.7175E-10	0	0	2.6881E+43		
54	2250	1255	0.0002	480.45	4019.55	118.980671	1.1253E-08	0	2.6881E+43	1.26E-08	2250	1254	481.86	4018.14	118.93259	1.006E-08	0	0	2.6881E+43		
55	2250	1270	0	459.3	4040.7	119.689599	5.7328E-08	0	2.6881E+43	6.06E-08	2250	1269	460.71	4039.29	119.642467	5.1589E-08	0	0	2.6881E+43		
56	2250	1285	0.0002	438.15	4061.85	120.394352	2.6505E-07	0	2.6881E+43	2.64E-07	2250	1284	439.56	4060.44	120.347497	2.4005E-07	0	0	2.6881E+43		
57	2250	1300	0	417	4083	121.085004	1.1162E-06	0	2.6881E+43	1.05E-06	2250	1299	418.41	4081.59	121.04842	1.0171E-06	0	0	2.6881E+43		
58	2250	1315	0	395.85	4104.15	121.791255	4.2963E-06	0	2.6881E+43	3.79E-06	2250	1314	397.26	4102.74	121.745308	3.9378E-06	0	0	2.6881E+43		
59	2250	1330	0.0002	374.7	4125.3	122.484285	1.5162E-05	0	2.6881E+43	1.25E-05	2250	1329	376.11	4123.89	122.438229	1.3976E-05	0	0	2.6881E+43		
60	2250	1360	0.0006	332.4	4167.6	123.857983	0.00014743	0	2.6881E+43	0.000106	2250	1359	333.81	4166.19	123.812439	0.00013737	0	0	2.6881E+43		
61	2250	1390	0.0014	290.1	4209.9	125.216612	0.00105129	0	2.6881E+43	0.000654	2250	1389	291.51	4208.49	125.171562	0.00098935	0	0	2.6881E+43		
62	2250	1420	0.0036	247.8	4252.2	126.560657	0.00362346	0	2.6881E+43	0.002381	2250	1419	249.21	4250.79	126.160886	0.0034119	0	0	2.6881E+43		
63	2250	1450	0.0064	205.5	4294.5	127.890578	0.02306126	0	2.6881E+43	0.010248	2250	1449	206.91	4293.09	127.84647	0.02209101	0	0	2.6881E+43		
64	2250	1464	0	185.76	4314.24	128.506498	0.04092509	0	2.6881E+43	0.016638	2250	1463	187.17	4312.83	128.462602	0.0394991	0	0	2.6881E+43		
65	2250	1478	0	166.02	4333.98	129.11948	0.06900674	0	2.6881E+43	0.02554	2250	1477	167.43	4332.57	129.075792	0.06658874	0	0	2.6881E+43		
66	2250	1492	0	146.28	4353.72	129.729565	0.11079416	0	2.6881E+43	0.037128	2250	1491	147.69	4352.31	129.686083	0.10727908	0	0	2.6881E+43		
67	2250	1506	0	126.54	4373.46	130.338794	0.16397424	0	2.6881E+43	0.051192	2250	1505	127.96	4372.05	130.293518	0.16490368	0	0	2.6881E+43		
68	2250	1520	0	106.8	4393.2	130.941208	0.24871377	0	2.6881E+43	0.067044	2250	1519	108.21	4391.79	130.898128	0.24236642	0	0	2.6881E+43		
69	2250	1534	0	87.06	4412.94	131.542845	0.34928364	0	2.6881E+43	0.083518	2250	1533	88.47	4411.53	131.499962	0.34137663	0	0	2.6881E+43		
70	2250	1548	0	67.32	4432.68	132.141742	0.47123242	0	2.6881E+43	0.099096	2250	1547	68.73	4431.27	132.099054	0.46185051	0	0	2.6881E+43		
71	2250	1562	0	47.58	4452.42	132.737937	0.6122069	0	2.6881E+43	0.112141	2250	1561	48.99	4451.01	132.695441	0.60159004	0	0	2.6881E+43		
72	2250	1576	0	27.84	4472.16	133.331467	0.76777088	0	2.6881E+43	0.121183	2250	1575	29.25	4470.75	133.289159	0.75623792	0	0	2.6881E+43		
73	2250	1590	0	8.1	4491.9	133.922366	0.93183559	0	2.6881E+43	0.125206	2250	1589	9.51	4							

	A	B	C	D	E	F	G	H	I	J	K	L	M	N	O	P	Q	R	
1	Country Club Bayou Tracer Test Model																		
2	Advection and Dispersion																		
3	Evergreen to Hughes																		
4																			
5																			
6	Tracer mass	50	grams																
7	Depth	2.5	ft																
8	Width	13	ft																
9	Length	1	ft																
10	Labeled Volume	0.9467456	cu.meters.																
11	Label Concentration	62.8125	mg/L																
12																			
13	Transport Parameters																		
14	Co	52.8125	Tracer Concentration	(mg/liter)															
15	U	2.7	Fluid Velocity	(feet/min)															
16	a	11	Mixing Length	(feet)															
17	D	29.7	Dispersion Coefficient	(ft <sup>2</sup> /min)															
18	τ	1	Pulse Length	(min)															
19	Computed Constants																		
20	σ <sub>TD</sub>	0.09090909																	
21																			
22																			
23																			
24	Intermediate Calculations																		
25			(1)	(2)	(3)	(4)	(5)	(6)	(7)	Simulation Parameters	(1)	(2)	(3)	(4)	(5)	(6)			
			C(x,t) Field Data	σ <sub>TD</sub> (D)	ERFCC(1/(σ <sub>TD</sub> ))	ERFCC(2/(σ <sub>TD</sub> ))	EXP(-kx/VD)	C(x,t) (mg/m <sup>3</sup> )											
26	750	0.001	0.0000	2749.9973	2750.0027	0.34467376	0	0.26881E+43	0	2750	2750	2750	2750	2750	2750	2750	2750	2750	2750
27	750	10		2723	2777	34.4673759	0	0.26881E+43	0	2750	9	2725.7	2774.3	32.6986238	0	0	0.26881E+43		
28	750	50		2615	2885	77.0713955	0	0.26881E+43	0	2750	49	2617.7	2882.3	76.2967899	0	0	0.26881E+43		
29	750	100		2480	3020	108.985413	0	0.26881E+43	6.30E-09	2750	99	2482.7	3017.3	108.480666	0	0	0.26881E+43		
30	750	200		2210	3290	154.142971	0	0.26881E+43	0	2750	199	2212.7	3287.3	153.756951	0	0	0.26881E+43		
31	750	300		1940	3560	188.785593	0	0.26881E+43	0	2750	299	1942.7	3557.3	188.470687	0	0	0.26881E+43		
32	750	400		1670	3830	217.990625	0	0.26881E+43	0	2750	399	1672.7	3827.3	217.718166	0	0	0.26881E+43		
33	750	500		1400	4100	243.721152	4.4409E-16	0.26881E+43	2.93E-15	2750	499	1402.7	4097.3	243.477309	3.3307E-16	0	0.26881E+43		
34	750	600		1130	4370	266.983146	2.1556E-09	0.26881E+43	6.30E-09	2750	599	1132.7	4367.3	266.763587	1.9147E-09	0	0.26881E+43		
35	750	650		995	4505	277.884868	4.1107E-07	0.26881E+43	9.62E-07	2750	649	997.7	4502.3	277.671028	3.7484E-07	0	0.26881E+43		
36	750	700		860	4640	288.374756	2.4701E-05	0.26881E+43	4.55E-05	2750	699	862.7	4637.3	288.168701	2.208E-05	0	0.26881E+43		
37	750	750		725	4775	298.496231	0.00059277	0.26881E+43	0.00085	2750	749	727.7	4772.3	298.297167	0.0005906	0	0.26881E+43		
38	750	800		590	4910	308.285582	0.00679892	0.26881E+43	0.007473	2750	799	592.7	4907.3	308.092843	0.00651591	0	0.26881E+43		
39	750	850		455	5045	317.773504	0.04287518	0.26881E+43	0.035362	2750	849	457.7	5042.3	317.586524	0.04153602	0	0.26881E+43		
40	750	900		320	5180	326.986238	0.16635911	0.26881E+43	0.099829	2750	899	322.7	5177.3	326.804529	0.16257861	0	0.26881E+43		
41	750	935	0.0523	225.5	5274.5	333.283663	0.33863891	0.26881E+43	0.158713	2750	934	228.2	5271.8	333.105389	0.33262846	0	0.26881E+43		
42	750	945	0.1128	198.5	5301.5	335.061188	0.40213109	0.26881E+43	0.174832	2750	944	201.2	5298.8	334.88386	0.39551025	0	0.26881E+43		
43	750	955	0.1555	171.5	5328.5	336.829334	0.47148752	0.26881E+43	0.189725	2750	954	174.2	5325.5	336.652937	0.46430265	0	0.26881E+43		
44	750	970	0.1949	131	5389	339.464284	0.58523913	0.26881E+43	0.208754	2750	969	133.7	5386.3	339.289257	0.57733365	0	0.26881E+43		
45	750	985	0.2088	90.5	5409.5	342.078938	0.70825817	0.26881E+43	0.222649	2750	984	93.2	5406.8	341.90525	0.69886551	0	0.26881E+43		
46	750	1000	0.2179	50	5450	344.673759	0.83745299	0.26881E+43	0.230511	2750	999	52.7	5447.3	344.501379	0.82872358	0	0.26881E+43		
47	750	1015	0.1992	9.50000001	5490.5	347.24919	0.96913764	0.26881E+43	0.231966	2750	1014	12.2	5487.8	347.078089	0.96035313	0	0.26881E+43		
48	750	1030	0.1891	-31	5531	349.80506	1.09973651	0.26881E+43	0.227174	2750	1029	-28.3	5528.3	349.63811	1.09113349	0	0.26881E+43		
49	750	1045	0.1858	-71.5	5571.5	352.343562	1.22857391	0.26881E+43	0.216772	2750	1044	-68.8	5568.8	352.174957	1.21766479	0	0.26881E+43		
50	750	1060	0.1757	-112	5612	354.863354	1.34465283	0.26881E+43	0.207162	2750	1059	-109.3	5609.3	354.695926	1.33701214	0	0.26881E+43		
51	750	1075	0.1608	-152.5	5652.5	357.365359	1.45381937	0.26881E+43	0.183367	2750	1074	-149.8	5649.8	357.199104	1.44687528	0	0.26881E+43		
52	750	1105	0.1642	-233.5	5733.5	362.31754	1.63791921	0.26881E+43	0.141552	2750	1104	-230.8	5730.8	362.153599	1.63255867	0	0.26881E+43		
53	750	1135	0.1507	-314.5	5814.5	367.202941	1.77419577	0.26881E+43	0.100449	2750	1134	-311.8	5811.8	367.041142	1.77039171	0	0.26881E+43		
54	750	1165	0.1277	-475.5	5976.5	374.783759	1.92830248	0.26881E+43	0.040304	2750	1164	-473.8	5973.8	376.526075	1.92477615	0	0.26881E+43		
55	750	1200		-490	5990	377.571185	1.93354197	0.26881E+43	0.036884	2750	1199	-487.3	5987.3	377.413831	1.93214518	0	0.26881E+43		
56	750	1300		-760	6260	392.988549	1.99376094	0.26881E+43	0.00439	2750	1299	-757.3	6257.3	392.837371	1.9935947	0	0.26881E+43		
57	750	1400		-1030	6530	407.823491	1.99964538	0.26881E+43	0.000294	2750	1399	-1027.3	6527.3	407.677814	1.99963427	0	0.26881E+43		
58	750	1500		-1300	6800	422.137418	1.99999657	0.26881E+43	1.24E-05	2750	1499	-1297.3	6797.3	421.996682	1.99998623	0	0.26881E+43		
59	750	1600		-1570	7070	435.981651	1.99999965	0.26881E+43	3.59E-07	2750	1599	-1567.3	7067.3	435.945385	1.99999963	0	0.26881E+43		
60	750	1700		-1840	7340	449.399599	1.99999999	0.26881E+43	7.66E-09	2750	1699	-1837.3	7337.3	449.267404	1.99999999	0	0.26881E+43		
61	750	1800		-2110	7610	462.428373	2	0.26881E+43	1.26E-10	2750	1799	-2107.3	7607.3	462.299903	2	0.26881E+43			
62	750	1900		-2380	7880	475.099989	2	0.26881E+43	1.68E-12	2750	1899	-2377.3	7877.3	474.974947	2	0.26881E+43			
63	750	2000		-2650	8150	487.442304	2	0.26881E+43	0	2750	1999	-2647.3	8147.3	487.320428	2	0.26881E+43			
64	750	2100		-2920	8420	499.479729	2	0.26881E+43	0	2750	2099	-2917.3	8417.3	499.360791	2	0.26881E+43			
65	750	2200		-3190	8690	511.233802	2	0.26881E+43	0	2750	2199	-3187.3	8687.3	511.117599	2	0.26881E+43			
66	750	2300		-3460	8960	522.723636	2	0.26881E+43	0	2750	2299	-3457.3	8957.3	522.609888	2	0.26881E+43			
67	750	2400		-3730	9230	533.966291	2	0.26881E+43	0	2750	2399	-3727.3	9227.3	533.855037	2	0.26881E+43			
68	750	2500		-4000	9500	544.977064	2	0.26881E+43	0	2750	2499	-3997.3	9497.3	544.868057	2	0.26881E+43			
69	750	2600		-4270	9770	555.769736	2	0.26881E+43	0	2750	2599	-4267.3	9767.3	555.62847	2	0.26881E+43			
70	750	2700		-4540	10040	566.356778	2	0.26881E+43	0	2750	2699	-4537.3	10037.3	566.251887	2	0.26881E+43			
71	750	2800		-4810	10310	576.749512	2	0.26881E+43	0	2750	2799	-4807.3	10307.3	576.646512	2	0.26881E+43			
72	750	2900		-5080	10580	586.958261	2	0.26881E+43	0	2750	2899	-5077.3	10577.3	586.857052	2	0.26881E+43			
73	750	3000		-5350	10850	596.992462	2	0.26881E+43	0	2750	2999	-5347.3	10847.3	596.892955	2	0.26881E+43			
74	750	3100		-5620	11120	606.860775	2	0.26881E+43	0	2750	3099	-5617.3	11117.3	606.762886	2	0.26881E+43			
75	750	3200		-5890	11390	616.571164	2	0.26881E+43	0	2750	3199	-5887							

	A	B	C	D	E	F	G	H	I	J	K	L	M	N	O	P	Q	R
1	Country Club Bayou Tracer Test Model																	
2	Advection and Dispersion																	
3	Polk and Elm to Hughes																	
4	Polk and Elm to Hughes																	
5	Polk and Elm to Hughes																	
6	Tracer mass	100	grams															
7	Depth	2.5	ft															
8	Width	13	ft															
9	Length																	
10	Labeled Volume	0.94674556	cu.meters.															
11	Label Concentration	105.625	mg/L															
12	Transport Parameters																	
13	Co	105.625	Tracer Concentration															
14	U	2.6	Fluid Velocity															
15	a	13	Mixing Length															
16	D	33.8	Dispersion Coefficient															
17	T		Pulse Length															
18	Computed Constants																	
19	vd	0.07692308																
20	Q (gpm)	632.06																
21	Intermediate Calculations																	
22	Model																	
23	Intermediate Calculations																	
24	Intermediate Calculations																	
25				(1)	(2)	(3)	(4)	(5)	(6)	(7)	Simulation Parameters	(1)	(2)	(3)	(4)	(5)	(6)	
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Appendix – III Field Data (ACCESS Database Printout)































Appendix – IV Selected Field Data for Statistical Analysis

Ennis and Lamar (N3)											
Odor at Hughes						No Odor at Hughes					
Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)	Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)
29-Oct-98				6500		16-Sep-98	4.8	0.7	28		
25-Nov-98	3.3	1.86	42			22-Sep-98	3	4.12		240000	
25-Nov-98	3.3	1.86	42			22-Sep-98					
2-Dec-98	6.41	1.25	46			30-Sep-98	4.57	5.22	65	22000	
2-Dec-98				3800		30-Sep-98					
23-Dec-98	5.45	1.25	56			7-Oct-98		0.28	35		
30-Dec-98		1.44	52			14-Oct-98	3.3	1.47	24	810	
30-Dec-98				1		14-Oct-98					
6-Jan-99	4.22	2.06	69			21-Oct-98	9.24	0.15	45		
6-Jan-99				48000		28-Oct-98	6.16	0.81	69		
21-Jan-99	4.32	1.59	70			4-Nov-98	9.45	0.64	44		
2-Feb-99	0.95	3.48	56			4-Nov-98				12000	
2-Feb-99				19000		13-Nov-98	9.03	1.06	3		
9-Feb-99	4.38	3.76	70			18-Nov-98	3.84	0.69	74		
9-Feb-99				130000		18-Nov-98				44000	
16-Feb-99	3.12	6.2	61			13-Jan-99	6.92	1.07	73		
16-Feb-99				44000		13-Jan-99				4100	
2-Mar-99	4.44	2.88	82			23-Feb-99	4.36	3.04	63		
2-Mar-99				74000		23-Feb-99				12000	
9-Mar-99	4.43	3.68	60			16-Mar-99	6.85	2.78	55	10000	
9-Mar-99						16-Mar-99					
23-Mar-99	4.74	2.52	68			15-Apr-99	8.79	1.06	51		
23-Mar-99				1090000		11-May-99	8.9	0.72	30		
30-Mar-99	8.15	0.57	22			15-Jun-99	2.42	0.82	35		
5-Apr-99	5.71	2.31	70			15-Jun-99				32000	
23-Apr-99	7.5	3.74	48			22-Jun-99	2.107	0.52	54		
27-Apr-99	7.93	2.08	35			22-Jun-99				3600	
25-May-99	2.7	3.6	70			6-Jul-99	2.93	0.44	45		
25-May-99				14000		15-Jul-99	4.2	4.24	58		
8-Jun-99	2.37	2.56	70			20-Jul-99	3.3	1.77	52		
8-Jun-99				58000		20-Jul-99				870	
29-Jun-99	4.33	1	46			3-Aug-99		3.08	73		
29-Jun-99				9		3-Aug-99				610	
27-Jul-99	5.4	3.68	63			21-Oct-99	3.39	4.22	48		
27-Jul-99				800		1-Nov-99	4.43	0.82	51		
4-Oct-99	3.25	0.24	59								
Mean Values											
	DO	Ammonia	Sulfate	Fecal Coliform	BOD						
Odor	4.59	<b>2.44</b>	57.14	114,470							
No Odor	5.33	<b>1.73</b>	48.86	31,833							
Differences in <b>Bold</b> values are statistically significant at p=0.05											

Park and Elm (N1)											
Odor at Hughes						No Odor at Hughes					
Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)	Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)
29-Oct-98				22000		22-Sep-98		0.2		4600	2.1
25-Nov-98	4.93	0.1	46			22-Sep-98	2.61	0.45		4600	2.1
25-Nov-98	4.93	0.1	46			30-Sep-98		0.1		45000	44.7
2-Dec-98	6.61	0.24	42			30-Sep-98	2.99	1.13	70	45000	44.7
2-Dec-98		0.1		22000	0.8	7-Oct-98	2.86	1.39	42		
23-Dec-98	7.21	1	66			7-Oct-98					
30-Dec-98	10.59	1.02	44			14-Oct-98		0.3		5400	4.9
30-Dec-98		0.5			2	14-Oct-98	1.25	0.32	30	5400	4.9
6-Jan-99	5.64	2.44	72			21-Oct-98					
6-Jan-99		2.3		880000	7.6	21-Oct-98	8.02	0.15	52		
21-Jan-99	5.38	2.45	59			28-Oct-98	10.29	1.27	88		
2-Feb-99	0.92	0.85	55			4-Nov-98	10.3	0.12	70		
2-Feb-99				33000	42	4-Nov-98		0.1		11000	1.2
9-Feb-99	3.87	1.44	55			13-Nov-98	8.63	0.55	7		
9-Feb-99		0.1		42000	39	18-Nov-98	5.38	0.1	32		
16-Feb-99	2.61	4.1	36			18-Nov-98		0.1		5800	3.9
16-Feb-99		0.5		32000	400	13-Jan-99	8.86	2.66	85		
2-Mar-99	2.52	4.68	51			13-Jan-99		3.4		2300000	11.6
2-Mar-99		0.1		7200	700	23-Feb-99		0.1		11000	40
9-Mar-99	5.89	0.87	54			23-Feb-99	5.01	1.74	55		
9-Mar-99		0.4		-999	44.1	16-Mar-99				94000	
23-Mar-99	6.55	0.32	69			16-Mar-99	6.51	0.4	0.3	94000	
23-Mar-99		0.1		94000	8.8	15-Jun-99	2.83	0.28	42		
30-Mar-99	8.38	0.37	16			15-Jun-99				4000	3.2
5-Apr-99	4.34	1.99	58			22-Jun-99				4400	3.6
25-May-99				260000	44	22-Jun-99					
8-Jun-99	4.33	0	59			6-Jul-99					
8-Jun-99				3000	8.9	15-Jul-99					
29-Jun-99	4.93	1.11	49			20-Jul-99				300	1.1
29-Jun-99				10	2.1	3-Aug-99				2400	2.8
27-Jul-99	6.4	0.17	73			3-Aug-99		0.3	62		
27-Jul-99				2600	2.8						
Mean Values											
	DO	Ammonia	Sulfate	Fecal Coliform	BOD						
Odor	5.34	1.05	52.78	107,447	100						
No Odor	5.81	0.72	48.87	164,806	12						
Differences in <b>Bold</b> values are statistically significant at p=0.05											

Polk and Elm (S1)											
Odor at Hughes						No Odor at Hughes					
Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)	Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)
29-Oct-98				72		22-Sep-98	3.54	0.96		690	4.6
25-Nov-98	6.3	0.97	72			22-Sep-98		0.6		690	4.6
25-Nov-98	6.3	0.97	72			30-Sep-98		0.9		4300	32.5
2-Dec-98	4.63	0.26	53			7-Oct-98	2.94	0.55	59		
2-Dec-98		0.2		24000	3.5	14-Oct-98		0.7		2800	2.3
23-Dec-98	7.7	0.11	47			14-Oct-98	4.3	1	71	2800	2.3
30-Dec-98		0.2		3300	3	21-Oct-98	8.39	0.5	60		
30-Dec-98		0.64	74			28-Oct-98	9.27	1.42	64		
6-Jan-99		0.1		51000	7.2	4-Nov-98	5.4	0.79	60		
6-Jan-99	4.49	0.04	74			4-Nov-98		0.5		7700	4.8
21-Jan-99	7.59	0.62	74			13-Nov-98	5.72	0.62	14		
2-Feb-99	0.59	0.27	72			18-Nov-98	7.8	0.58	46		
2-Feb-99		0.2		28000	5.7	18-Nov-98		0.2		340	3
9-Feb-99		0.9		270	11.6	13-Jan-99	13.02	0.49	72		
9-Feb-99	4.53	1.34	59			13-Jan-99		0.6		4200	3.9
16-Feb-99		0.9		15000	29.2	23-Feb-99		0.9		140000	
16-Feb-99	4.4	0.92	76			23-Feb-99	4.59	1.07	73		
2-Mar-99	3.86	1.17	73			16-Mar-99				22000	
2-Mar-99		0.7		33000	113.4	16-Mar-99	4.94	0.82	62	22000	
9-Mar-99	2.82	0.62	61			15-Apr-99	6.62	0	53		
9-Mar-99		0.6			136.5	11-May-99	8.27	0.19	38		
23-Mar-99		0.2		21000	125.2	15-Jun-99	2.96	1.09	55		
23-Mar-99	3.87	0.32	56			15-Jun-99				20	38.5
30-Mar-99	8.15	0	24			22-Jun-99	1.22	0.67	50		
5-Apr-99	1.19	1.17	70			22-Jun-99				4800	11.9
23-Apr-99	7.24	0.43	75			6-Jul-99	1.29	1.37	31		
27-Apr-99	6.14	0.46	42			15-Jul-99	3.3	0.15	64		
25-May-99				30000	18	20-Jul-99	4.5	0.23	51		
25-May-99	7.6	1.03	66			20-Jul-99				3600	2.8
8-Jun-99	6.33	0.53	63			3-Aug-99		0.34	84		
8-Jun-99				11000	37	3-Aug-99				16000	4.2
29-Jun-99				820	15.9	13-Aug-99		0.39	64		
29-Jun-99	2.72	0.82	61			21-Oct-99	3.93	0.48	45		
27-Jul-99	4.8	0.25	65			1-Nov-99	4.87	0.9	51		
27-Jul-99				4100	4.1						
4-Oct-99	2.62	0.01	48								
Mean Values											
	DO	Ammonia	Sulfate	Fecal Coliform	BOD						
Odor	4.95	0.55	62.59	17043.23	39.25						
No Odor	5.34	0.66	55.57	15462.67	9.62						
Differences in Bold values are statistically significant at p=0.05											













Ennis and Lamar (N3)											
Filaments at Hughes						No Filaments at Hughes					
Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)	Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)
22-Sep-98	3	4.12		240000		7-Oct-98		0.28	35		
22-Sep-98				240000		21-Oct-98	9.24	0.15	45		
30-Sep-98				22000		4-Nov-98	9.45	0.64	44		
30-Sep-98	4.57	5.22	65	22000		4-Nov-98				12000	
14-Oct-98				810		13-Nov-98	9.03	1.06	3		
14-Oct-98	3.3	1.47	24	810		30-Dec-98		1.44	52		
28-Oct-98	6.16	0.81	69		30-Dec-98					1	
29-Oct-98				6500		23-Feb-99	4.36	3.04	63		
18-Nov-98	3.84	0.69	74		23-Feb-99					12000	
18-Nov-98				44000		16-Mar-99				10000	
25-Nov-98	3.3	1.86	42		16-Mar-99	6.85	2.78	55	10000		
25-Nov-98	3.3	1.86	42		23-Mar-99	4.74	2.52	68			
2-Dec-98	6.41	1.25	46		23-Mar-99					1090000	
2-Dec-98				3800	30-Mar-99	8.15	0.57	22			
23-Dec-98	5.45	1.25	56		15-Apr-99	8.79	1.06	51			
6-Jan-99	4.22	2.06	69		27-Apr-99	7.93	2.08	35			
6-Jan-99				48000	11-May-99	8.9	0.72	30			
13-Jan-99	6.92	1.07	73								
13-Jan-99				4100							
21-Jan-99	4.32	1.59	70								
2-Feb-99				19000							
2-Feb-99	0.95	3.48	56								
9-Feb-99				130000							
9-Feb-99	4.38	3.76	70								
16-Feb-99	3.12	6.2	61								
16-Feb-99				44000							
2-Mar-99				74000							
2-Mar-99	4.44	2.88	82								
9-Mar-99	4.43	3.68	60								
9-Mar-99											
5-Apr-99	5.71	2.31	70								
23-Apr-99	7.5	3.74	48								
Mean Values											
	DO	Ammonia	Sulfate	Fecal Coliform	BOD						
Filaments	4.49	2.59	59.83	59,935							
No Filament	7.74	1.36	41.92	189,000							
Differences in Bold values are statistically significant at p=0.05											

Park and Elm (N1)											
Filaments at Hughes						No Filaments at Hughes					
Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)	Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)
22-Sep-98		0.2		4600	2.1	7-Oct-98	2.86	1.39	42		
22-Sep-98	2.61	0.45		4600	2.1	7-Oct-98					
30-Sep-98		0.1		45000	44.7	21-Oct-98					
30-Sep-98	2.99	1.13	70	45000	44.7	21-Oct-98	8.02	0.15	52		
14-Oct-98		0.3		5400	4.9	4-Nov-98	10.3	0.12	70		
14-Oct-98	1.25	0.32	30	5400	4.9	4-Nov-98		0.1		11000	1.2
28-Oct-98	10.29	1.27	88			13-Nov-98	8.63	0.55	7		
29-Oct-98				22000		30-Dec-98	10.59	1.02	44		
18-Nov-98	5.38	0.1	32			30-Dec-98		0.5			2
18-Nov-98		0.1		5800	3.9	23-Feb-99		0.1		11000	40
25-Nov-98	4.93	0.1	46			23-Feb-99	5.01	1.74	55		
25-Nov-98	4.93	0.1	46			18-Mar-99				94000	
2-Dec-98	6.61	0.24	42			16-Mar-99	6.51	0.4	0.3	94000	
2-Dec-98		0.1		22000	0.8	23-Mar-99	6.55	0.32	69		
23-Dec-98	7.21	1	66			23-Mar-99		0.1		94000	8.8
6-Jan-99	5.64	2.44	72			30-Mar-99	8.38	0.37	16		
6-Jan-99		2.3		880000	7.6						
13-Jan-99	8.86	2.66	85								
13-Jan-99		3.4		2300000	11.6						
21-Jan-99	5.38	2.45	59								
2-Feb-99	0.92	0.85	55								
2-Feb-99				33000	42						
9-Feb-99		0.1		42000	39						
9-Feb-99	3.87	1.44	55								
16-Feb-99		0.5		32000	400						
16-Feb-99	2.61	4.1	36								
2-Mar-99		0.1		7200	700						
2-Mar-99	2.52	4.68	51								
9-Mar-99	5.89	0.87	54								
9-Mar-99		0.4			44.1						
5-Apr-99	4.34	1.99	58								
Mean Values											
	DO	Ammonia	Sulfate	Fecal Coliform	BOD						
Filaments	<b>4.79</b>	<b>1.17</b>	55.59	230,267	90						
No Filament	<b>7.43</b>	<b>0.53</b>	39.48	60,800	13						
Differences in <b>Bold</b> values are statistically significant at p=0.05											



Evergreen Cemetery, North and South Vents						No Filaments at Hughes					
Field Visit Date	Filaments at Hughes					Field Visit Date	No Filaments at Hughes				
	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100m)	BOD (mg/L)		DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100m)	BOD (mg/L)
2-Apr-98						27-May-98					
28-Apr-98						14-Sep-98	2.78				
12-May-98						7-Oct-98	1.28	0.48	50		
21-May-98						21-Oct-98	5.44	0.49	55		
28-May-98						4-Nov-98				12500	
3-Jun-98						4-Nov-98		0.2		520000	3.9
8-Jul-98						4-Nov-98				910000	
30-Sep-98				200000		4-Nov-98	1.7	0.21	63		
30-Sep-98				76000		13-Nov-98	6.33	0.58	0		
30-Sep-98		0.6		200000	27.1	30-Dec-98				1	
30-Sep-98		0.82	59	200000	27.1	30-Dec-98				76	
14-Oct-98	1.78	1.19	65	20000	4	30-Dec-98	8.73	1.21	64		
14-Oct-98				450		30-Dec-98		0.7		1	2.2
14-Oct-98				4900		23-Feb-99		0.1		29000	4.6
14-Oct-98		0.8		20000	4	23-Feb-99	2.47	0.6	32		
28-Oct-98	8.634	2.13	72			23-Feb-99	2.63	0.15	65		
29-Oct-98						23-Feb-99		0.5		4800	19.7
29-Oct-98				36		16-Mar-99				53000	
29-Oct-98				330		16-Mar-99	4.04	0.35	62	53000	
18-Nov-98	8.3	0.57	57			16-Mar-99	5.49	0.96	47	3500	
18-Nov-98		0.2		120000	5.4	16-Mar-99				3500	
18-Nov-98				320000		23-Mar-99		0.4		4100	294.4
18-Nov-98				3500		23-Mar-99	3.57	0.69	30		
25-Nov-98	4.6	1.31	68			23-Mar-99		0.6		6000	200
25-Nov-98	4.6	1.31	68			23-Mar-99	4.78	0.06	73		
2-Dec-98		0.2		13000	5.5	30-Mar-99	7.66	0.42	20		
2-Dec-98	12.76	0.43	20			30-Mar-99	8.05	0.26	26		
2-Dec-98				9700		15-Apr-99	6.93	0.55	25		
2-Dec-98				23000		15-Apr-99	2.63	0.57	31		
23-Dec-98	7.23	0.13	72			27-Apr-99	4.35	1.61	20		
6-Jan-99		2.8		1200000	23.9	27-Apr-99	5.32	1.33	28		
6-Jan-99	3.25	2.79	74			11-May-99	7.9	0.21	28		
6-Jan-99	4.25	0.86	48			11-May-99	7.91	0.45	26		
6-Jan-99		0.9		31000							
13-Jan-99	9.97	1.95	70								
13-Jan-99	12.95	0.99	58								
13-Jan-99		1.7		1700000	10.3						
13-Jan-99		1.1		220000	2.8						
21-Jan-99	6.97	1.41	57								
21-Jan-99	3.33	1.36	60								
2-Feb-99	1.73	1.85	48								
2-Feb-99	18.8	0.7	68								
2-Feb-99		0.6		5400	3.7						
2-Feb-99		1.5		5700	33.9						
9-Feb-99	3.9	1.34	49								
9-Feb-99		1.2		6700	1.3						
9-Feb-99	3.52	2.06	49								
9-Feb-99		1.3		57000	141.2						
18-Feb-99	4.79	2.36	57								
18-Feb-99		1.9		1900	2.7						
18-Feb-99	1.28	0.89	68								
16-Feb-99		0.8		43000	42.4						
2-Mar-99		0.4		51000	140.5						
2-Mar-99	2.41	0.64	67								
2-Mar-99	1.2	0.71	72								
2-Mar-99		0.2		8600	24.8						
9-Mar-99	1.8	0.66	55								
9-Mar-99		0.7			11.8						
9-Mar-99		0.1			212.2						
9-Mar-99	1.88	1.15	48								
5-Apr-99	3.93	1.35	50								
5-Apr-99	1.38	0.67	64								
23-Apr-99	6.2	2.13	26								
23-Apr-99	3.23	0.36	72								
Mean Values											
	DO	Ammonia	Sulfate	Fecal Coliform	BOD						
Filament	5.32	1.12	<b>68.61</b>	162,186	38.14						
No Filament	5.00	0.55	<b>39.21</b>	114,248	87.47						
Differences in <b>Bold</b> values are statistically significant at p=0.05											











Ennis and Lamar (N3)											
Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)	Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)
16-Sep-98	4.8	0.7	28			6-Jan-99	4.22	2.06	69		
22-Sep-98	3	4.12		240000		6-Jan-99				48000	
22-Sep-98				240000		13-Jan-99	6.92	1.07	73		
30-Sep-98	4.57	5.22	65	22000		13-Jan-99				4100	
30-Sep-98				22000		21-Jan-99	4.32	1.59	70		
7-Oct-98		0.28	35			2-Feb-99	0.95	3.48	56		
14-Oct-98	3.3	1.47	24	810		2-Feb-99				19000	
14-Oct-98				810		9-Feb-99	4.38	3.76	70		
21-Oct-98	9.24	0.15	45			9-Feb-99				130000	
28-Oct-98	6.16	0.81	69			16-Feb-99	3.12	6.2	61		
29-Oct-98				6500		16-Feb-99				44000	
4-Nov-98	9.45	0.64	44			23-Feb-99	4.36	3.04	63		
4-Nov-98				12000		23-Feb-99				12000	
13-Nov-98	9.03	1.06	3			2-Mar-99	4.44	2.88	82		
18-Nov-98	3.84	0.69	74			2-Mar-99				74000	
18-Nov-98				44000		9-Mar-99	4.43	3.68	60		
25-Nov-98	3.3	1.86	42			9-Mar-99					
2-Dec-98	6.41	1.25	46			16-Mar-99	6.85	2.78	55	10000	
2-Dec-98				3800		16-Mar-99				10000	
9-Dec-98				3800		23-Mar-99	4.74	2.52	68		
17-Dec-98		0.88	40			23-Mar-99				1090000	
22-Dec-98				36000		30-Mar-99	8.15	0.57	22		
23-Dec-98	5.45	1.25	56			5-Apr-99	5.71	2.31	70		
30-Dec-98		1.44	52			13-Apr-99				1090000	
30-Dec-98				1		15-Apr-99	8.79	1.06	51		
Mean 98	5.7125	1.454667	44.5	48593.92	#DIV/0!	23-Apr-99	7.5	3.74	48		
						27-Apr-99	7.93	2.08	35		
						11-May-99	8.9	0.72	30		
						25-May-99	2.7	3.6	70		
						25-May-99				14000	
						1-Jun-99				950	
						8-Jun-99	2.37	2.56	70		
						8-Jun-99				58000	
						15-Jun-99	2.42	0.82	35		
						15-Jun-99				32000	
						22-Jun-99	2.107	0.52	54		
						22-Jun-99				3600	
						29-Jun-99	4.33	1	46		
						29-Jun-99				9	
						6-Jul-99	2.93	0.44	45		
						13-Jul-99				130	
						15-Jul-99	4.2	4.24	58		
						20-Jul-99	3.3	1.77	52		
						20-Jul-99				870	
						27-Jul-99	5.4	3.68	63		
						27-Jul-99				800	
						3-Aug-99		3.08	73		
						3-Aug-99				610	
						10-Aug-99				14000	
						17-Aug-99				60000	149.1
						31-Aug-99				34000	
						13-Sep-99				440	
						20-Sep-99				400	
						27-Sep-99				2100	
						4-Oct-99	3.25	0.24	59		
						21-Oct-99	3.39	4.22	48		
						1-Nov-99	4.43	0.82	51		
						29-Nov-99				5500	
						Mean 99	4.708172	2.351	56.9	98518.18	149.1

Park and Elm N1											
Field Visit Dat	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)	Field Visit Da	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)
22-Sep-98		0.2		4600	2.1	6-Jan-99	5.64	2.44	72		
22-Sep-98	2.61	0.45		4600	2.1	6-Jan-99		2.3		880000	7.6
30-Sep-98		0.1		45000	44.7	13-Jan-99	8.86	2.66	85		
30-Sep-98	2.99	1.13	70	45000	44.7	13-Jan-99		3.4		2300000	11.6
7-Oct-98	2.86	1.39	42			21-Jan-99	5.38	2.45	59		
7-Oct-98						2-Feb-99	0.92	0.85	55		
14-Oct-98		0.3		5400	4.9	2-Feb-99				33000	42
14-Oct-98	1.25	0.32	30	5400	4.9	9-Feb-99		0.1		42000	39
21-Oct-98						9-Feb-99	3.87	1.44	55		
21-Oct-98	8.02	0.15	52			16-Feb-99		0.5		32000	400
28-Oct-98	10.29	1.27	88			16-Feb-99	2.61	4.1	36		
29-Oct-98				22000		23-Feb-99		0.1		11000	40
4-Nov-98	10.3	0.12	70			23-Feb-99	5.01	1.74	55		
4-Nov-98		0.1		11000	1.2	2-Mar-99		0.1		7200	700
13-Nov-98	8.63	0.55	7			2-Mar-99	2.52	4.68	51		
18-Nov-98	5.38	0.1	32			9-Mar-99	5.89	0.87	54		
18-Nov-98		0.1		5800	3.9	9-Mar-99		0.4		-999	44.1
25-Nov-98	4.93	0.1	46			16-Mar-99				94000	
2-Dec-98	6.61	0.24	42			16-Mar-99	6.51	0.4	0.3	94000	
2-Dec-98		0.1		22000	0.8	23-Mar-99	6.55	0.32	69		
9-Dec-98		0.1		40000	23.1	23-Mar-99		0.1		94000	8.8
17-Dec-98	7.14	0.39	66			30-Mar-99	8.38	0.37	16		
22-Dec-98				49000	42	5-Apr-99	4.34	1.99	58		
23-Dec-98	7.21	1	66			13-Apr-99				250000	
30-Dec-98	10.59	1.02	44			25-May-99				260000	44
30-Dec-98		0.5			2	1-Jun-99				680	0.9
						8-Jun-99	4.33	0	59		
						8-Jun-99				3000	8.9
						15-Jun-99	2.83	0.28	42		
						15-Jun-99				4000	3.2
						22-Jun-99				4400	3.6
						22-Jun-99					
						29-Jun-99	4.93	1.11	49		
						29-Jun-99				10	2.1
						6-Jul-99					
						13-Jul-99				240	0.7
						15-Jul-99					
						20-Jul-99				300	1.1
						27-Jul-99	6.4	0.17	73		
						27-Jul-99				2600	2.8
						3-Aug-99				2400	2.8
						3-Aug-99		0.3	62		
						10-Aug-99				23000	5.4
						17-Aug-99				60000	22.7
						31-Aug-99				3800	2.8
						13-Sep-99				2700	1.4
						20-Sep-99				940	3.5
						27-Sep-99				720	
						29-Nov-99				24000	
Mean 1998	6.34	0.44	50.38	21,650	14.70						
Mean 1999	5.00	1.28	52.79	145,827	58.29						

Polk and Elm (S1)											
1998						1999					
Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)	Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)
22-Sep-98	3.54	0.98		690	4.6	6-Jan-99	4.49	0.04	74		
22-Sep-98		0.6		690	4.6	6-Jan-99		0.1		51000	7.2
29-Sep-98		1.24	71	4300	32.5	13-Jan-99	13.02	0.49	72		
30-Sep-98		0.9		4300	32.5	13-Jan-99		0.6		4200	3.9
7-Oct-98	2.94	0.55	59			21-Jan-99	7.59	0.62	74		
14-Oct-98	4.3	1	71	2800	2.3	2-Feb-99	0.59	0.27	72		
14-Oct-98		0.7		2800	2.3	2-Feb-99		0.2		28000	5.7
21-Oct-98	8.39	0.5	60			9-Feb-99	4.53	1.34	59		
28-Oct-98	9.27	1.42	64			9-Feb-99		0.9		270	11.6
29-Oct-98				72		16-Feb-99	4.4	0.92	76		
4-Nov-98	5.4	0.79	60			16-Feb-99		0.9		15000	29.2
4-Nov-98		0.5		7700	4.8	23-Feb-99	4.59	1.07	73		
13-Nov-98	5.72	0.62	14			23-Feb-99		0.9		140000	1550
18-Nov-98	7.8	0.58	46			2-Mar-99	3.86	1.17	73		
18-Nov-98		0.2		340	3	2-Mar-99		0.7		33000	113.4
25-Nov-98	6.3	0.97	72			9-Mar-99	2.82	0.62	61		
2-Dec-98	4.63	0.26	53			9-Mar-99		0.6		-999	136.5
2-Dec-98		0.2		24000	3.5	16-Mar-99	4.94	0.82	62	22000	
9-Dec-98		0.5		14000	5.1	16-Mar-99				22000	
17-Dec-98		0.25	66			23-Mar-99	3.87	0.32	56		
22-Dec-98				7300	10.9	23-Mar-99		0.2		21000	125.2
23-Dec-98	7.7	0.11	47			30-Mar-99	8.15	0	24		
30-Dec-98		0.64	74			5-Apr-99	1.19	1.17	70		
30-Dec-98		0.2		3300	3	13-Apr-99		0.4		490	18.4
						15-Apr-99	6.62	0	53		
						23-Apr-99	7.24	0.43	75		
						27-Apr-99	6.14	0.46	42		
						11-May-99	8.27	0.19	38		
						18-May-99					
						25-May-99	7.6	1.03	66		
						25-May-99				30000	18
						1-Jun-99				5300	2.6
						8-Jun-99	6.33	0.53	63		
						8-Jun-99				11000	37
						15-Jun-99	2.96	1.09	55		
						15-Jun-99				20	38.5
						22-Jun-99	1.22	0.67	50		
						22-Jun-99				4800	11.9
						29-Jun-99	2.72	0.82	61		
						29-Jun-99				820	15.9
						6-Jul-99	1.29	1.37	31		
						13-Jul-99				3000	40
						15-Jul-99	3.3	0.15	64		
						20-Jul-99	4.5	0.23	51		
						20-Jul-99				3600	2.8
						27-Jul-99	4.8	0.25	65		
						27-Jul-99				4100	4.1
						3-Aug-99		0.34	84		
						3-Aug-99				16000	4.2
						10-Aug-99				5400	2.1
						13-Aug-99		0.39	64		
						17-Aug-99				340	0.1
						31-Aug-99				3400	4.7
						13-Sep-99				10	3.7
						20-Sep-99				20000	4.4
						27-Sep-99				2500	25.1
						4-Oct-99	2.62	0.01	48		
						21-Oct-99	3.93	0.48	45		
						1-Nov-99	4.87	0.9	51		
						29-Nov-99				9	
Mean 1998	6.00	0.62	58.23	5,561	9.09						
Mean 1999	4.77	0.58	59.74	15,388	85.24						

Evergreen Cemetery, North and South Vents										
Sample Visit Date	1998					1999				
	DO (ppm)	Ammonia_NH308 (mg/L)	Sulfide_S051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)	Sample Visit Date	DO (ppm)	Ammonia_NH308 (mg/L)	Sulfide_S051 (mg/L)	Fecal Coliform (cfu/100mL)
8-Sep-97						6-Jan-99	2.8			
2-Apr-98						6-Jan-99	3.25	2.79	74	120000
28-Apr-98						6-Jan-99	4.23	0.80	48	
12-May-98						6-Jan-99		0.9		31000
21-May-98						13-Jan-99	9.97	1.95	70	
27-May-98						13-Jan-99	12.95	0.99	58	
28-May-98						13-Jan-99		1.7		170000
3-Jun-98						13-Jan-99		1.1		220000
6-Jun-98						21-Jan-99	6.07	1.41	57	
11-Jun-98						21-Jan-99	3.33	1.36	80	
15-Jun-98						2-Feb-99	1.73	1.85	46	
17-Jun-98						2-Feb-99	18.8	0.7	88	5400
22-Jun-98						2-Feb-99		0.6		5700
25-Jun-98						2-Feb-99		1.5		33.0
28-Jun-98						9-Feb-99	3.9	1.34	49	
8-Jul-98						9-Feb-99		1.2		6700
15-Jul-98						9-Feb-99	3.52	2.00	49	1.3
20-Jul-98						9-Feb-99		1.3		57000
29-Jul-98	2.7	0.47	53			16-Feb-99	4.79	2.38	57	141.2
12-Aug-98	4.08	0.85	53			16-Feb-99		1.9		1900
16-Aug-98	1.7		41			16-Feb-99	1.28	0.89	68	2.7
25-Aug-98	1.24	0.57	67			16-Feb-99		0.8		43000
8-Sep-98	3.06	0.4	30			23-Feb-99		0.1		29000
14-Sep-98	2.76					23-Feb-99	2.47	0.6	32	4.6
16-Sep-98	4.9	0.28	27			23-Feb-99	2.63	0.15	65	
23-Sep-98	1.84	0.53	73			23-Feb-99		0.5		4800
30-Sep-98				20000		2-Mar-99		0.4		51000
30-Sep-98				75000		2-Mar-99	2.41	0.04	67	140.5
30-Sep-98		0.6		20000	27.1	2-Mar-99	1.2	0.71	72	
30-Sep-98		0.82	59	20000	27.1	2-Mar-99		0.2		8800
7-Oct-98	1.28	0.46	50			9-Mar-99	1.8	0.89	55	24.8
11-Oct-98	1.78	1.19	85		4	9-Mar-99		0.7		899
14-Oct-98				450		9-Mar-99		0.1		999
14-Oct-98				4000		9-Mar-99	1.88	1.15	48	212.2
14-Oct-98		0.8		20000	4	16-Mar-99				53000
21-Oct-98	5.44	0.49	55			16-Mar-99	4.04	0.95	82	53000
28-Oct-98	8.834	2.13	72			16-Mar-99	5.49	0.98	47	3500
29-Oct-98						16-Mar-99				3500
29-Oct-98				36		23-Mar-99		0.4		4100
29-Oct-98				330		23-Mar-99	3.57	0.69	30	294.4
4-Nov-98		0.2		12500		23-Mar-99		0.6		6000
4-Nov-98				520000	3.9	23-Mar-99	4.78	0.06	73	200
4-Nov-98				910000		30-Mar-99	7.86	0.42	20	
4-Nov-98	1.7	0.21	83			30-Mar-99	8.05	0.26	26	
13-Nov-98	6.33	0.58	0			5-Apr-99	3.93	1.35	50	
18-Nov-98	8.3	0.57	57			5-Apr-99	1.38	0.67	64	
18-Nov-98		0.2		120000	5.4	13-Apr-99		0.2		1370000
18-Nov-98				320000		13-Apr-99		0.8		410000
18-Nov-98				3500		15-Apr-99	6.93	0.55	25	87.5
23-Nov-98	4.6	1.31	68			15-Apr-99	2.03	0.57	51	52.7
2-Dec-98		0.2		13000	5.5	23-Apr-99	6.2	2.13	28	
2-Dec-98	12.76	0.43	20			23-Apr-99	3.23	0.36	72	
2-Dec-98				9700		27-Apr-99	4.85	1.81	20	
2-Dec-98				23000		27-Apr-99	5.32	1.33	28	
8-Dec-98				180000		11-May-99	7.9	0.21	28	
5-Dec-98		0.3		190000	24.7	11-May-99	7.91	0.45	26	
9-Dec-98				31000		18-May-99				
17-Dec-98	5.58	0.37	69			25-May-99				
22-Dec-98		0.1		45000	41	25-May-99	5.8	0.32	80	
22-Dec-98				56000		25-May-99				21000
22-Dec-98				620000		25-May-99				40
23-Dec-98	7.23	0.13	72			1-Jun-99				4700
30-Dec-98				1		1-Jun-99				20.7
30-Dec-98				76		8-Jun-99				58000
30-Dec-98	8.73	1.21	64			8-Jun-99				
30-Dec-98		0.7		1	2.2	8-Jun-99				
						8-Jun-99	1.14	0.19	65	
						15-Jun-99				
						15-Jun-99				10
						15-Jun-99	3.26	0.93	54	25.4
						15-Jun-99				
						22-Jun-99	3	0.73	45	
						22-Jun-99				
						22-Jun-99				
						22-Jun-99				27000
						22-Jun-99				3.3
						29-Jun-99	4.46	0.96	54	
						29-Jun-99				
						29-Jun-99				3
						6-Jul-99	1.84	0.27	15	5.5
						6-Jul-99				
						13-Jul-99				10000
						13-Jul-99				4
						15-Jul-99	3.88	0.43	25	
						15-Jul-99				
						20-Jul-99				7100
						20-Jul-99				2.8
						26-Jul-99	5.8	0.29	32	
						27-Jul-99				15000
						27-Jul-99				10.5
						27-Jul-99	7.15	0.57	51	
						27-Jul-99				
						3-Aug-99				
						3-Aug-99				
						3-Aug-99				33000
						3-Aug-99		0.49	86	6.5
						10-Aug-99				
						10-Aug-99				9700
						13-Aug-99		1.64	67	2.2
						17-Aug-99				
						17-Aug-99				55000
						31-Aug-99				14.5
						31-Aug-99				80000
						13-Sep-99				340
						13-Sep-99				2.9
						20-Sep-99				6000
						20-Sep-99				2.9
						27-Sep-99				150
						27-Sep-99				41
						4-Oct-99	1.43	0.25	57	
						21-Oct-99	2.06	1.85	54	
						1-Nov-99		0.79	53	
						29-Nov-99				9
						29-Nov-99				
Mean 1998	4.77	0.60	52.90	134.839	14.40					
Mean 1999	4.92	0.32	49.40	130.392	43.04					



Highway Street	Peak Visit Date	DO (ppm)	Ammonia_Nitrate (mg/L)	Sulfate_MgSO4 (mg/L)	Feed Coliform (col/100ml)	BOD (mg/L)	Feed Visit Date	DO (ppm)	Ammonia_Nitrate (mg/L)	Sulfate_MgSO4 (mg/L)	Feed Coliform (col/100ml)	BOD (mg/L)	Feed Visit Date	DO (ppm)	Ammonia_Nitrate (mg/L)	Sulfate_MgSO4 (mg/L)	Feed Coliform (col/100ml)	BOD (mg/L)		
	15-Feb-90			180000			15-Jan-98			9700	0	8-Jan-99	3.86	1.26	87					
	30-May-90			180000			16-Jan-98			9700	8	8-Jan-99		1			470000	26.8		
	15-Jun-90			180000			21-Jan-98		0.1	1100000	10.7	13-Jan-99	3.42	0.79	71					
	28-Sep-92			180000			21-Jan-98		10.7	1100000	0.1	13-Jan-99		0.8			1200000	10.8		
	24-Feb-93	0.7		170	1.8	28-Jan-98			57	730000	0.09	21-Jan-99	3.89	0.38	86					
	16-Apr-93			180000			28-Jan-98			15000	57.4	22-Jan-99	2.04	1.24	86					
	23-Apr-93	0.2		12000	2	12-Feb-98			3.3	36000	0.2	28-Jan-99	1.16	2.13	53					
	28-Jun-93			780000			12-Feb-98		0.2	36000	3.3	2-Feb-99	2.94	1.18	58					
	15-Jul-93			890000			24-Feb-98		0.2	1400	8.2	2-Feb-99		0.9			9100	4.5		
	22-Jul-93			1300000			24-Feb-98		0.16	1400	8.2	4-Feb-99	2.72	1.84	58					
	4-Aug-93			180000			3-Mar-98		0.1	120	5.5	5-Feb-99	0.28	1.48	83					
	17-Aug-93	0.9		880000	4.4	3-Mar-98			0.1	120	5.5	8-Feb-99	3.29	0.98	48					
	26-Oct-93			9	1.6	25-Mar-98			1.8	200000	18.8	8-Feb-99		1.4			89000			
	28-Oct-93	1		80000			25-Mar-98		1.8	200000	18.8	12-Feb-99		2.12	47					
	17-Feb-94			200		1-Apr-98			1	1400000	27.1	18-Feb-99	5.45	1.55	58					
	31-Mar-94	1.8		7700	2.4	1-Apr-98			1	1400000	27.1	18-Feb-99		2			450	2.5		
	12-Apr-94			180000			2-Apr-98			76		18-Feb-99	4.58	0.47	9					
	27-Sep-94	1		3300	1.8	15-Apr-98			0.2	380000	30.3	23-Feb-99	1.83	0.15	36					
	2-Nov-94	1.2		485000	2.7	15-Apr-98			0.2	380000	30.3	23-Feb-99		0.4			1400	3.4		
	13-Dec-94			180000			21-Apr-98		0.5	32000	3.8	26-Feb-99	1.43	0.35	51					
	25-Jan-95			22000			21-Apr-98		0.5	32000	3.8	2-Mar-99		0.2			220000	121.7		
	8-Feb-95	1.4		100000	5.4	27-Apr-98			0.8	31000	8.3	2-Mar-99	1.3	0.74	40					
	23-May-95			180000			27-Apr-98		0.8	31000	8.3	5-Mar-99	1.28	1.41	41					
	1-May-95	2.5		1415000	5.9	28-Apr-98	0.8	1.3	68			8-Mar-99	1.3	0.73	36					
	13-May-95			200000			5-May-98		1.1	8200	4.8	8-Mar-99		0.8				13.1		
	23-May-95			180000			8-May-98		1.1	8200	4.8	11-Mar-99	0.87	0.83	53					
	15-Jun-95			2000000			12-May-98	1.41	0.98			15-Mar-99		7	0.88					
	26-Jul-95			180000			12-May-98		0.8	45000	7.7	15-Mar-99					10			
	8-Aug-95	0.2		190000	3.3	12-May-98				45000	7.7	23-Mar-99		0.4			2300	3.8		
	22-Aug-95			860000			20-May-98		0.4	100000	9.3	23-Mar-99	2.1	0.36	57					
	20-Sep-95			2300			21-May-98	1.2	1.23	61		25-Mar-99	1.14	0.21	55					
	5-Oct-95			180000			27-May-98	1.1	0.38	83		30-Mar-99	7.8	0.19	18					
	17-Oct-95	2		200000	8.7	27-May-98		0		49000	28.5	1-Apr-99	4.82	0.43	37					
	18-Nov-95	3.2		180000			28-May-98	1.7	0.94	85		5-Apr-99	1.44	0.45	62					
	18-Dec-95			180000			3-Jun-98	1.2	0.5		80000	3.3	7-Apr-99	1.35	0.74	53				
	13-Dec-95			210000			3-Jun-98	1.2	0.31	78		13-Apr-99		0.8			4700	2.9		
	4-Jan-96			180000			8-Jun-98	1	3.88	38		15-Apr-99	4.03	0.78	24					
	7-Feb-96			180000			8-Jun-98	1.8		370000		28-Apr-99	3.14	0.89	53					
	5-Mar-96	0.73		8900	4.4	11-Jun-98	1.7	2.5	44			27-Apr-99	2.82	1.18	27					
	2-Apr-96	3.24		59000			15-Jun-98	2.8	0.57	82		11-May-99	7.72	0.41	7					
	24-Apr-96			17000			17-Jun-98	2.74	0.88	56		18-May-99								
	7-May-96	0.32		180000	18	17-Jun-98		0.4		200000	22.1	25-May-99		0.1			8700			
	6-Jun-96	0.33		180000	8	23-Jun-98	2.71	0.82	80			25-May-99	0.6	1.31	38					
	18-Jun-96	1.1		28000	8.1	26-Jun-98		0.8		720000	6.8	1-Jun-99		0.1			5800	2.5		
	3-Jul-96			180000			26-Jun-98	3.1	1.28	84		8-Jun-99		0.3			34000	8.8		
	5-Aug-96			80000			26-Jun-98	7.9	0.29	18		8-Jun-99	0.86	0.29	85					
	11-Sep-96			180000			30-Jun-98		0.8		14	4.3	11-Jun-99	1.11	0.58	40				
	12-Sep-96			110000			6-Jul-98	0.28	0.21	47		15-Jun-99		0.4			5900	8.1		
	8-Oct-96			7000			13-Jul-98			27000		15-Jun-99	2.81	0.88	54					
	24-Oct-96	0.9		72000	10.2	13-Jul-98	2.8	0.85	57			22-Jun-99	1.18	0.38	94					
	29-Nov-96			200			20-Jul-98	8.1	0.32	24		22-Jun-99		0.2			16000	2.8		
	2-Jan-97			180000			23-Jul-98		0.1	710000	13.2	28-Jun-99	4.59	0.97	53					
	5-Feb-97			180000			28-Jul-98		0.2	550000	21.8	28-Jun-99		0.8			20	2.6		
	14-May-97	1.4		95000	20.8	28-Jul-98	1.37	0.85	55			8-Jul-99	2.1	0.29	28					
	14-May-97	4.2		1800000	35.1	4-Aug-98		0.8		300000	7.8	13-Jul-99		0.3			540	4.8		
	9-Sep-97	1		180000	40.2	12-Aug-98	1.88	0.29	32			15-Jul-99	3.18	0.52	20					
	5-Nov-97	3.8		310000	53.4	17-Aug-98		1			0.8	20-Jul-99		0.1			4000	2.4		
	8-Nov-97	0.2		330000	10	18-Aug-98	2.72		25			20-Jul-99		0.3	10					
	7-Nov-97	0.2		80000	8	25-Aug-98		0.8		210000	8	27-Jul-99		0.1			38000	12.7		
	10-Nov-97	0.5		155000	18.1	28-Aug-98	1.8	0.27	58			27-Jul-99	4.3	0.42	52					
	13-Nov-97	0.4		86000	18.2	1-Sep-98		0.1		220000	4	3-Aug-99		0.58	53					
	14-Nov-97	1.2		220000	33.7	2-Sep-98	3.1	0.18	20			3-Aug-99		0.5			87000	4.3		
	17-Nov-97			200000			3-Sep-98			18000		10-Aug-99		0.8			5900	8.8		
	17-Nov-97	2.5		2000000	40.8	14-Sep-98	2.76	0.24	35			13-Aug-99		0.81	88					
	19-Nov-97			820000			15-Sep-98	7.85	0.27	3		17-Aug-99		0.6			8700	3.3		
	19-Nov-97	5.7		520000	114.8	22-Sep-98		0.5		370000	8.1	31-Aug-99		1.1			130000	39		
	21-Nov-97	0.8		800000	15.8	22-Sep-98	1.78	0.35		370000	8.1	13-Sep-99		0.3			20000	4.1		
	21-Nov-97	0.6		500000	15.8	30-Sep-98		0.3		83000	4.4	20-Sep-99		2.2			7800	3.2		
	24-Nov-97			260000	1.8	30-Sep-98		0.32	52	89000	4.4	27-Sep-99		0.8			380	2.9		
	24-Nov-97	1.6		260000			2-Oct-98	1.56	0.38	57		4-Oct-98	0.84	0.42	46					
	26-Nov-97	1.2		43000	10.1	7-Oct-98	2.95	0.33	13			21-Oct-98	2.08	1.22	54					
	28-Nov-97	1.2		43000	10.1	8-Oct-98	3.32	0.72	75			1-Nov-98	4.54	0.83	40					
	1-Dec-97			57000			12-Oct-98	3.88	1.38	87		2-Nov-98					2400			
	1-Dec-97	0.1		57000	5.1	14-Oct-98	3.95	0.78	83	2500	1.5	18-Nov-98								
	10-Dec-97	1.9		840000	28.8	14-Oct-98		0.7		2600	1.5	28-Nov-98								
	10-Dec-97			470000			21-Oct-98	6.05	0.81	25										
	15-Dec-97			270000			28-Oct-98	1.14	1.08	70										
	15-Dec-97	1.3		270000	8.4	28-Oct-98				26000										
	19-Dec-97	0.7		230000	82.7	28-Oct-98	3.11	1.28	78											
	30-Dec-97	1.3		780	3.4	2-Nov-98		0.33	3											
	30-Dec-97	1.3		780	3.4	4-Nov-98		0.3		4100	2.1									
						4-Nov-98	1.119	0.32												
						12-Nov-98	4.82	0.84	45											
						13-Nov-98	7.82	0	3											
						18-Nov-98	8.24	0.88	30											
						19-Nov-98	9.81	0	40											
						18-Nov-98		0.3		13000	4.7									
						25-Nov-98	2.08	0.51	75											
						25-Nov-98	2.85	0.27	53											

Polk and 66th											
1998						1999					
Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)	Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)
8-Sep-97						6-Jan-99	5.57	1.39	70		
2-Apr-98						13-Jan-99	3.55	0.96	75		
28-Apr-98						22-Jan-99	3.55	0.88	69		
12-May-98						26-Jan-99	3.33	1.45	61		
21-May-98						4-Feb-99	6.34	0.84	69		
27-May-98						5-Feb-99	2.08	1.52	60		
28-May-98						12-Feb-99		2.64	62		
3-Jun-98						18-Feb-99	4.88	0.47	23		
8-Jun-98						26-Feb-99	2.38	0.83	52		
11-Jun-98						5-Mar-99	1.11	1.11	35		
15-Jun-98						11-Mar-99	1.01	0.85	47		
17-Jun-98	2.17	1.06	50			16-Mar-99	7.41	0.87	55		
23-Jun-98	2.4	0.76	62			25-Mar-99	5.2	0.59	55		
26-Jun-98	2.6	1.15	69			1-Apr-99	6.6	0.61	43		
29-Jun-98	7.2	0.37	25			7-Apr-99	0.72	0.88	55		
8-Jul-98	0.285	0.11	49			15-Apr-99	6.26	0.86	26		
13-Jul-98	2.3	1.46	42			23-Apr-99	2.39	0.49	55		
20-Jul-98	6.1	0.33	29			27-Apr-99	4.56	1.23	28		
29-Jul-98	2.84	1.09	50			18-May-99	4.57	0.96	55		
12-Aug-98	2.76	0.53	40			11-Jun-99	2.91	0.78	39		
19-Aug-98	4.08	0.45	26			15-Jun-99	2.76	0.71	42		
28-Aug-98	2.04	1.46	14			22-Jun-99	2.11	0.49	35		
31-Aug-98	2.42	1.82	55			29-Jun-99	3.5	1.23	51		
14-Sep-98	2.75	0.87	47			6-Jul-99	1.85	0.97	32		
18-Sep-98	1.6	1.85	57			15-Jul-99	4.76	0.31	21		
23-Sep-98	1.95	1.74				20-Jul-99	4.1	0.46	16		
2-Oct-98	3.4	1.27	62			27-Jul-99	6.3	0.78	47		
9-Oct-98	5.9	0.09	73			3-Aug-99		0.88	56		
12-Oct-98	6.3	2.46	48			13-Aug-99		0.62	66		
29-Oct-98	3.52	1.33	78			4-Oct-99	3.24	0.77	47		
2-Nov-98		0.56	7			21-Oct-99	3.28	1.13	49		
12-Nov-98	9.2	0.39	44			1-Nov-99	3.78	0.99	44		
16-Nov-98	7.67	0.54	30								
25-Nov-98	1.87	0.65	66								
23-Dec-98	5.18	0	57								
30-Dec-98	6.7	0.38	63								
Mean 1998	3.88	0.91	47.63								
Mean 1999	3.80	0.92	48.13								

Yates		1998					1999				
Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)	Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)
8-Sep-97						6-Jan-99	7.65	1.17	64		
2-Apr-98						13-Jan-99	4.79	2.17	60		
28-Apr-98						22-Jan-99	3.52	0.37	62		
12-May-98						26-Jan-99	4.93	0.42	58		
21-May-98						4-Feb-99	4.78	1.24	66		
27-May-98						5-Feb-99	3.59	0.32	64		
28-May-98						12-Feb-99		0.56	63		
3-Jun-98						18-Feb-99	4.61	0.3	41		
8-Jun-98						26-Feb-99	6.72	0.39	70		
11-Jun-98						5-Mar-99	2.61	0.43	46		
15-Jun-98	3.6	1.38	65			11-Mar-99	2	0.86	34		
17-Jun-98	4.12	1.26	58			16-Mar-99	7.55	0.86	61		
22-Jun-98	4.2	1.06	65			25-Mar-99	2.91	0.52	48		
26-Jun-98	3.3	0.8	61			1-Apr-99	6.14	1.08	48		
29-Jun-98	5.6	0.26	32			7-Apr-99	9.3	2	34		
8-Jul-98	0.537	0.11	62			15-Apr-99	7.35	0.31	30		
13-Jul-98	4.5	0.16	59			23-Apr-99	6.65	0.89	49		
20-Jul-98	3.7	0.5	31			27-Apr-99	6.87	0.69	19		
29-Jul-98	4.16	0.03	54			18-May-99	7.74	0.28	55		
12-Aug-98	6.32	1.54	44			11-Jun-99	4.02	0.67	29		
19-Aug-98	4.25		45			15-Jun-99	2.72	1.84	47		
28-Aug-98	3.76	0.23	47			22-Jun-99	1.38	0.44	25		
31-Aug-98	1.78	0.25	49			29-Jun-99	2.14	1	43		
14-Sep-98						6-Jul-99	1.99	2.75	30		
18-Sep-98	4.25	0.51	60			15-Jul-99	1.18	0.31	43		
23-Sep-98	4	0.7				20-Jul-99	5.2	0.71	41		
2-Oct-98	4.7	0.47	67			27-Jul-99	4.2	0.74	29		
9-Oct-98	11.62	1.19	67			3-Aug-99		0.8	29		
12-Oct-98	3.42	0.21	48			13-Aug-99		0.1	36		
29-Oct-98	3.78	0.41	45			4-Oct-99	3.33	0.26	51		
2-Nov-98		0.3	24			21-Oct-99	3.13	0.12	36		
12-Nov-98	5.08	0.67	54			1-Nov-99	3.93	0.58	28		
16-Nov-98	4.35	0.59	0								
25-Nov-98	1.85	1.13	54								
23-Dec-98	8.76	2.42	47								
30-Dec-98	11.6	0.83	68								
Mean 1998	4.72	0.71	50.25								
Mean 1999	4.58	0.79	44.97								

Wayside		1998				1999					
Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)	Field Visit Date	DO (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100mL)	BOD (mg/L)
8-Sep-97						6-Jan-99	9.84	1.02	63		
2-Apr-98						13-Jan-99	12.66	0.94	71		
28-Apr-98						22-Jan-99	3.86	0.63	64		
12-May-98						26-Jan-99	7.45	1.64	70		
21-May-98						4-Feb-99	7.3				
27-May-98						5-Feb-99	3.02	1.17	69		
28-May-98						12-Feb-99	1.95	2.14	52		
3-Jun-98						26-Feb-99	2.97	0.5	53		
8-Jun-98						5-Mar-99	2.34	0.66	46		
11-Jun-98						11-Mar-99	1.78	1.2	44		
15-Jun-98						16-Mar-99	5.46	0.47	57		
17-Jun-98						25-Mar-99	5.38	0.37	54		
22-Jun-98						1-Apr-99	5.36	0.56	42		
26-Jun-98						7-Apr-99	1.49	0.71	53		
29-Jun-98						15-Apr-99	6.61	0.73	32		
8-Jul-98						23-Apr-99	6.29	0.49	60		
13-Jul-98						27-Apr-99	4.82	1.25	36		
20-Jul-98						18-May-99	4.09	0.29	60		
29-Jul-98						11-Jun-99	2.19	0.82	35		
12-Aug-98						15-Jun-99	2.52	0.66	30		
19-Aug-98						22-Jun-99	2.85	0.55	27		
28-Aug-98	2.18	1.56	60			29-Jun-99	3.34	1.09	49		
31-Aug-98	1.53	1.89	56			6-Jul-99	2.23	0	43		
14-Sep-98	2.25	0.63	46			15-Jul-99	3.98	0.86	31		
18-Sep-98	1.25	1.22	56			20-Jul-99	4	0.78	24		
23-Sep-98	2.2	1.37				27-Jul-99	4.6	0.61	50		
2-Oct-98	3.08	1.27	69			3-Aug-99		0.45	62		
9-Oct-98	5.4	0.94	72			13-Aug-99		0.7	71		
12-Oct-98	2.51	1.94	74			4-Oct-99	2.71	0.65	42		
29-Oct-98	6.72	1.07	78			21-Oct-99	4.14	0.61	51		
2-Nov-98		0.34	10								
12-Nov-98	4.66	0.31	40								
16-Nov-98	6.88	0.58	35								
25-Nov-98	1.72	0.79	48								
23-Dec-98	2.94	0.04	73								
30-Dec-98	7.92	0.52	66								
Mean 1998	3.66	0.96	55.93								
Mean 1999	4.47	0.78	49.69								

Upstream Locations - All Upstream													
Location	Field Visit Date	DO (ppm)	Ammonia_NH303 (mg/L)	Sulfide_SH001 (mg/L)	Fecal Coliform (nuff/100ml)	BOC (mg/L)	Location	Field Visit Date	DO (ppm)	Ammonia_NH303 (mg/L)	Sulfide_SH001 (mg/L)	Fecal Coliform (nuff/100ml)	BOC (mg/L)
Evergreen Cemetery	8-Sep-97						Park and Elm N1	6-Jan-99	5.64	2.44	72		
Evergreen Cemetery	2-Apr-98						Park and Elm S1	6-Jan-99	4.49	0.04	74		
Evergreen Cemetery	26-Apr-98						Park and Elm N1	6-Jan-99	0.1	2.3		61000	7.2
Evergreen Cemetery	12-May-98						Park and Elm N1	13-Jan-99	8.06	2.66	86		7.6
Evergreen Cemetery	21-May-98						Park and Elm S1	13-Jan-99	13.02	0.49	72		
Evergreen Cemetery	27-May-98						Park and Elm N1	13-Jan-99	3.4			2300000	11.6
Evergreen Cemetery	3-Jun-98						Park and Elm S1	13-Jan-99	0.6			4200	3.9
Evergreen Cemetery	9-Jun-98						Park and Elm N1	21-Jan-99	5.38	2.45	59		
Evergreen Cemetery	11-Jun-98						Park and Elm S1	21-Jan-99	7.69	0.62	74		
Evergreen Cemetery	15-Jun-98						Park and Elm N1	2-Feb-99	0.92	0.85	56		
Evergreen Cemetery	17-Jun-98						Park and Elm S1	2-Feb-99	0.59	0.27	72		
Evergreen Cemetery	23-Jun-98						Park and Elm N1	2-Feb-99				33000	42
Evergreen Cemetery	26-Jun-98						Park and Elm S1	2-Feb-99	0.2			28000	5.7
Evergreen Cemetery	28-Jun-98						Park and Elm N1	9-Feb-99	3.87	1.44	66		
Evergreen Cemetery	6-Jul-98						Park and Elm S1	9-Feb-99	4.53	1.34	59		
Evergreen Cemetery	13-Jul-98						Park and Elm N1	9-Feb-99	0.1			42000	39
Evergreen Cemetery	20-Jul-98						Park and Elm S1	9-Feb-99	0.9			270	11.6
Evergreen Cemetery	29-Jul-98	2.7	0.47	53			Park and Elm N1	16-Feb-99	2.61	4.1	36		
Evergreen Cemetery	12-Aug-98	4.09	0.85	53			Park and Elm S1	16-Feb-99	4.4	0.82	76		
Evergreen Cemetery	19-Aug-98	1.7		41			Park and Elm N1	16-Feb-99		0.5		32000	400
Evergreen Cemetery	26-Aug-98	1.24	0.57	67			Park and Elm S1	16-Feb-99		0.9		15000	29.2
Evergreen Cemetery	5-Sep-98	3.98	0.4	30			Park and Elm N1	23-Feb-99	5.01	1.74	56		
Evergreen Cemetery	14-Sep-98	2.76					Park and Elm S1	23-Feb-99	4.59	1.07	73		
Evergreen Cemetery	16-Sep-98	4.9	0.28	27			Park and Elm N1	23-Feb-99	0.1			11000	40
Park and Elm N1	22-Sep-98	2.61	0.45		4600	2.1	Park and Elm S1	23-Feb-99	0.9			140000	1560
Park and Elm S1	22-Sep-98	3.54	0.96		690	4.6	Park and Elm N1	2-Mar-99	2.62	4.68	61		
Park and Elm N1	22-Sep-98		0.2		4600	2.1	Park and Elm S1	2-Mar-99	3.86	1.17	73		
Park and Elm S1	22-Sep-98		0.8		690	4.6	Park and Elm N1	2-Mar-99		0.7		33000	113.4
Evergreen Cemetery	23-Sep-98	1.64	0.53	73			Park and Elm S1	2-Mar-99		0.1		7200	700
Park and Elm S1	23-Sep-98	1.34	0.71	4300	32.6		Park and Elm N1	9-Mar-99	5.89	0.87	54		
Evergreen Cemetery	30-Sep-98	0.82	59	20000	27.1		Park and Elm S1	9-Mar-99	2.82	0.82	61		
Park and Elm N1	30-Sep-98	2.99	1.13	70	45000	44.7	Park and Elm N1	9-Mar-99	0.4			999	44.1
Evergreen Cemetery	30-Sep-98	0.6		20000	27.1		Park and Elm S1	9-Mar-99	0.8			999	136.5
Park and Elm N1	30-Sep-98	0.1		45000	44.7		Park and Elm N1	16-Mar-99	6.51	0.4	0.3	94000	
Park and Elm S1	30-Sep-98	0.9		4300	32.6		Park and Elm S1	16-Mar-99	4.94	0.82	62	22000	
Evergreen Cemetery	7-Oct-98	1.28	0.46	50			Park and Elm N1	16-Mar-99				94000	
Park and Elm N1	7-Oct-98	2.85	1.39	42			Park and Elm S1	16-Mar-99				22000	
Park and Elm S1	7-Oct-98	2.94	0.85	59			Park and Elm N1	23-Mar-99	6.56	0.32	69		
Park and Elm N1	7-Oct-98						Park and Elm S1	23-Mar-99	3.87	0.32	56		
Evergreen Cemetery	14-Oct-98	1.76	1.19	65	20000	4	Park and Elm N1	23-Mar-99	0.1			94000	8.8
Park and Elm N1	14-Oct-98	1.25	0.32	30	5400	4.9	Park and Elm S1	23-Mar-99	0.2			21000	129.2
Park and Elm S1	14-Oct-98	4.3	1	71	2800	2.3	Park and Elm N1	30-Mar-99	6.38	0.37	16		
Evergreen Cemetery	14-Oct-98		0.8		20000	4	Park and Elm S1	30-Mar-99	6.15	0	24		
Park and Elm N1	14-Oct-98		0.3		5400	4.9	Park and Elm N1	5-Apr-99	4.34	1.99	58		
Park and Elm S1	14-Oct-98		0.7		2800	2.3	Park and Elm S1	5-Apr-99	1.19	1.17	70		
Evergreen Cemetery	21-Oct-98	5.44	0.49	55			Park and Elm N1	13-Apr-99				250000	
Park and Elm N1	21-Oct-98	8.02	0.16	52			Park and Elm S1	13-Apr-99	0.4			490	16.4
Park and Elm S1	21-Oct-98	9.39	0.5	60			Park and Elm N1	15-Apr-99	6.62	0	53		
Park and Elm N1	21-Oct-98						Park and Elm S1	23-Apr-99	7.24	0.43	75		
Evergreen Cemetery	28-Oct-98	6.634	2.13	72			Park and Elm N1	27-Apr-99	6.14	0.46	42		
Park and Elm N1	28-Oct-98	10.29	1.27	88			Park and Elm S1	11-May-99	6.27	0.19	38		
Park and Elm S1	28-Oct-98	9.27	1.42	64			Park and Elm N1	18-May-99					
Evergreen Cemetery	28-Oct-98				22000		Park and Elm S1	25-May-99	7.6	1.03	66	260000	44
Park and Elm N1	28-Oct-98				72		Park and Elm N1	25-May-99				30000	18
Park and Elm S1	28-Oct-98						Park and Elm S1	25-May-99				680	0.9
Evergreen Cemetery	4-Nov-98	1.7	0.21	63			Park and Elm N1	1-Jun-99				5300	2.6
Park and Elm N1	4-Nov-98	10.3	0.12	70			Park and Elm S1	1-Jun-99					
Park and Elm S1	4-Nov-98	5.4	0.79	60			Park and Elm N1	6-Jun-99	4.33	0	59		
Evergreen Cemetery	4-Nov-98	0.2		52000	3.9		Park and Elm S1	6-Jun-99	6.33	0.53	63	3000	9.9
Park and Elm N1	4-Nov-98	0.1		11000	1.2		Park and Elm N1	8-Jun-99				11000	31
Park and Elm S1	4-Nov-98	0.5		7700	4.8		Park and Elm S1	8-Jun-99					
Evergreen Cemetery	13-Nov-98	6.33	0.58	0			Park and Elm N1	15-Jun-99	2.83	0.28	42		
Park and Elm N1	13-Nov-98	8.63	0.55	7			Park and Elm S1	15-Jun-99	2.96	1.09	55		
Park and Elm S1	13-Nov-98	5.72	0.62	14			Park and Elm N1	15-Jun-99				4000	3.2
Evergreen Cemetery	18-Nov-98	8.3	0.57	57			Park and Elm S1	15-Jun-99				20	38.5
Park and Elm N1	18-Nov-98	8.38	0.1	32			Park and Elm N1	22-Jun-99					
Park and Elm S1	18-Nov-98	7.8	0.58	46			Park and Elm S1	22-Jun-99	1.22	0.67	60		
Evergreen Cemetery	18-Nov-98	0.2		120000	5.4		Park and Elm N1	22-Jun-99				4400	3.6
Park and Elm N1	18-Nov-98	0.1		8800	3.9		Park and Elm S1	22-Jun-99				4800	11.9
Park and Elm S1	18-Nov-98	0.2		340	3		Park and Elm N1	29-Jun-99	4.93	1.11	49		
Evergreen Cemetery	25-Nov-98	4.6	1.31	68			Park and Elm S1	29-Jun-99	2.72	0.62	61		
Park and Elm N1	25-Nov-98	4.53	0.1	46			Park and Elm N1	29-Jun-99				10	2.1
Park and Elm S1	25-Nov-98	6.3	0.97	72			Park and Elm S1	29-Jun-99				820	15.9
Evergreen Cemetery	2-Dec-98	12.76	0.43	20			Park and Elm N1	6-Jul-99					
Park and Elm N1	2-Dec-98	6.61	0.24	42			Park and Elm S1	6-Jul-99	1.29	1.37	31		
Park and Elm S1	2-Dec-98	4.63	0.26	53			Park and Elm N1	13-Jul-99				240	0.7
Evergreen Cemetery	2-Dec-98	0.2		13000	5.5		Park and Elm S1	13-Jul-99				3000	40
Park and Elm N1	2-Dec-98	0.1		22000	0.8		Park and Elm N1	15-Jul-99					
Park and Elm S1	2-Dec-98	0.2		24000	3.5		Park and Elm S1	15-Jul-99	3.3	0.16	64		
Evergreen Cemetery	9-Dec-98	0.3		190000	24.7		Park and Elm N1	20-Jul-99	4.5	0.23	51		
Park and Elm N1	9-Dec-98	0.1		40000	23.1		Park and Elm S1	20-Jul-99				300	1.1
Park and Elm S1	9-Dec-98	0.5		14000	5.1		Park and Elm N1	20-Jul-99				3600	2.8
Evergreen Cemetery	17-Dec-98	5.58	0.37	69			Park and Elm S1	27-Jul-99	6.4	0.17	73		
Park and Elm N1	17-Dec-98	7.14	0.39	66			Park and Elm N1	27-Jul-99	4.8	0.25	65		
Park and Elm S1	17-Dec-98	0.25	66				Park and Elm S1	27-Jul-99				2600	2.8
Evergreen Cemetery	22-Dec-98	0.1		45000	41		Park and Elm N1	27-Jul-99				4100	4.1
Park and Elm N1	22-Dec-98	0.1		7300	10.9		Park and Elm S1	3-Aug-99		0.3	62		
Park and Elm S1	22-Dec-98			49000	42		Park and Elm N1	3-Aug-99		0.34	84		
Evergreen Cemetery	23-Dec-98	7.23	0.13	72			Park and Elm S1	3-Aug-99				2400	2.8
Park and Elm N1	23-Dec-98	7.21	1	86			Park and Elm N1	3-Aug-99				16000	4.2
Park and Elm S1	23-Dec-98	7.7	0.11	47			Park and Elm S1	10-Aug-99				23000	5.4
Evergreen Cemetery	30-Dec-98	8.73	1.21	64			Park and Elm N1	10-Aug-99				5400	2.1
Park and Elm N1	30-Dec-98	10.59	1.02	44			Park and Elm S1	13-Aug-99	0.39	64			
Park and Elm S1	30-Dec-98	0.64	74				Park and Elm N1	17-Aug-99				60000	22.7
Evergreen Cemetery	30-Dec-98	0.7		1	22		Park and Elm S1	17-Aug-99				340	0.1
Park and Elm N1	30-Dec-98	0.21		3300	3		Park and Elm N1	31-Aug-99				3800	2.8
Park and Elm S1	30-Dec-98	0.5			2		Park and Elm S1	31-Aug-99				3400	4.7
							Park and Elm N1	13-Sep-99				2700	1.4
							Park and Elm S1	13-Sep-99				10	3.7
							Park and Elm N1	20-Sep-99				940	3.5
							Park and Elm S1	20-Sep-99				20000	4.4
							Park and Elm N1	27-Sep-99				720	
							Park and Elm S1	27-Sep-99				2500	26.1
							Park and Elm N1	4-Oct-99	2.62	0.01	48		
							Park and Elm S1	21-Oct-99	3.93	0.48	45		
							Park and Elm N1	1-Nov-99	4.87	0.9	51		
							Park and Elm S1	29-Nov-99				24000	
							Park and Elm N1	29-Nov-99				9	
Mean Values 1998		5.56	0.56	53.70	47.431	12.66							
Mean Values 1999		4.86	0.85	67.19	80.608	72.30							

Upstream Odor							Upstream No Odor						
Location	Field Visit Date	DO (ppm)	Ammonia_Mg035 (mg/L)	Sulfate_Mg051 (mg/L)	Fecal Coliform (cfu/100ml)	BOD (mg/L)	Location	Field Visit Date	DO (ppm)	Ammonia_Mg035 (mg/L)	Sulfate_Mg051 (mg/L)	Fecal Coliform (cfu/100ml)	BOD (mg/L)
Evergreen Cemetery	2-Apr-98						Polk and Elm S1	1-Nov-99	4.87	0.9	51		
Evergreen Cemetery	28-Apr-98						Evergreen Cemetery	28-May-98					
Evergreen Cemetery	12-May-98						Evergreen Cemetery	3-Jun-98					
Evergreen Cemetery	12-May-98						Evergreen Cemetery	8-Jun-98					
Evergreen Cemetery	21-May-98						Evergreen Cemetery	15-Jun-98					
Evergreen Cemetery	27-May-98						Evergreen Cemetery	26-Jun-98					
Evergreen Cemetery	11-Jun-98						Evergreen Cemetery	29-Jun-98					
Evergreen Cemetery	17-Jun-98						Evergreen Cemetery	8-Jul-98					
Evergreen Cemetery	22-Jun-98						Evergreen Cemetery	20-Jul-98					
Evergreen Cemetery	13-Jul-98						Evergreen Cemetery	29-Jul-98	2.7	0.47	53		
Evergreen Cemetery	26-Aug-98	1.24	0.57	67			Evergreen Cemetery	12-Aug-98	4.08	0.85	53		
Evergreen Cemetery	25-Nov-98	4.8	1.31	88			Evergreen Cemetery	19-Aug-98	1.7		41		
Evergreen Cemetery	25-Nov-98	4.8	1.31	88			Evergreen Cemetery	14-Sep-98	2.78				
Evergreen Cemetery	2-Dec-98	12.78	0.43	20			Evergreen Cemetery	16-Sep-98	4.9	0.28	27		
Evergreen Cemetery	23-Dec-98	7.23	0.13	72			Park and Elm N1	22-Sep-98	0.2			4800	2.1
Evergreen Cemetery	30-Dec-98	8.73	1.21	64			Polk and Elm S1	22-Sep-98	0.6			890	4.8
Park and Elm N1	25-Nov-98	4.93	0.1	46			Park and Elm N1	22-Sep-98	2.61	0.45		4800	2.1
Park and Elm N1	25-Nov-98	4.93	0.1	46			Polk and Elm S1	22-Sep-98	3.54	0.98		890	4.8
Park and Elm N1	2-Dec-98	0.61	0.24	42			Evergreen Cemetery	30-Sep-98	0.6			200000	27.1
Park and Elm N1	23-Dec-98	7.21	1	86			Park and Elm N1	30-Sep-98	0.1			45000	44.7
Park and Elm N1	30-Dec-98	10.59	1.02	44			Park and Elm N1	30-Sep-98	2.99	1.13	70	45000	44.7
Park and Elm N1	6-Jan-99	5.64	2.44	72			Polk and Elm S1	30-Sep-98	0.9			4300	32.5
Park and Elm N1	21-Jan-99	5.38	2.45	59			Evergreen Cemetery	30-Sep-98	0.82			200000	27.1
Park and Elm N1	2-Feb-99	0.92	0.85	56			Evergreen Cemetery	7-Oct-98	1.28	0.48	59		
Park and Elm N1	9-Feb-99	3.87	1.44	55			Park and Elm N1	7-Oct-98	2.88	1.39	42		
Park and Elm N1	16-Feb-99	2.61	4.1	36			Park and Elm N1	7-Oct-98					
Park and Elm N1	2-Mar-99	2.52	4.68	51			Polk and Elm S1	7-Oct-98	2.94	0.55	59		
Park and Elm N1	9-Mar-99	5.89	0.87	54			Park and Elm N1	14-Oct-98	1.25	0.32	30	5400	4.9
Park and Elm N1	23-Mar-99	6.55	0.32	99			Park and Elm N1	14-Oct-98	0.3			5400	4.9
Park and Elm N1	30-Mar-99	8.38	0.37	18			Polk and Elm S1	14-Oct-98	4.3	1	71	2800	2.3
Park and Elm N1	5-Apr-99	4.34	1.99	58			Evergreen Cemetery	14-Oct-98	0.8			20000	4
Park and Elm N1	8-Jun-99	4.33	0	59			Evergreen Cemetery	14-Oct-98	1.76	1.19	65	20000	4
Park and Elm N1	29-Jun-99	4.93	1.11	49			Polk and Elm S1	14-Oct-98	0.7			2800	2.3
Park and Elm N1	27-Jul-99	6.4	0.17	73			Park and Elm N1	21-Oct-98	8.02	0.15	52		
Polk and Elm S1	25-Nov-98	6.3	0.97	72			Polk and Elm S1	21-Oct-98	8.39	0.5	60		
Polk and Elm S1	25-Nov-98	8.3	0.97	72			Park and Elm N1	21-Oct-98					
Polk and Elm S1	2-Dec-98	4.63	0.28	53			Evergreen Cemetery	21-Oct-98	5.44	0.49	55		
Polk and Elm S1	23-Dec-98	7.7	0.11	47			Polk and Elm S1	28-Oct-98	9.27	1.42	64		
Polk and Elm S1	30-Dec-98		0.64	74			Park and Elm N1	28-Oct-98	10.29	1.27	88		
Polk and Elm S1	8-Jan-99	4.49	0.04	74			Evergreen Cemetery	28-Oct-98	8.634	2.13	72		
Polk and Elm S1	21-Jan-99	7.59	0.62	74			Park and Elm N1	4-Nov-98	0.1			11000	1.2
Polk and Elm S1	2-Feb-99	0.59	0.27	72			Evergreen Cemetery	4-Nov-98	1.7	0.21	63		
Polk and Elm S1	9-Feb-99	4.53	1.34	59			Polk and Elm S1	4-Nov-98	0.5			7700	4.8
Polk and Elm S1	16-Feb-99	4.4	0.92	76			Polk and Elm S1	4-Nov-98	5.4	0.79	60		
Polk and Elm S1	2-Mar-99	3.86	1.17	73			Park and Elm N1	4-Nov-98	10.3	0.12	70		
Polk and Elm S1	9-Mar-99	2.82	0.62	61			Evergreen Cemetery	4-Nov-98	0.2			520000	3.9
Polk and Elm S1	23-Mar-99	3.87	0.32	56			Evergreen Cemetery	13-Nov-98	6.33	0.58	0		
Polk and Elm S1	30-Mar-99	8.15	0	24			Polk and Elm S1	13-Nov-98	5.72	0.62	14		
Polk and Elm S1	5-Apr-99	1.16	1.17	70			Park and Elm N1	13-Nov-98	8.63	0.55	7		
Polk and Elm S1	23-Apr-99	7.24	0.43	75			Evergreen Cemetery	18-Nov-98	0.2			120000	5.4
Polk and Elm S1	27-Apr-99	6.14	0.48	42			Polk and Elm S1	18-Nov-98	0.2			340	3
Polk and Elm S1	25-May-99	7.6	1.03	66			Park and Elm N1	18-Nov-98	0.1			5800	3.9
Polk and Elm S1	8-Jun-99	6.33	0.53	63			Polk and Elm S1	18-Nov-98	7.8	0.58	46		
Polk and Elm S1	29-Jun-99	2.72	0.82	61			Evergreen Cemetery	18-Nov-98	8.3	0.57	57		
Polk and Elm S1	27-Jul-99	4.8	0.25	65			Park and Elm N1	18-Nov-98	5.38	0.1	32		
Polk and Elm S1	4-Oct-99	2.62	0.01	48			Polk and Elm S1	13-Jan-99	13.02	0.49	72		
Evergreen Cemetery	29-Oct-98						Park and Elm N1	13-Jan-99	3.4			2300000	11.8
Evergreen Cemetery	2-Dec-98		0.2	13000		5.5	Polk and Elm S1	13-Jan-99	0.6			4200	3.9
Evergreen Cemetery	30-Dec-98		0.7	1		2.2	Park and Elm N1	13-Jan-99	8.68	2.66	85		
Park and Elm N1	29-Oct-98			22000			Polk and Elm S1	23-Feb-99	4.59	1.07	73		
Park and Elm N1	2-Dec-98	0.1		22000		0.8	Park and Elm N1	23-Feb-99	0.1			11000	40
Park and Elm N1	2-Feb-99			33000		42	Park and Elm N1	23-Feb-99	5.01	1.74	66		
Park and Elm N1	9-Feb-99	0.1		42000		39	Polk and Elm S1	23-Feb-99	0.9			140000	1550
Park and Elm N1	16-Feb-99	0.5		32000		400	Park and Elm N1	16-Mar-99				94000	
Park and Elm N1	9-Mar-99	0.4				44.1	Polk and Elm S1	16-Mar-99				22000	
Park and Elm N1	23-Mar-99	0.1		94000		8.8	Park and Elm N1	18-Mar-99	6.51	0.4	0.3	94000	
Park and Elm N1	25-May-99			280000		44	Polk and Elm S1	18-Mar-99	4.94	0.82	62	22000	
Park and Elm N1	8-Jun-99			3000		8.9	Polk and Elm S1	15-Apr-99	6.82	0	53		
Park and Elm N1	29-Jun-99			10		2.1	Polk and Elm S1	11-May-99	8.27	0.19	38		
Park and Elm N1	27-Jul-99			2800		2.8	Polk and Elm S1	15-Jun-99				20	38.5
Polk and Elm S1	29-Oct-98			72			Park and Elm N1	15-Jun-99				4000	3.2
Polk and Elm S1	2-Dec-98	0.2		24000		3.5	Park and Elm N1	15-Jun-99	2.83	0.28	42		
Polk and Elm S1	30-Dec-98	0.2		3300		3	Polk and Elm S1	15-Jun-99	2.96	1.08	55		
Polk and Elm S1	8-Jan-99	0.1		51000		7.2	Park and Elm N1	22-Jun-99				4400	3.6
Polk and Elm S1	2-Feb-99	0.2		28000		5.7	Polk and Elm S1	22-Jun-99	1.22	0.67	50		
Polk and Elm S1	9-Feb-99	0.9		270		11.6	Polk and Elm S1	22-Jun-99				4800	11.9
Polk and Elm S1	16-Feb-99	0.9		15000		29.2	Park and Elm N1	22-Jun-99					
Polk and Elm S1	2-Mar-99	0.7		33000		113.4	Polk and Elm S1	6-Jul-99	1.29	1.37	31		
Polk and Elm S1	9-Mar-99	0.8				136.5	Park and Elm N1	6-Jul-99					
Polk and Elm S1	23-Mar-99	0.2		21000		125.2	Park and Elm N1	15-Jul-99					
Polk and Elm S1	25-May-99			30000		18	Polk and Elm S1	15-Jul-99	3.3	0.15	64		
Polk and Elm S1	8-Jun-99			11000		37	Polk and Elm S1	20-Jul-99				3800	2.8
Polk and Elm S1	29-Jun-99			820		15.9	Park and Elm N1	20-Jul-99				300	1.1
Polk and Elm S1	27-Jul-99			4100		4.1	Polk and Elm S1	20-Jul-99	4.5	0.23	51		
Park and Elm N1	30-Dec-98		0.5			2	Park and Elm N1	3-Aug-99				2400	2.8
Park and Elm N1	8-Jan-99	2.3		880000		7.6	Polk and Elm S1	3-Aug-99		0.34	84		
Park and Elm N1	2-Mar-99		0.1	7200		700	Park and Elm N1	3-Aug-99		0.3	62		
							Polk and Elm S1	3-Aug-99				18000	4.2
							Polk and Elm S1	13-Aug-99		0.39	84		
							Polk and Elm S1	21-Oct-99	3.93	0.46	45		
Upstream Odor		5.31	0.77	58.39	60,458	65.00							
Upstream No Odor		5.16	0.68	52.12	108,725	57.81							

Downstream Locations - All Stations													
Location	Field Visit Date	DO (ppm)	Ammonia_NH3 (mg/L)	Sulfide_H2S (mg/L)	Fecal Coliform (du/100mL)	BOD (mg/L)	Location	Field Visit Date	DO (ppm)	Ammonia_NH3 (mg/L)	Sulfide_H2S (mg/L)	Fecal Coliform (du/100mL)	BOD (mg/L)
Hughes Street Bridge	16-Feb-96				160000		Hughes Street Bridge	6-Jan-99	3.86	1.26		87	
Hughes Street Bridge	30-Mar-96				160000		Polk and 66th	6-Jan-99	5.87	1.39		70	
Hughes Street Bridge	15-Jun-96				160000		Hughes Street Bridge	6-Jan-99				470000	26.8
Hughes Street Bridge	28-Sep-92				160000		Hughes Street Bridge	13-Jan-99	3.42	0.79		71	
Hughes Street Bridge	24-Feb-93		0.7		170	1.6	Polk and 66th	13-Jan-99	3.56	0.96		75	
Hughes Street Bridge	18-Apr-93				160000		Hughes Street Bridge	13-Jan-99		0.9		1200000	10.9
Hughes Street Bridge	22-Apr-93		0.2		12000	2	Hughes Street Bridge	21-Jan-99	3.93	0.38		66	
Hughes Street Bridge	29-Jun-93				160000		Hughes Street Bridge	22-Jan-99	2.04	1.24		68	
Hughes Street Bridge	15-Jul-93				690000		Polk and 66th	22-Jan-99	3.56	0.88		89	
Hughes Street Bridge	22-Jul-93				1300000		Hughes Street Bridge	26-Jan-99	1.16	2.13		83	
Hughes Street Bridge	4-Aug-93				160000		Polk and 66th	26-Jan-99	3.33	1.45		61	
Hughes Street Bridge	17-Aug-93		0.9		660000	4.4	Hughes Street Bridge	2-Feb-99	2.54	1.19		58	
Hughes Street Bridge	25-Oct-93		1		9	1.5	Hughes Street Bridge	2-Feb-99		0.9		8100	4.5
Hughes Street Bridge	29-Oct-93				30000		Hughes Street Bridge	4-Feb-99	2.22	1.64		59	
Hughes Street Bridge	17-Feb-94				200		Polk and 66th	4-Feb-99	6.34	0.84		69	
Hughes Street Bridge	31-Mar-94		1.6		7700	2.4	Hughes Street Bridge	6-Feb-99	0.26	1.49		53	
Hughes Street Bridge	12-Apr-94				160000		Polk and 66th	5-Feb-99	2.06	1.52		60	
Hughes Street Bridge	27-Sep-94		1		3300	1.9	Hughes Street Bridge	9-Feb-99	3.29	0.98		48	
Hughes Street Bridge	2-Nov-94		1.2		485000	2.7	Hughes Street Bridge	9-Feb-99		1.4		69000	
Hughes Street Bridge	13-Dec-94				160000		Hughes Street Bridge	12-Feb-99		2.12		47	
Hughes Street Bridge	25-Jan-95				22000		Polk and 66th	12-Feb-99		2.64		62	
Hughes Street Bridge	6-Feb-95		1.4		100000	6.4	Hughes Street Bridge	16-Feb-99	5.45	1.55		58	
Hughes Street Bridge	23-Mar-95				160000		Hughes Street Bridge	16-Feb-99		2		450	2.5
Hughes Street Bridge	1-May-95		2.5		1415000	5.5	Hughes Street Bridge	18-Feb-99	4.56	0.47		9	
Hughes Street Bridge	13-May-95				200000		Polk and 66th	18-Feb-99	4.88	0.47		23	
Hughes Street Bridge	23-May-95				160000		Hughes Street Bridge	23-Feb-99	1.83	0.15		26	
Hughes Street Bridge	16-Jun-95				2000000		Hughes Street Bridge	23-Feb-99		0.4		1400	3.4
Hughes Street Bridge	29-Jun-95				160000		Polk and 66th	26-Feb-99	1.43	0.35		51	
Hughes Street Bridge	8-Aug-95		0.2		150000	3.3	Polk and 66th	26-Feb-99	2.38	0.83		82	
Hughes Street Bridge	22-Aug-95				86000		Hughes Street Bridge	2-Mar-99	1.3	0.74		48	
Hughes Street Bridge	20-Sep-95				2300		Hughes Street Bridge	2-Mar-99		0.2		220000	121.7
Hughes Street Bridge	5-Oct-95				160000		Hughes Street Bridge	5-Mar-99	1.28	1.41		41	
Hughes Street Bridge	17-Oct-95		2		200000	6.7	Polk and 66th	6-Mar-99	1.11	1.11		35	
Hughes Street Bridge	15-Nov-95		3.2		160000		Hughes Street Bridge	9-Mar-99	1.3	0.73		38	
Hughes Street Bridge	13-Dec-95				160000		Hughes Street Bridge	9-Mar-99		0.6			13.1
Hughes Street Bridge	13-Dec-95				210000		Hughes Street Bridge	11-Mar-99	0.97	0.83		63	
Hughes Street Bridge	4-Jan-96				160000		Polk and 66th	11-Mar-99	1.01	0.85		47	
Hughes Street Bridge	7-Feb-96		2.06		160000		Hughes Street Bridge	16-Mar-99	7	0.99		49	
Hughes Street Bridge	5-Mar-96		0.73		6500	4.4	Polk and 66th	16-Mar-99	7.41	0.87		56	
Hughes Street Bridge	2-Apr-96		3.24		60000		Hughes Street Bridge	16-Mar-99				10	
Hughes Street Bridge	24-Apr-96				17000		Hughes Street Bridge	23-Mar-99	2.1	0.36		57	
Hughes Street Bridge	7-May-96		0.32		160000	18	Hughes Street Bridge	23-Mar-99		0.4		2300	3.9
Hughes Street Bridge	5-Jun-96		0.33		160000	8	Hughes Street Bridge	25-Mar-99	1.14	0.21		55	
Hughes Street Bridge	18-Jun-96		1.1		28000	6.1	Polk and 66th	25-Mar-99	5.2	0.59		55	
Hughes Street Bridge	3-Jul-96				160000		Hughes Street Bridge	30-Mar-99	7.6	0.19		18	
Hughes Street Bridge	5-Aug-96				90000		Hughes Street Bridge	1-Apr-99	4.82	0.43		37	
Hughes Street Bridge	11-Sep-96				160000		Polk and 66th	1-Apr-99	6.6	0.61		43	
Hughes Street Bridge	12-Sep-96				110000		Hughes Street Bridge	5-Apr-99	1.44	0.46		62	
Hughes Street Bridge	8-Oct-96				7000		Hughes Street Bridge	7-Apr-99	1.35	0.74		53	
Hughes Street Bridge	24-Oct-96		0.9		72000	10.2	Polk and 66th	7-Apr-99	0.72	0.89		55	
Hughes Street Bridge	20-Nov-96				200		Hughes Street Bridge	13-Apr-99		0.6		4700	2.3
Hughes Street Bridge	2-Jan-97				160000		Hughes Street Bridge	15-Apr-99	4.03	0.79		24	
Hughes Street Bridge	5-Feb-97		1.4		65000	20.9	Polk and 66th	15-Apr-99	6.26	0.85		26	
Hughes Street Bridge	5-Feb-97				160000		Hughes Street Bridge	23-Apr-99	3.14	0.89		53	
Hughes Street Bridge	14-May-97		4.2		190000	35.1	Polk and 66th	23-Apr-99	2.39	0.49		55	
Polk and 66th	8-Sep-97						Hughes Street Bridge	27-Apr-99	2.82	1.16		27	
Hughes Street Bridge	9-Sep-97		1		180000	40.2	Polk and 66th	27-Apr-99	4.56	1.23		28	
Hughes Street Bridge	5-Nov-97		3.8		310000	53.4	Hughes Street Bridge	11-May-99	7.72	0.41		7	
Hughes Street Bridge	6-Nov-97		0.2		330000	10	Hughes Street Bridge	18-May-99					
Hughes Street Bridge	7-Nov-97		0.2		60000	8	Polk and 66th	18-May-99	4.37	0.98		55	
Hughes Street Bridge	10-Nov-97		0.5		155000	16.1	Hughes Street Bridge	25-May-99	0.6	1.31		39	
Hughes Street Bridge	12-Nov-97		0.4		86000	19.2	Hughes Street Bridge	25-May-99		0.1		8700	
Hughes Street Bridge	14-Nov-97		1.2		320000	33.7	Hughes Street Bridge	1-Jun-99	0.11	0.28		65	2.5
Hughes Street Bridge	17-Nov-97		2.5		2000000	40.6	Hughes Street Bridge	8-Jun-99	0.88	0.28		66	
Hughes Street Bridge	17-Nov-97				2000000		Hughes Street Bridge	8-Jun-99		0.3		34000	6.8
Hughes Street Bridge	19-Nov-97		5.7		520000	114.9	Hughes Street Bridge	11-Jun-99	1.11	0.58		40	
Hughes Street Bridge	19-Nov-97				920000		Polk and 66th	11-Jun-99	2.91	0.78		39	
Hughes Street Bridge	21-Nov-97		0.6		500000	15.6	Hughes Street Bridge	15-Jun-99	2.91	0.88		54	
Hughes Street Bridge	21-Nov-97		0.6		500000	15.6	Polk and 66th	15-Jun-99	2.76	0.71		42	
Hughes Street Bridge	24-Nov-97				260000	1.6	Hughes Street Bridge	15-Jun-99		0.4		8900	6.1
Hughes Street Bridge	24-Nov-97		1.6		260000		Hughes Street Bridge	22-Jun-99	1.16	0.39		34	
Hughes Street Bridge	26-Nov-97		1.2		43000	10.1	Polk and 66th	22-Jun-99	2.11	0.49		35	
Hughes Street Bridge	26-Nov-97		1.2		43000	10.1	Hughes Street Bridge	22-Jun-99		0.2		16000	2.8
Hughes Street Bridge	1-Dec-97		0.1		57000	5.1	Hughes Street Bridge	29-Jun-99	4.89	0.97		53	
Hughes Street Bridge	1-Dec-97				57000		Polk and 66th	29-Jun-99	3.5	1.23		51	
Hughes Street Bridge	10-Dec-97		1.3		640000	26.5	Hughes Street Bridge	29-Jun-99		0.8		20	2.6
Hughes Street Bridge	10-Dec-97				470000		Hughes Street Bridge	6-Jul-99	2.1	0.29		29	
Hughes Street Bridge	15-Dec-97		1.3		270000	9.4	Polk and 66th	6-Jul-99	1.95	0.97		32	
Hughes Street Bridge	15-Dec-97				270000		Hughes Street Bridge	13-Jul-99		0.3		540	4.6
Hughes Street Bridge	19-Dec-97		0.7		230000	62.7	Hughes Street Bridge	15-Jul-99	3.19	0.52		20	
Hughes Street Bridge	30-Dec-97		1.3		780	3.4	Polk and 66th	15-Jul-99	4.76	0.31		21	
Hughes Street Bridge	30-Dec-97		1.3		780	3.4	Hughes Street Bridge	20-Jul-99		0.3		10	
Hughes Street Bridge	15-Jan-98		6		8700	0	Polk and 66th	20-Jul-99	4.1	0.46		16	
Hughes Street Bridge	15-Jan-98		0		8700	6	Hughes Street Bridge	20-Jul-99		0.1		4000	2.4
Hughes Street Bridge	21-Jan-98		10.7		1100000	0.1	Hughes Street Bridge	27-Jul-99	4.3	0.42		82	
Hughes Street Bridge	21-Jan-98		0.1		1100000	10.7	Polk and 66th	27-Jul-99	6.3	0.78		47	
Hughes Street Bridge	29-Jan-98		57		720000	0.93	Hughes Street Bridge	27-Jul-99		0.1		38000	12.7
Hughes Street Bridge	29-Jan-98				15000	57.4	Hughes Street Bridge	3-Aug-99		0.68		53	
Hughes Street Bridge	12-Feb-98		3.3		38000	0.2	Polk and 66th	3-Aug-99		0.88		66	
Hughes Street Bridge	12-Feb-98		0.2		38000	3.3	Hughes Street Bridge	3-Aug-99		0.6		87000	4.3
Hughes Street Bridge	24-Feb-98		0.18		1400	8.2	Hughes Street Bridge	10-Aug-99		0.8		5900	6.8
Hughes Street Bridge	24-Feb-98		0.2		1400	8.2	Hughes Street Bridge	13-Aug-99		0.81		68	
Hughes Street Bridge	3-Mar-98		0.1		120	5.5	Polk and 66th	13-Aug-99		0.62		66	
Hughes Street Bridge	3-Mar-98		0.1		120	5.5	Hughes Street Bridge	17-Aug-99		0.6		8700	3.3
Hughes Street Bridge	25-Mar-98		1.6		200000	16.8	Hughes Street Bridge	31-Aug-99		1.1		130000	39
Hughes Street Bridge	25-Mar-98		1.6		200000	16.8	Hughes Street Bridge	13-Sep-99		0.3		20000	4.1
Hughes Street Bridge	1-Apr-98		1		1400000	27.1	Hughes Street Bridge	20-Sep-99		2.2		7600	3.2
Hughes Street Bridge	1-Apr-98		1		1400000	27.1	Hughes Street Bridge	27-Sep-99		0.9		380	2.9
Hughes Street Bridge	2-Apr-98			75			Hughes Street Bridge	4-Oct-99	0.84	0.42		46	
Polk and 66th	2-Apr-98						Polk and 66th	4-Oct-99	3.24	0.77		47	
Hughes Street Bridge	15-Apr-98		0.2		360000	30.3	Hughes Street Bridge	21-Oct-99	2.06	1.32		54	
Hughes Street Bridge	15-Apr-98		0.2		360000	30.3	Polk and 66th	21-Oct-99	3.28	1.13		49	
Hughes Street Bridge	21-Apr-98		0.8		32000	3.8	Hughes Street Bridge	1-Nov-99	4.54	0.63		40	
Hughes Street Bridge	21-Apr-98		0.5		32000	3.8	Polk and 66th	1-Nov-99	3.78	0.99		44	
Hughes Street Bridge	27-Apr-98		0.8		31000	6.3	Hughes Street Bridge	2-Nov-99				2400	
Hughes Street Bridge	27-Apr-98		0.8		31000	6.3	Hughes Street Bridge	16-Nov-99					



Hughes Street Bridge	28-Apr-98	0.6	1.3	59			Hughes Street Bridge	29-Nov-99						9
Polk and 66th	29-Apr-98													
Hughes Street Bridge	5-May-98		1.1		6200	4.6								
Hughes Street Bridge	6-May-98		1.1		6200	4.6								
Hughes Street Bridge	12-May-98				45000									
Hughes Street Bridge	12-May-98	1.41	0.86											
Polk and 66th	12-May-98													
Hughes Street Bridge	12-May-98		0.6		48000	7.7								
Hughes Street Bridge	20-May-98		0.4		100000	9.3								
Hughes Street Bridge	21-May-98	1.2	1.23	61										
Polk and 66th	21-May-98													
Hughes Street Bridge	27-May-98	1.1	0.36	63										
Polk and 66th	27-May-98													
Hughes Street Bridge	27-May-98		0		48000	26.5								
Hughes Street Bridge	28-May-98	1.7	0.54	66										
Polk and 66th	29-May-98													
Hughes Street Bridge	2-Jun-98		0.5		60000	3.3								
Hughes Street Bridge	3-Jun-98	1.2	0.31	70										
Polk and 66th	3-Jun-98													
Hughes Street Bridge	8-Jun-98	1	3.96	36										
Polk and 66th	8-Jun-98													
Hughes Street Bridge	9-Jun-98		1.8		370000									
Hughes Street Bridge	11-Jun-98	1.7	2.5	44										
Polk and 66th	11-Jun-98													
Hughes Street Bridge	15-Jun-98	2.9	0.87	62										
Polk and 66th	15-Jun-98													
Hughes Street Bridge	17-Jun-98	2.74	0.99	96										
Polk and 66th	17-Jun-98	2.17	1.06	50										
Hughes Street Bridge	17-Jun-98		0.4		200000	22.1								
Hughes Street Bridge	22-Jun-98	2.71	0.82	60										
Polk and 66th	23-Jun-98	2.4	0.76	62										
Hughes Street Bridge	26-Jun-98	3.1	1.29	64										
Polk and 66th	26-Jun-98	2.6	1.15	69										
Hughes Street Bridge	26-Jun-98		0.6		720000	6.8								
Hughes Street Bridge	29-Jun-98	7.9	0.28	16										
Polk and 66th	29-Jun-98	7.2	0.37	26										
Hughes Street Bridge	30-Jun-98		0.8		14	4.3								
Hughes Street Bridge	8-Jul-98	0.28	0.21	47										
Polk and 66th	8-Jul-98	0.260	0.11	48										
Hughes Street Bridge	13-Jul-98	2.9	0.85	57										
Polk and 66th	13-Jul-98	2.3	1.46	42										
Hughes Street Bridge	13-Jul-98				27000									
Hughes Street Bridge	20-Jul-98	8.1	0.32	24										
Polk and 66th	20-Jul-98	6.1	0.33	29										
Hughes Street Bridge	23-Jul-98		0.1		710000	13.2								
Hughes Street Bridge	28-Jul-98		0.2		650000	21.6								
Hughes Street Bridge	29-Jul-98	1.37	0.85	55										
Polk and 66th	29-Jul-98	2.84	1.09	60										
Hughes Street Bridge	4-Aug-98		0.6		300000	7.6								
Hughes Street Bridge	12-Aug-98	1.88	0.29	32										
Polk and 66th	12-Aug-98	2.76	0.53	40										
Hughes Street Bridge	17-Aug-98		1			0.8								
Hughes Street Bridge	19-Aug-98	2.72		26										
Polk and 66th	19-Aug-98	4.08	0.48	26										
Hughes Street Bridge	25-Aug-98		0.6		210000	3								
Hughes Street Bridge	26-Aug-98	1.8	0.27	58										
Polk and 66th	28-Aug-98	2.04	1.46	14										
Polk and 66th	31-Aug-98	2.42	1.82	55										
Hughes Street Bridge	1-Sep-98		0.1		230000	4								
Hughes Street Bridge	2-Sep-98	3.1	0.16	20										
Hughes Street Bridge	3-Sep-98				18000									
Hughes Street Bridge	14-Sep-98	2.76	0.24	35										
Polk and 66th	14-Sep-98	2.75	0.87	47										
Hughes Street Bridge	16-Sep-98	7.68	0.27	3										
Polk and 66th	18-Sep-98	1.8	1.85	57										
Hughes Street Bridge	22-Sep-98	1.78	0.35		370000	8.1								
Hughes Street Bridge	23-Sep-98		0.5		370000	8.1								
Polk and 66th	23-Sep-98	1.95	1.74											
Hughes Street Bridge	30-Sep-98		0.32	52	83000	4.4								
Hughes Street Bridge	30-Sep-98		0.3		83000	4.4								
Hughes Street Bridge	2-Oct-98	1.56	0.39	57										
Polk and 66th	2-Oct-98	3.4	1.27	62										
Hughes Street Bridge	7-Oct-98	2.95	0.33	13										
Hughes Street Bridge	9-Oct-98	3.32	0.72	75										
Polk and 66th	9-Oct-98	5.9	0.09	73										
Hughes Street Bridge	12-Oct-98	3.56	1.38	67										
Polk and 66th	12-Oct-98	6.3	2.46	48										
Hughes Street Bridge	14-Oct-98	3.58	0.79	63	2500	1.5								
Hughes Street Bridge	14-Oct-98		0.7		2500	1.5								
Hughes Street Bridge	21-Oct-98	6.05	0.61	25										
Hughes Street Bridge	28-Oct-98	1.14	1.06	70										
Hughes Street Bridge	29-Oct-98	3.11	1.26	76										
Polk and 66th	29-Oct-98	3.82	1.33	78										
Hughes Street Bridge	29-Oct-98				25000									
Hughes Street Bridge	2-Nov-98		0.33	3										
Polk and 66th	2-Nov-98		0.56	7										
Hughes Street Bridge	4-Nov-98	1.118	0.32											
Hughes Street Bridge	4-Nov-98		0.3		4100	2.1								
Hughes Street Bridge	12-Nov-98	4.62	0.64	45										
Polk and 66th	12-Nov-98	9.2	0.39	44										
Hughes Street Bridge	13-Nov-98	7.92	0	3										
Hughes Street Bridge	16-Nov-98	9.24	0.69	30										
Polk and 66th	16-Nov-98	7.67	0.54	30										
Hughes Street Bridge	18-Nov-98	3.01	0	40										
Hughes Street Bridge	18-Nov-98		0.3		13000	4.7								
Hughes Street Bridge	25-Nov-98	2.06	0.51	75										
Hughes Street Bridge	25-Nov-98	2.85	0.27	53										
Polk and 66th	25-Nov-98	1.87	0.65	66										
Hughes Street Bridge	2-Dec-98	0.82	0.66	12										
Hughes Street Bridge	2-Dec-98		0.2		8800	5.4								
Hughes Street Bridge	9-Dec-98		0.2		560000	4.1								
Hughes Street Bridge	22-Dec-98		0.6		79000	4.6								
Hughes Street Bridge	23-Dec-98	2.58	0	74										
Polk and 66th	23-Dec-98	5.18	0	57										
Hughes Street Bridge	30-Dec-98	6.4	0.26	40										
Polk and 66th	30-Dec-98	6.7	0.38	63										
Hughes Street Bridge	30-Dec-98		0.8			18.7								
		3.35	1.34	47.02	293,456	12.88			3.21	0.81	46.76	83,918	12.38	

Downstream Odor							Downstream No Odor						
Location	Field Visit Date	DC (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100ml)	BOD (mg/L)	Location	Field Visit Date	DC (ppm)	Ammonia_M8038 (mg/L)	Sulfate_M8051 (mg/L)	Fecal Coliform (cfu/100ml)	BOD (mg/L)
Hughes Street Bridge	5-Nov-97		3.8		310000	53.4	Hughes Street Bridge	1-Nov-99	4.54	0.63	40		
Hughes Street Bridge	6-Nov-97		0.2		330000	10	Polk and 66th	1-Nov-99	3.78	0.99	44		
Hughes Street Bridge	7-Nov-97		0.2		60000	8	Hughes Street Bridge	24-Nov-97				260000	1.6
Hughes Street Bridge	10-Nov-97		0.5		155000	16.1	Hughes Street Bridge	24-Nov-97		1.6		260000	
Hughes Street Bridge	12-Nov-97		0.4		86000	19.2	Hughes Street Bridge	28-Nov-97		1.2		43000	10.1
Hughes Street Bridge	14-Nov-97		1.2		320000	33.7	Hughes Street Bridge	28-Nov-97		1.2		43000	10.1
Hughes Street Bridge	17-Nov-97		2.5		2000000	40.6	Hughes Street Bridge	1-Dec-97				57000	
Hughes Street Bridge	19-Nov-97		5.7		520000	114.9	Hughes Street Bridge	1-Dec-97		0.1		57000	5.1
Hughes Street Bridge	21-Nov-97		0.6		500000	15.6	Hughes Street Bridge	10-Dec-97				470000	
Hughes Street Bridge	15-Dec-97		1.3		270000	9.4	Hughes Street Bridge	10-Dec-97		1.3		640000	26.6
Hughes Street Bridge	19-Dec-97		0.7		230000	62.7	Hughes Street Bridge	30-Dec-97		1.3		780	3.4
Hughes Street Bridge	21-Jan-98		10.7		1100000	0.1	Hughes Street Bridge	30-Dec-97		1.3		780	3.4
Hughes Street Bridge	25-Mar-98		1.6		200000	16.8	Hughes Street Bridge	15-Jan-98		6		8700	0
Hughes Street Bridge	1-Apr-98		1		1400000	27.1	Hughes Street Bridge	15-Jan-98		0		8700	6
Hughes Street Bridge	2-Apr-98			75			Hughes Street Bridge	29-Jan-98				15000	57.4
Hughes Street Bridge	15-Apr-98		0.2		360000	30.3	Hughes Street Bridge	29-Jan-98		57		720000	0.03
Hughes Street Bridge	27-Apr-98		0.8		31000	6.3	Hughes Street Bridge	12-Feb-98		0.2		38000	3.3
Hughes Street Bridge	28-Apr-98	0.6	1.3	59			Hughes Street Bridge	12-Feb-98		3.3		38000	0.2
Hughes Street Bridge	6-May-98		1.1		6200	4.6	Hughes Street Bridge	24-Feb-98		0.2		1400	8.2
Hughes Street Bridge	12-May-98				45000		Hughes Street Bridge	24-Feb-98		0.18		1400	8.2
Hughes Street Bridge	12-May-98				45000		Hughes Street Bridge	3-Mar-98		0.1		120	5.5
Hughes Street Bridge	12-May-98	1.41	0.66				Hughes Street Bridge	3-Mar-98		0.1		120	5.5
Hughes Street Bridge	12-May-98	1.41	0.66				Hughes Street Bridge	21-Apr-98		0.5		32000	3.8
Hughes Street Bridge	21-May-98	1.2	1.23	61			Hughes Street Bridge	21-Apr-98		0.5		32000	3.8
Hughes Street Bridge	27-May-98	1.1	0.36	63			Polk and 66th	28-May-98					
Hughes Street Bridge	11-Jun-98	1.7	2.5	44			Hughes Street Bridge	28-May-98	1.7	0.54	65		
Hughes Street Bridge	17-Jun-98	2.74	0.99	56			Hughes Street Bridge	3-Jun-98	1.2	0.31	70		
Hughes Street Bridge	22-Jun-98	2.71	0.82	60			Polk and 66th	3-Jun-98					
Hughes Street Bridge	13-Jul-98	2.8	0.65	57			Polk and 66th	8-Jun-98					
Hughes Street Bridge	26-Aug-98	1.8	0.27	58			Hughes Street Bridge	8-Jun-98	1	3.96	36		
Hughes Street Bridge	2-Oct-98	1.56	0.39	57			Hughes Street Bridge	15-Jun-98	2.9	0.57	62		
Hughes Street Bridge	29-Oct-98	3.11	1.26	78			Polk and 66th	15-Jun-98					
Hughes Street Bridge	25-Nov-98	2.06	0.51	75			Hughes Street Bridge	28-Jun-98	3.1	1.29	64		
Hughes Street Bridge	25-Nov-98	2.06	0.51	75			Hughes Street Bridge	28-Jun-98		0.6		720000	6.8
Hughes Street Bridge	25-Nov-98	2.85	0.27	53			Polk and 66th	28-Jun-98	2.6	1.15	69		
Hughes Street Bridge	25-Nov-98	2.85	0.27	53			Polk and 66th	29-Jun-98	7.2	0.37	25		
Hughes Street Bridge	2-Dec-98	0.52	0.66	12			Hughes Street Bridge	29-Jun-98	7.9	0.28	16		
Hughes Street Bridge	4-Oct-99	0.84	0.42	46			Hughes Street Bridge	8-Jul-98	0.28	0.21	47		
Hughes Street Bridge	23-Dec-98	2.55	0	74			Polk and 66th	8-Jul-98	0.285	0.11	49		
Hughes Street Bridge	30-Dec-98	5.4	0.26	40			Hughes Street Bridge	20-Jul-98	8.1	0.32	24		
Hughes Street Bridge	6-Jan-99	3.66	1.26	67			Polk and 66th	20-Jul-98	6.1	0.33	29		
Hughes Street Bridge	21-Jan-99	3.93	0.36	66			Polk and 66th	28-Jul-98	2.84	1.09	50		
Hughes Street Bridge	22-Jan-99	2.04	1.24	68			Hughes Street Bridge	29-Jul-98	1.37	0.85	55		
Hughes Street Bridge	26-Jan-99	1.15	2.13	53			Hughes Street Bridge	12-Aug-98	1.68	0.29	32		
Hughes Street Bridge	2-Feb-99	2.54	1.19	58			Polk and 66th	12-Aug-98	2.76	0.53	40		
Hughes Street Bridge	4-Feb-99	2.22	1.64	58			Hughes Street Bridge	19-Aug-98	2.72		25		
Hughes Street Bridge	5-Feb-99	0.26	1.49	53			Polk and 66th	19-Aug-98	4.08	0.45	26		
Hughes Street Bridge	9-Feb-99	3.29	0.98	48			Hughes Street Bridge	2-Sep-98	3.1	0.16	20		
Hughes Street Bridge	12-Feb-99		2.12	47			Hughes Street Bridge	14-Sep-98	2.76	0.24	35		
Hughes Street Bridge	16-Feb-99	5.45	1.55	58			Polk and 66th	14-Sep-98	2.75	0.87	47		
Hughes Street Bridge	18-Feb-99	4.58	0.47	9			Hughes Street Bridge	16-Sep-98	7.65	0.27	3		
Hughes Street Bridge	26-Feb-99	1.43	0.35	51			Hughes Street Bridge	22-Sep-98		0.5		370000	8.1
Hughes Street Bridge	2-Mar-99	1.3	0.74	40			Hughes Street Bridge	22-Sep-98	1.78	0.35		370000	8.1
Hughes Street Bridge	5-Mar-99	1.28	1.41	41			Hughes Street Bridge	30-Sep-98		0.3		83000	4.4
Hughes Street Bridge	9-Mar-99	1.3	0.73	38			Hughes Street Bridge	30-Sep-98		0.32	52	83000	4.4
Hughes Street Bridge	11-Mar-99	0.97	0.83	53			Hughes Street Bridge	7-Oct-98	2.95	0.33	13		
Hughes Street Bridge	23-Mar-99	2.1	0.98	57			Hughes Street Bridge	9-Oct-98	3.32	0.72	75		
Hughes Street Bridge	25-Mar-99	1.14	0.21	55			Polk and 66th	9-Oct-98	5.9	0.09	73		
Hughes Street Bridge	30-Mar-99	7.6	0.19	18			Hughes Street Bridge	12-Oct-98	3.56	1.38	67		
Hughes Street Bridge	5-Apr-99	1.44	0.46	62			Polk and 66th	12-Oct-98	6.3	2.48	48		
Hughes Street Bridge	7-Apr-99	1.35	0.74	53			Hughes Street Bridge	14-Oct-98	3.55	0.79	63	2500	1.5
Hughes Street Bridge	23-Apr-99	3.14	0.89	53			Hughes Street Bridge	14-Oct-98		0.7		2500	1.5
Hughes Street Bridge	27-Apr-99	2.82	1.16	27			Hughes Street Bridge	21-Oct-98	6.05	0.61	25		
Hughes Street Bridge	25-May-99	0.6	1.31	39			Hughes Street Bridge	28-Oct-98	1.14	1.06	70		
Hughes Street Bridge	8-Jun-99	0.88	0.28	65			Hughes Street Bridge	2-Nov-98		0.33	3		
Hughes Street Bridge	11-Jun-99	1.11	0.58	40			Polk and 66th	2-Nov-98		0.56	7		
Hughes Street Bridge	29-Jun-99	4.59	0.97	53			Hughes Street Bridge	4-Nov-98	1.118	0.32			
Hughes Street Bridge	27-Jul-99	4.3	0.42	52			Hughes Street Bridge	4-Nov-98		0.3		4100	2.1
Polk and 66th	2-Apr-98						Hughes Street Bridge	12-Nov-98	4.62	0.64	45		
Polk and 66th	28-Apr-98						Polk and 66th	12-Nov-98	9.2	0.39	44		
Polk and 66th	12-May-98						Hughes Street Bridge	13-Nov-98	7.92	0	3		
Polk and 66th	12-May-98						Hughes Street Bridge	16-Nov-98	9.24	0.69	30		
Polk and 66th	21-May-98						Polk and 66th	16-Nov-98	7.67	0.54	30		
Polk and 66th	27-May-98						Hughes Street Bridge	18-Nov-98	3.01	0	40		
Polk and 66th	11-Jun-98						Hughes Street Bridge	18-Nov-98		0.3		13000	4.7
Polk and 66th	17-Jun-98	2.17	1.06	50			Hughes Street Bridge	13-Jan-99	3.42	0.79	71		

Polk and 66th	13-Jul-98	2.3	1.46	42		Polk and 66th	13-Jan-99	3.55	0.96	75		
Polk and 66th	2-Oct-98	3.4	1.27	62		Hughes Street Bridge	13-Jan-99		0.9		120000	10.9
Polk and 66th	29-Oct-98	3.52	1.33	78		Hughes Street Bridge	23-Feb-99	1.63	0.15	36		
Polk and 66th	25-Nov-98	1.87	0.65	66		Hughes Street Bridge	23-Feb-99		0.4		1400	3.4
Polk and 66th	25-Nov-98	1.87	0.65	66		Polk and 66th	16-Mar-99	7.41	0.87	55		
Polk and 66th	23-Dec-98	5.18	0	57		Hughes Street Bridge	16-Mar-99				10	
Polk and 66th	30-Dec-98	6.7	0.36	63		Hughes Street Bridge	16-Mar-99	7	0.99	49		
Polk and 66th	6-Jan-99	5.57	1.39	70		Polk and 66th	1-Apr-99	6.6	0.61	43		
Polk and 66th	22-Jan-99	3.55	0.88	69		Hughes Street Bridge	1-Apr-99	4.62	0.43	37		
Polk and 66th	25-Jan-99	3.33	1.45	61		Polk and 66th	15-Apr-99	6.26	0.66	26		
Polk and 66th	4-Feb-99	6.34	0.84	69		Hughes Street Bridge	15-Apr-99	4.03	0.79	24		
Polk and 66th	5-Feb-99	2.06	1.52	60		Hughes Street Bridge	11-May-99	7.72	0.41	7		
Polk and 66th	12-Feb-99		2.64	62		Polk and 66th	15-Jun-99	2.76	0.71	42		
Polk and 66th	18-Feb-99	4.88	0.47	23		Hughes Street Bridge	15-Jun-99		0.4		5900	6.1
Polk and 66th	26-Feb-99	2.38	0.83	52		Hughes Street Bridge	15-Jun-99	2.91	0.88	54		
Polk and 66th	5-Mar-99	1.11	1.11	35		Polk and 66th	22-Jun-99	2.11	0.49	35		
Polk and 66th	11-Mar-99	1.01	0.85	47		Hughes Street Bridge	22-Jun-99		0.2		16000	2.8
Polk and 66th	25-Mar-99	5.2	0.59	55		Hughes Street Bridge	22-Jun-99	1.16	0.39	34		
Polk and 66th	7-Apr-99	0.72	0.88	55		Polk and 66th	6-Jul-99	1.85	0.97	32		
Polk and 66th	23-Apr-99	2.39	0.49	55		Hughes Street Bridge	6-Jul-99	2.1	0.29	29		
Polk and 66th	27-Apr-99	4.56	1.23	28		Polk and 66th	15-Jul-99	4.76	0.31	21		
Polk and 66th	11-Jun-99	2.91	0.78	39		Hughes Street Bridge	15-Jul-99	3.19	0.52	20		
Polk and 66th	29-Jun-99	3.5	1.23	51		Polk and 66th	20-Jul-99	4.1	0.46	16		
Polk and 66th	27-Jul-99	6.3	0.78	47		Hughes Street Bridge	20-Jul-99		0.1		4000	2.4
Polk and 66th	4-Oct-99	3.24	0.77	47		Hughes Street Bridge	20-Jul-99		0.3	10		
Hughes Street Bridge	25-May-99		0.1		8700	Polk and 66th	3-Aug-99		0.88	56		
Hughes Street Bridge	8-Jun-99		0.3		34000	Hughes Street Bridge	3-Aug-99		0.5		87000	4.3
Hughes Street Bridge	29-Jun-99		0.8		20	Hughes Street Bridge	3-Aug-99		0.68	53		
Hughes Street Bridge	27-Jul-99		0.1		36000	Polk and 66th	13-Aug-99		0.62	66		
Hughes Street Bridge	2-Dec-98		0.2		8800	Hughes Street Bridge	13-Aug-99		0.81	69		
Hughes Street Bridge	30-Dec-98		0.8			Polk and 66th	21-Oct-99	3.28	1.13	49		
Hughes Street Bridge	8-Jan-99		1		470000	Hughes Street Bridge	21-Oct-99	2.06	1.32	54		
Hughes Street Bridge	2-Feb-99		0.9		8100							
Hughes Street Bridge	9-Feb-99		1.4		68000							
Hughes Street Bridge	16-Feb-99		2		450							
Hughes Street Bridge	2-Mar-99		0.2		220000							
Hughes Street Bridge	9-Mar-99		0.6									
Hughes Street Bridge	23-Mar-99		0.4		2300							
Hughes Street Bridge	17-Nov-97				2000000							
Hughes Street Bridge	19-Nov-97				920000							
Hughes Street Bridge	21-Nov-97		0.6		500000							
Hughes Street Bridge	15-Dec-97				270000							
Hughes Street Bridge	21-Jan-98		0.1		1100000							
Hughes Street Bridge	25-Mar-98		1.0		200000							
Hughes Street Bridge	1-Apr-98		1		1400000							
Hughes Street Bridge	15-Apr-98		0.2		360000							
Hughes Street Bridge	27-Apr-98		0.8		31000							
Hughes Street Bridge	12-May-98		0.6		45000							
Hughes Street Bridge	12-May-98		0.6		45000							
Hughes Street Bridge	27-May-98		0		48000							
Hughes Street Bridge	17-Jun-98		0.4		200000							
Hughes Street Bridge	13-Jul-98				27000							
Hughes Street Bridge	29-Oct-98				25000							
	Odor	2.69	0.99	53.22	355,502	22.64						
	NoOdor	3.97	1.29	40.66	149,721	6.87						

## Appendix – V Map and Cross Sections for Open Portion Hydraulic Model

The channel distances are measured from the confluence of Braes Bayou and Country Club Bayou and this location is shown as the Eastern reference mark on the map. The section geometry 400 feet upstream was chosen as representative of the geometry through the wide portion of the country club, while the geometry 1300 feet upstream was chosen as representative of the geometry from that location to Wayside Drive. Sections 3000 and 3800 feet were selected to model the portion from Polk St. to Wayside Drive. Section 4900 represents the approach to the Polk Street culvert, and section 0600 represents the upper portion of the bayou. The sections are referenced to the centerline of the channel depicted on the map for the area calculations.

Figure V.1 is an excerpt from a USGS topographic map that displays the sections analyzed to develop the hydraulic model of the open portion.

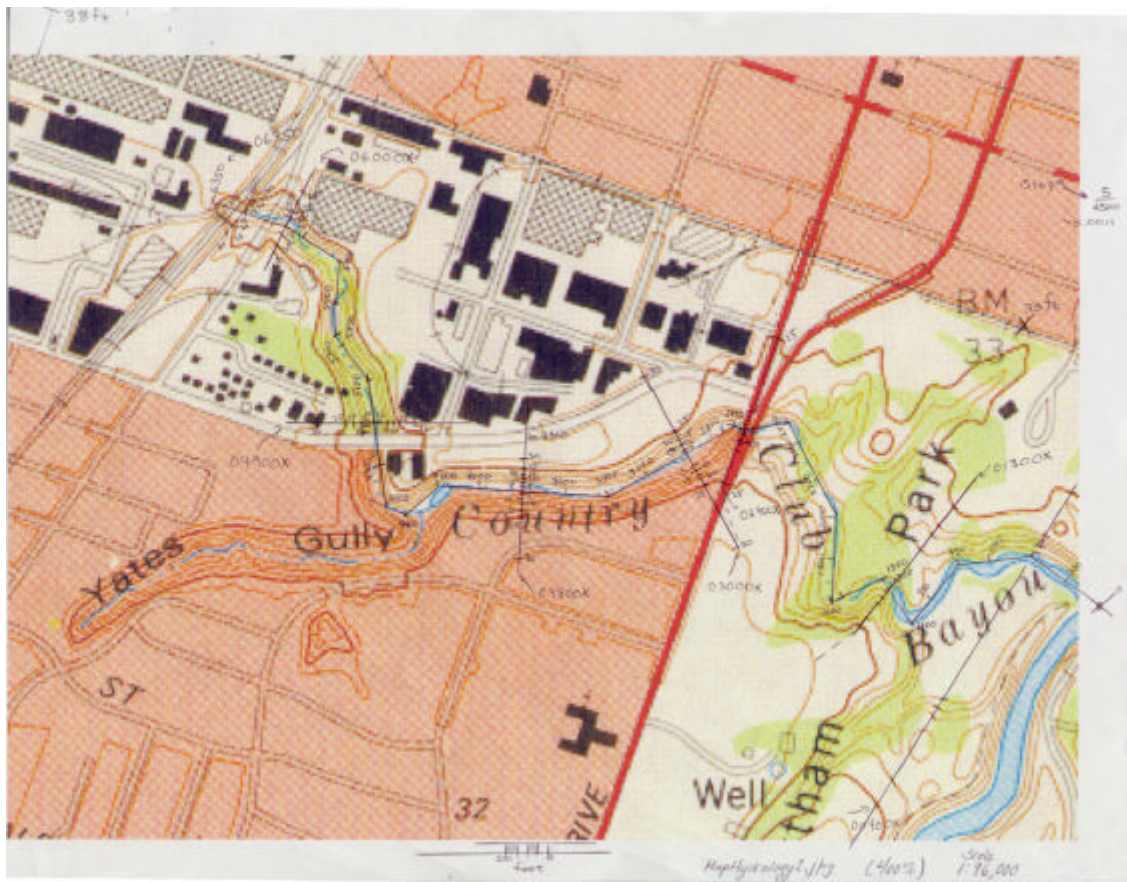
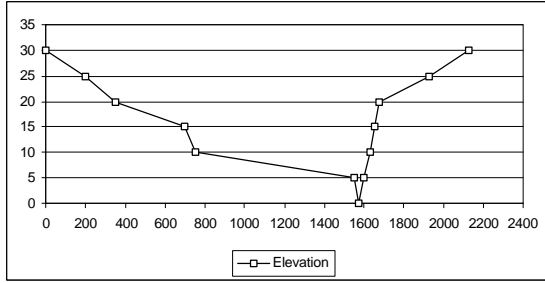
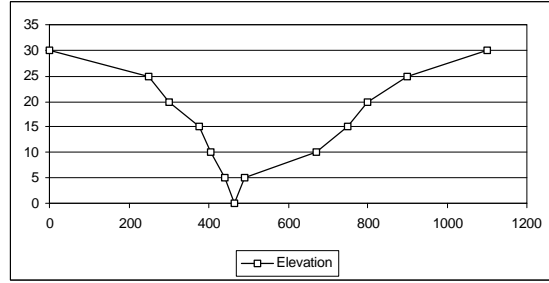


Figure V.1. Topographic map of Country Club showing cross section location

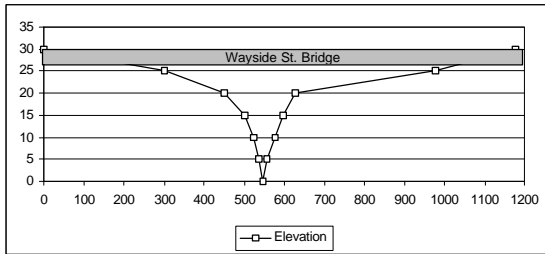
Figures V.2-V.9 show the geometry of the cross sections.



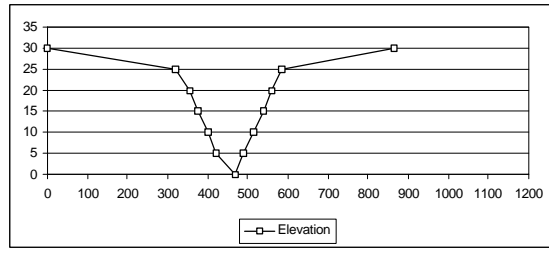
**Figure V.2 Section 00400**  
 (400 ft upstream of Country Club Bayou - Braes Bayou Confluence)



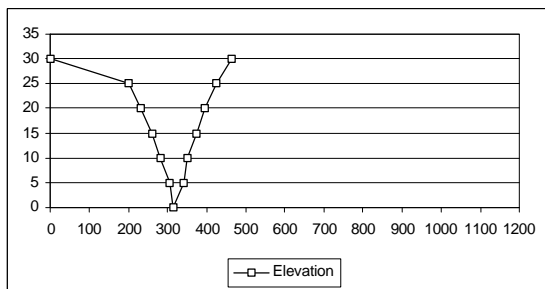
**Figure V.3 Section 01300**  
 (1300 ft upstream of Country Club Bayou - Braes Bayou Confluence)



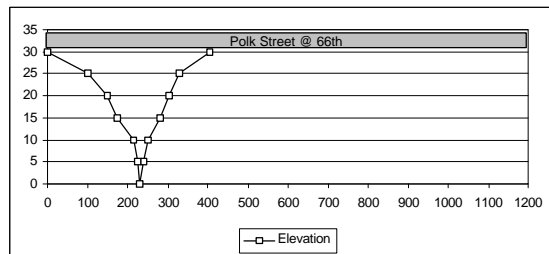
**Figure V.4 Section 02700**  
 (2700 ft upstream of Country Club Bayou - Braes Bayou Confluence-Upstream of Wayside)



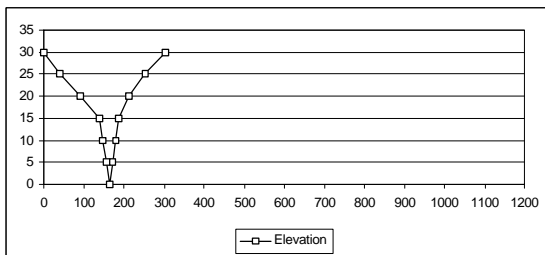
**Figure V.5 Section 03000**  
 (2700 ft upstream of Country Club Bayou - Braes Bayou Confluence)



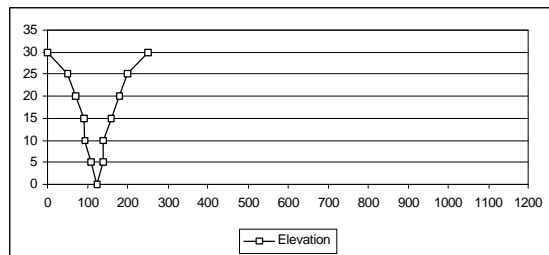
**Figure V.6 Section 03800**  
 (2700 ft upstream of Country Club Bayou - Braes Bayou Confluence)



**Figure V.7 Section 04900**  
 (2700 ft upstream of Country Club Bayou - Braes Bayou Confluence @ Polk Street)



**Figure V.8 Section 06000**  
 (6000 ft upstream of Country Club Bayou - Braes Bayou Confluence ~ East of Hughes Street)



**Figure V.9 Section 06400**  
 (6400 ft upstream of Country Club Bayou - Braes Bayou Confluence ~ Under RR Bridge)

Appendix – VII Representative QUAL2EU Input and Output Files

C855D.dat

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TITLE01          COUNTRY CLUB BAYOU -- 855GPM; 10-14-1999
TITLE02          NO ODOR HSB=DO(3.24,3.96);BOD(5.04,6.16); WAYSIDE=DO(4.32,5,28)
TITLE03  NO      CONSERVATIVE MINERAL  I      TDS IN MG/L
TITLE04  NO      CONSERVATIVE MINERAL  II
TITLE05  NO      CONSERVATIVE MINERAL  III
TITLE06  YES     TEMPERATURE
TITLE07  YES     5-DAY BIOCHEMICAL OXYGEN DEMAND
TITLE08  NO      ALGAE AS CHL-A IN UG/L
TITLE09  NO      PHOSPHORUS CYCLE AS P IN MG/L
TITLE10          (ORGANIC-P; DISSOLVED-P)
TITLE11  NO      NITROGEN CYCLE AS N IN MG/L
TITLE12          (ORGANIC-N; AMMONIA-N; NITRITE-N;' NITRATE-N)
TITLE13  YES     DISSOLVED OXYGEN IN MG/L
TITLE14  NO      FECAL COLIFORM IN NO./100 ML
TITLE15  NO      ARBITRARY NON-CONSERVATIVE
ENDTITLE

LIST DATA INPUT
NOWRITE OPTIONAL SUMMARY
NO FLOW AUGMENTATION
STEADY STATE
NO TRAP CHANNELS
PRINT LCD/SOLAR DATA
PLOT DO AND BOD
FIXED DNSTM CONC (YES=1)=      0.          5D-ULT BOD CONV K COEF =      0.25
INPUT METRIC                  =      1.          OUTPUT METRIC                  =      1.
NUMBER OF REACHES             =      5.          NUMBER OF JUNCTIONS           =      1.
NUM OF HEADWATERS             =      2.          NUMBER OF POINT LOADS        =      4.
TIME STEP (HOURS)             =                LNTH. COMP. ELEMENT (KM)=      0.121
MAXIMUM ROUTE TIME (HRS)=     30.          TIME INC. FOR RPT2 (HRS)=
LATITUDE OF BASIN (DEG) =     42.5         LONGITUDE OF BASIN (DEG)=     83.3
STANDARD MERIDIAN (DEG) =     75.          DAY OF YEAR START TIME =     196.
EVAP. COEF.,(AE)              =      0.0        EVAP. COEF.,(BE)              =     .0000056
ELEV. OF BASIN (METERS) =     150.         DUST ATTENUATION COEF.       =      0.13
ENDATA1
ENDATA1A
THETA      OXY TRAN      1.0159
ENDATA1B
STREAM REACH      1. RCH= ENNIS TO ALTIC      FROM      5.56      TO      4.35
STREAM REACH      1.1 RCH= ALTIC-EVERGRN     FROM      4.35      TO      3.14
STREAM REACH      2. RCH= S2-EVERGRN        FROM      3.51      TO      3.14
STREAM REACH      3. RCH= EVERGRN-HUGHES    FROM      3.14      TO      1.93
STREAM REACH      4. RCH= HUGHES-BRAES      FROM      1.81      TO      0.0
ENDATA2
ENDATA3
FLAG FIELD RCH=   1.          10.          1.2.2.2.2.2.2.2.2.2.
FLAG FIELD RCH=  1.1         10.          2.2.6.2.2.2.2.2.6.3.
FLAG FIELD RCH=   2.          3.           1.6.2.
FLAG FIELD RCH=   3.          10.          4.6.2.2.2.2.2.2.2.2.
FLAG FIELD RCH=   4.          16.          2.2.2.2.2.2.2.2.2.2.2.2.2.2.5.
ENDATA4
HYDRAULICS RCH=   1.          3.8          0.170        0.995        0.734        .0041        .020
HYDRAULICS RCH=  1.1         3.8          0.170        0.995        0.734        .0041        .020
HYDRAULICS RCH=   2.          3.8          0.170        0.995        0.734        .0041        .020
HYDRAULICS RCH=   3.          3.8          0.170        0.985        0.734        .0041        .020
HYDRAULICS RCH=   4.          7.6          0.14         1.004        1.184        .0005        .020
ENDATA5
REACT COEF RCH=   1.          0.050        0.000        0.900        1.          0.500        0.0000        0.0000
REACT COEF RCH=  1.1         0.050        0.000        0.900        1.          0.500        0.0000        0.0000
REACT COEF RCH=   2.          0.050        0.000        0.900        1.          0.500        0.0000        0.0000
REACT COEF RCH=   3.          0.050        0.000        0.900        1.          0.500        0.0000        0.0000
REACT COEF RCH=   4.          0.050        0.000        0.900        1.          1.000        0.0000        0.0000
ENDATA6
ENDATA6A
    
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ALG/OTHER COEF RCH= 1. 50.0 0.15 3.80 1.50
ALG/OTHER COEF RCH= 1.1 50.0 0.15 3.80 1.50
ALG/OTHER COEF RCH= 2. 50.0 0.15 0.38 1.50
ALG/OTHER COEF RCH= 3. 50.0 0.15 0.38 1.50
ALG/OTHER COEF RCH= 4. 50.0 0.15 3.80 1.50
ENDATA6B
INITIAL COND-1 RCH= 1. 20.00 0.00 0.00 0.00 0.00 0.00 0.000 0.0
INITIAL COND-1 RCH= 1.1 20.00 0.00 0.00 0.00 0.00 0.00 0.000 0.0
INITIAL COND-1 RCH= 2. 20.00 0.00 0.00 0.00 0.00 0.00 0.000 0.0
INITIAL COND-1 RCH= 3. 20.00 0.00 0.00 0.00 0.00 0.00 0.000 0.0
INITIAL COND-1 RCH= 4. 20.00 0.00 0.00 0.00 0.00 0.00 0.000 0.0
ENDATA7
ENDATA7A
INCR INFLOW-1 RCH= 1. 0.000 20.00 0.0 0.0 0.0 0.0 0.0 0.0 0.0
INCR INFLOW-1 RCH= 1.1 0.000 20.00 0.0 0.0 0.0 0.0 0.0 0.0 0.0
INCR INFLOW-1 RCH= 2. 0.000 20.00 0.0 0.0 0.0 0.0 0.0 0.0 0.0
INCR INFLOW-1 RCH= 3. 0.000 20.00 0.0 0.0 0.0 0.0 0.0 0.0 0.0
INCR INFLOW-1 RCH= 4. 0.000 20.00 0.0 0.0 0.0 0.0 0.0 0.0 0.0
ENDATA8
ENDATA8A
STREAM JUNCTION 1. JNC=S2-EVERGRN 20. 24. 23.
ENDATA9
HEADWTR-1 HDW= 1.ENNIS 0.025 20.0 3.0 09.5 0.0 0.0 0.0
HEADWTR-1 HDW= 2.S2 0.023 20.0 3.0 09.5 0.0 0.0 0.0
ENDATA10
ENDATA10A
POINTLD-1 PTL= 1.SWEETEX 0.00 0.006 20.0 0.1 99.0 0.0 0.0 0.0
POINTLD-1 PTL= 2.ALTIC 0.00 -0.000 20.0 5.5 0.0 0.0 0.0 0.0
POINTLD-1 PTL= 3.SOUTH 0.00 0.000 20.0 6.5 50.0 0.0 0.0 0.0
POINTLD-1 PTL= 4.VENTS 0.00 -0.000 20.0 5.5 0.0 0.0 0.0 0.0
ENDATA11
ENDATA11A
ENDATA12
ENDATA13
ENDATA13A
LOCAL CLIMATOLOGY .25 25. 20. 980. 2.5
BEGIN RCH 1
PLOT RCH 1 0 0 0 0
BEGIN RCH 3
PLOT RCH 0 0 3 0 0
BEGIN RCH 1
PLOT RCH 1 2 0 3 4

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C855D.out

\* \* \* QUAL-2E STREAM QUALITY ROUTING MODEL \* \* \*  
Version 3.22 -- May 1996

\$\$\$ (PROBLEM TITLES) \$\$\$

CARD TYPE	QUAL-2E PROGRAM TITLES
TITLE01	COUNTRY CLUB BAYOU -- 855GPM; 10-14-1999
TITLE02	NO ODOR HSB=DO(3.24,3.96);BOD(5.04,6.16); WAYSIDE=DO(4.32,5
TITLE03 NO	CONSERVATIVE MINERAL I TDS IN MG/L
TITLE04 NO	CONSERVATIVE MINERAL II
TITLE05 NO	CONSERVATIVE MINERAL III
TITLE06 YES	TEMPERATURE
TITLE07 YES	5-DAY BIOCHEMICAL OXYGEN DEMAND
TITLE08 NO	ALGAE AS CHL-A IN UG/L
TITLE09 NO	PHOSPHORUS CYCLE AS P IN MG/L
TITLE10	(ORGANIC-P; DISSOLVED-P)
TITLE11 NO	NITROGEN CYCLE AS N IN MG/L
TITLE12	(ORGANIC-N; AMMONIA-N; NITRITE-N;' NITRATE-N)
TITLE13 YES	DISSOLVED OXYGEN IN MG/L
TITLE14 NO	FECAL COLIFORM IN NO./100 ML
TITLE15 NO	ARBITRARY NON-CONSERVATIVE

ENDTITLE

\$\$\$ DATA TYPE 1 (CONTROL DATA) \$\$\$

CARD TYPE		CARD TYPE	
LIST DATA INPUT	0.00000		0.00000
NOWRITE OPTIONAL SUMMARY	0.00000		0.00000
NO FLOW AUGMENTATION	0.00000		0.00000
STEADY STATE	0.00000		0.00000
NO TRAP CHANNELS	0.00000		0.00000
PRINT LCD/SOLAR DATA	0.00000		0.00000
PLOT DO AND BOD	0.00000		0.00000
FIXED DNSTM CONC (YES=1)=	0.00000	5D-ULT BOD CONV K COEF =	0.25000
INPUT METRIC =	1.00000	OUTPUT METRIC =	1.00000
NUMBER OF REACHES =	5.00000	NUMBER OF JUNCTIONS =	1.00000
NUM OF HEADWATERS =	2.00000	NUMBER OF POINT LOADS =	4.00000
TIME STEP (HOURS) =	0.00000	LNTH. COMP. ELEMENT (KM)=	0.12100
MAXIMUM ROUTE TIME (HRS)=	30.00000	TIME INC. FOR RPT2 (HRS)=	0.00000
LATITUDE OF BASIN (DEG) =	42.50000	LONGITUDE OF BASIN (DEG)=	83.30000
STANDARD MERIDIAN (DEG) =	75.00000	DAY OF YEAR START TIME =	196.00000
EVAP. COEF., (AE) =	0.00000	EVAP. COEF., (BE) =	0.00001
ELEV. OF BASIN (METERS) =	150.00000	DUST ATTENUATION COEF. =	0.13000
ENDATA1	0.00000		0.00000



\$\$\$ DATA TYPE 1A (ALGAE PRODUCTION AND NITROGEN OXIDATION CONSTANTS) \$\$\$

CARD TYPE		CARD TYPE	
ENDATA1A	0.0000		0.0000

\$\$\$ DATA TYPE 1B (TEMPERATURE CORRECTION CONSTANTS FOR RATE COEFFICIENTS) \$\$\$

CARD TYPE	RATE CODE	THETA VALUE	
THETA( 1)	BOD DECA	1.047	DFLT
THETA( 2)	BOD SETT	1.024	DFLT
THETA( 3)	OXY TRAN	1.016	USER
THETA( 4)	SOD RATE	1.060	DFLT
THETA( 5)	ORGN DEC	1.047	DFLT
THETA( 6)	ORGN SET	1.024	DFLT
THETA( 7)	NH3 DECA	1.083	DFLT
THETA( 8)	NH3 SRCE	1.074	DFLT
THETA( 9)	NO2 DECA	1.047	DFLT
THETA(10)	PORG DEC	1.047	DFLT
THETA(11)	PORG SET	1.024	DFLT
THETA(12)	DISP SRC	1.074	DFLT
THETA(13)	ALG GROW	1.047	DFLT
THETA(14)	ALG RESP	1.047	DFLT
THETA(15)	ALG SETT	1.024	DFLT
THETA(16)	COLI DEC	1.047	DFLT
THETA(17)	ANC DECA	1.000	DFLT
THETA(18)	ANC SETT	1.024	DFLT
THETA(19)	ANC SRCE	1.000	DFLT
ENDATA1B			

\$\$\$ DATA TYPE 2 (REACH IDENTIFICATION) \$\$\$

CARD TYPE	REACH ORDER AND IDENT	R. MI/KM		R. MI/KM
STREAM REACH	1.0 RCH= ENNIS TO ALTIC	5.6	TO	4.3
STREAM REACH	1.1 RCH= ALTIC-EVERGRN	4.3	TO	3.1
STREAM REACH	2.0 RCH= S2-EVERGRN	3.5	TO	3.1
STREAM REACH	3.0 RCH= EVERGRN-HUGHES	3.1	TO	1.9
STREAM REACH	4.0 RCH= HUGHES-BRAES	1.8	TO	0.0
ENDATA2	0.0	0.0		0.0

\$\$\$ DATA TYPE 3 (TARGET LEVEL DO AND FLOW AUGMENTATION SOURCES) \$\$\$

CARD TYPE	REACH	AVAIL	HDWS	TARGET	ORDER OF AVAIL	SOURCES
ENDATA3	0.	0.	0.0	0.	0.	0. 0. 0. 0.

\$\$\$ DATA TYPE 4 (COMPUTATIONAL REACH FLAG FIELD) \$\$\$

CARD TYPE	REACH	ELEMENTS/REACH	COMPUTATIONAL FLAGS
FLAG FIELD	1.	10.	1.2.2.2.2.2.2.2.2.2.0.0.0.0.0.0.0.0.0.0.0.0.
FLAG FIELD	1.	10.	2.2.6.2.2.2.2.2.2.6.3.0.0.0.0.0.0.0.0.0.0.0.0.
FLAG FIELD	2.	3.	1.6.2.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.
FLAG FIELD	3.	10.	4.6.2.2.2.2.2.2.2.2.2.0.0.0.0.0.0.0.0.0.0.0.
FLAG FIELD	4.	16.	2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.2.5.0.0.0.0.
ENDATA4	0.	0.	0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.

\$\$\$ DATA TYPE 5 (HYDRAULIC DATA FOR DETERMINING VELOCITY AND DEPTH) \$\$\$

CARD TYPE	REACH	COEF-DSPN	COEFQV	EXPOQV	COEFQH	EXPOQH	CMANN
HYDRAULICS	1.	3.80	0.170	0.995	0.734	0.004	1.020
HYDRAULICS	1.	3.80	0.170	0.995	0.734	0.004	1.020
HYDRAULICS	2.	3.80	0.170	0.995	0.734	0.004	1.020
HYDRAULICS	3.	3.80	0.170	0.985	0.734	0.004	1.020
HYDRAULICS	4.	7.60	0.140	1.004	1.184	0.000	5.020
ENDATA5	0.	0.00	0.000	0.000	0.000	0.000	0.000

\$\$\$ DATA TYPE 5A (STEADY STATE TEMPERATURE AND CLIMATOLOGY DATA) \$\$\$

CARD TYPE	REACH	ELEVATION	DUST COEF	CLOUD COVER	DRY BULB TEMP	WET BULB TEMP	ATM PRESSURE	WIND	SOLAR RAD ATTENUATION
TEMP/LCD	1.	150.00	0.13	0.25	25.00	20.00	980.00	2.50	1.00
TEMP/LCD	1.	150.00	0.13	0.25	25.00	20.00	980.00	2.50	1.00
TEMP/LCD	2.	150.00	0.13	0.25	25.00	20.00	980.00	2.50	1.00
TEMP/LCD	3.	150.00	0.13	0.25	25.00	20.00	980.00	2.50	1.00
TEMP/LCD	4.	150.00	0.13	0.25	25.00	20.00	980.00	2.50	1.00
ENDATA5A	0.	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

\$\$\$ DATA TYPE 6 (REACTION COEFFICIENTS FOR DEOXYGENATION AND REAERATION) \$\$\$

CARD TYPE	REACH	K1	K3	SOD RATE	K2OPT	K2	COEQK2 TSIV COEF FOR OPT 8	OR OR	EXPQK2 SLOPE FOR OPT 8
REACT COEF	1.	0.05	0.00	0.900	1.	0.50	0.000		0.00000
REACT COEF	1.	0.05	0.00	0.900	1.	0.50	0.000		0.00000
REACT COEF	2.	0.05	0.00	0.900	1.	0.50	0.000		0.00000
REACT COEF	3.	0.05	0.00	0.900	1.	0.50	0.000		0.00000
REACT COEF	4.	0.05	0.00	0.900	1.	1.00	0.000		0.00000
ENDATA6	0.	0.00	0.00	0.000	0.	0.00	0.000		0.00000

\$\$\$ DATA TYPE 6A (NITROGEN AND PHOSPHORUS CONSTANTS) \$\$\$

CARD TYPE	REACH	CKNH2	SETNH2	CKNH3	SNH3	CKNO2	CKPORG	SETPORG	SPO4
ENDATA6A	0.	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

\$\$\$ DATA TYPE 6B (ALGAE/OTHER COEFFICIENTS) \$\$\$

CARD TYPE	REACH	ALPHA0	ALGSET	EXCOEF	CK5 CKCOLI	CKANC	SETANC	SRCANC
ALG/OTHER COEF	1.	50.00	0.15	3.80	1.50	0.00	0.00	0.00
ALG/OTHER COEF	1.	50.00	0.15	3.80	1.50	0.00	0.00	0.00
ALG/OTHER COEF	2.	50.00	0.15	0.38	1.50	0.00	0.00	0.00
ALG/OTHER COEF	3.	50.00	0.15	0.38	1.50	0.00	0.00	0.00
ALG/OTHER COEF	4.	50.00	0.15	3.80	1.50	0.00	0.00	0.00
ENDATA6B	0.	0.00	0.00	0.00	0.00	0.00	0.00	0.00

\$\$\$ DATA TYPE 7 (INITIAL CONDITIONS) \$\$\$

CARD TYPE	REACH	TEMP	D.O.	BOD	CM-1	CM-2	CM-3	ANC	COLI
INITIAL COND-1	1.	20.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
INITIAL COND-1	1.	20.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
INITIAL COND-1	2.	20.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
INITIAL COND-1	3.	20.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
INITIAL COND-1	4.	20.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ENDATA7	0.	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

\$\$\$ DATA TYPE 7A (INITIAL CONDITIONS FOR CHOROPHYLL A, NITROGEN, AND PHOSPHORUS) \$\$\$

CARD TYPE	REACH	CHL-A	ORG-N	NH3-N	NO2-N	NO3-N	ORG-P	DIS-P
ENDATA7A	0.	0.00	0.00	0.00	0.00	0.00	0.00	0.00

\$\$\$ DATA TYPE 8 (INCREMENTAL INFLOW CONDITIONS) \$\$\$

CARD TYPE	REACH	FLOW	TEMP	D.O.	BOD	CM-1	CM-2	CM-3	ANC	COLI
INCR INFLOW-1	1.	0.000	20.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
INCR INFLOW-1	1.	0.000	20.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
INCR INFLOW-1	2.	0.000	20.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
INCR INFLOW-1	3.	0.000	20.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
INCR INFLOW-1	4.	0.000	20.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ENDATA8	0.	0.000	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

\$\$\$ DATA TYPE 8A (INCREMENTAL INFLOW CONDITIONS FOR CHLOROPHYLL A, NITROGEN, AND PHOSPHORUS) \$\$\$

CARD TYPE	REACH	CHL-A	ORG-N	NH3-N	NO2-N	NO3-N	ORG-P	DIS-P
ENDATA8A	0.	0.00	0.00	0.00	0.00	0.00	0.00	0.00

\$\$\$ DATA TYPE 9 (STREAM JUNCTIONS) \$\$\$

CARD TYPE	JUNCTION ORDER AND IDENT	UPSTRM	JUNCTION	TRIB
STREAM JUNCTION	1. JNC=S2-EVERGRN	20.	24.	23.
ENDATA9	0.	0.	0.	0.

\$\$\$ DATA TYPE 10 (HEADWATER SOURCES) \$\$\$

CARD TYPE	HDWTR ORDER	NAME	FLOW	TEMP	D.O.	BOD	CM-1	CM-2	CM-3
HEADWTR-1	1.	ENNIS	0.03	20.00	3.00	9.00	50.00	0.00	0.00
HEADWTR-1	2.	S2	0.02	20.00	3.00	9.00	50.00	0.00	0.00
ENDATA10	0.		0.00	0.00	0.00	0.00	0.00	0.00	0.00

\$\$\$ DATA TYPE 10A (HEADWATER CONDITIONS FOR CHLOROPHYLL, NITROGEN, PHOSPHORUS, COLIFORM AND SELECTED NON-CONSERVATIVE CONSTITUENT) \$\$\$

CARD TYPE	HDWTR ORDER	ANC	COLI	CHL-A	ORG-N	NH3-N	NO2-N	NO3-N	ORG-P	DIS-P
ENDATA10A	0.	0.00	0.00E+00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

\$\$\$ DATA TYPE 11 (POINT SOURCE / POINT SOURCE CHARACTERISTICS) \$\$\$

CARD TYPE	POINT LOAD ORDER	NAME	EFF	FLOW	TEMP	D.O.	BOD	CM-1	CM-2	CM-3
POINTLD-1	1.	SWEETEX	0.00	0.01	20.00	0.10	99.00	0.00	0.00	0.00
POINTLD-1	2.	ALTIC	0.00	0.00	20.00	5.50	0.00	0.00	0.00	0.00
POINTLD-1	3.	SOUTH	0.00	0.00	20.00	6.50	50.00	0.00	0.00	0.00
POINTLD-1	4.	VENTS	0.00	0.00	20.00	5.50	0.00	0.00	0.00	0.00
ENDATA11	0.		0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

\$\$\$ DATA TYPE 11A (POINT SOURCE CHARACTERISTICS - CHLOROPHYLL A, NITROGEN, PHOSPHORUS, COLIFORMS AND SELECTED NON-CONSERVATIVE CONSTITUENT) \$\$\$

CARD TYPE	POINT LOAD ORDER	ANC	COLI	CHL-A	ORG-N	NH3-N	NO2-N	NO3-N	ORG-P	DIS-P
ENDATA11A	0.	0.00	0.00E+00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

\$\$\$ DATA TYPE 12 (DAM CHARACTERISTICS) \$\$\$

	DAM	RCH	ELE	ADAM	BDAM	FDAM	HDAM
ENDATA12	0.	0.	0.	0.00	0.00	0.00	0.00

\$\$\$ DATA TYPE 13 (DOWNSTREAM BOUNDARY CONDITIONS-1) \$\$\$

CARD TYPE	TEMP	D.O.	BOD	CM-1	CM-2	CM-3	ANC	COLI
ENDATA13	DOWNSTREAM BOUNDARY CONCENTRATIONS ARE UNCONSTRAINED							

\$\$\$ DATA TYPE 13A (DOWNSTREAM BOUNDARY CONDITIONS-2) \$\$\$

CARD TYPE                      CHL-A      ORG-N      NH3-N      NO2-N      NH3-N      ORG-P      DIS-P  
 ENDATA13A                      DOWNSTREAM BOUNDARY CONCENTRATIONS ARE UNCONSTRAINED

STEADY STATE TEMPERATURE SIMULATION; CONVERGENCE SUMMARY:  
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ITERATION	NUMBER OF NONCONVERGENT ELEMENTS
1	49
2	11
3	0

SUMMARY OF VALUES FOR STEADY STATE TEMPERATURE CALCULATIONS (SUBROUTINE HEATER):  
 -----

DAILY NET SOLAR RADIATION = 2630.436 BTU/FT-2 ( 713.822 LANGLEYS)  
 NUMBER OF DAYLIGHT HOURS = 14.8

HOURLY VALUES OF SOLAR RADIATION (BTU/FT-2)

1	0.00	9	170.88	17	186.71
2	0.00	10	221.98	18	130.99
3	0.00	11	262.87	19	72.70
4	0.00	12	290.07	20	16.40
5	0.00	13	301.19	21	0.00
6	4.32	14	295.09	22	0.00
7	55.66	15	272.41	23	0.00
8	113.93	16	235.23	24	0.00

STREAM QUALITY SIMULATION  
 QUAL-2E STREAM QUALITY ROUTING MODEL

OUTPUT PAGE NUMBER 1  
 Version 3.22 -- May 1996

\*\*\*\*\* STEADY STATE SIMULATION \*\*\*\*\*

\*\* HYDRAULICS SUMMARY \*\*

ELE ORD	RCH NUM	ELE NUM	BEGIN LOC KILO	END LOC KILO	FLOW CMS	POINT SRCE CMS	INCR FLOW CMS	VEL MPS	TRVL TIME DAY	DEPTH M	WIDTH M	VOLUME K-CU-M	BOTTOM AREA K-SQ-M	X-SECT AREA SQ-M	DSPRSN COEF SQ-M/S
1	1	1	5.56	5.44	0.03	0.00	0.00	0.004	0.324	0.723	7.985	0.70	1.14	5.77	0.04
2	1	2	5.44	5.32	0.03	0.00	0.00	0.004	0.324	0.723	7.985	0.70	1.14	5.77	0.04
3	1	3	5.32	5.20	0.03	0.00	0.00	0.004	0.324	0.723	7.985	0.70	1.14	5.77	0.04
4	1	4	5.20	5.08	0.03	0.00	0.00	0.004	0.324	0.723	7.985	0.70	1.14	5.77	0.04
5	1	5	5.08	4.95	0.03	0.00	0.00	0.004	0.324	0.723	7.985	0.70	1.14	5.77	0.04
6	1	6	4.95	4.83	0.03	0.00	0.00	0.004	0.324	0.723	7.985	0.70	1.14	5.77	0.04
7	1	7	4.83	4.71	0.03	0.00	0.00	0.004	0.324	0.723	7.985	0.70	1.14	5.77	0.04
8	1	8	4.71	4.59	0.03	0.00	0.00	0.004	0.324	0.723	7.985	0.70	1.14	5.77	0.04
9	1	9	4.59	4.47	0.03	0.00	0.00	0.004	0.324	0.723	7.985	0.70	1.14	5.77	0.04
10	1	10	4.47	4.35	0.03	0.00	0.00	0.004	0.324	0.723	7.985	0.70	1.14	5.77	0.04
11	2	1	4.35	4.23	0.03	0.00	0.00	0.004	0.324	0.723	7.985	0.70	1.14	5.77	0.04
12	2	2	4.23	4.11	0.03	0.00	0.00	0.004	0.324	0.723	7.985	0.70	1.14	5.77	0.04
13	2	3	4.11	3.99	0.03	0.01	0.00	0.005	0.261	0.724	7.986	0.70	1.14	5.78	0.05
14	2	4	3.99	3.87	0.03	0.00	0.00	0.005	0.261	0.724	7.986	0.70	1.14	5.78	0.05
15	2	5	3.87	3.74	0.03	0.00	0.00	0.005	0.261	0.724	7.986	0.70	1.14	5.78	0.05
16	2	6	3.74	3.62	0.03	0.00	0.00	0.005	0.261	0.724	7.986	0.70	1.14	5.78	0.05
17	2	7	3.62	3.50	0.03	0.00	0.00	0.005	0.261	0.724	7.986	0.70	1.14	5.78	0.05
18	2	8	3.50	3.38	0.03	0.00	0.00	0.005	0.261	0.724	7.986	0.70	1.14	5.78	0.05
19	2	9	3.38	3.26	0.03	0.00	0.00	0.005	0.261	0.724	7.986	0.70	1.14	5.78	0.05
20	2	10	3.26	3.14	0.03	0.00	0.00	0.005	0.261	0.724	7.986	0.70	1.14	5.78	0.05
21	3	1	3.51	3.39	0.02	0.00	0.00	0.004	0.352	0.723	7.984	0.70	1.14	5.77	0.04
22	3	2	3.39	3.27	0.02	0.00	0.00	0.004	0.352	0.723	7.984	0.70	1.14	5.77	0.04
23	3	3	3.27	3.15	0.02	0.00	0.00	0.004	0.352	0.723	7.984	0.70	1.14	5.77	0.04
24	4	1	3.14	3.02	0.05	0.00	0.00	0.010	0.146	0.725	7.761	0.68	1.11	5.63	0.09
25	4	2	3.02	2.90	0.05	0.00	0.00	0.010	0.146	0.725	7.761	0.68	1.11	5.63	0.09
26	4	3	2.90	2.78	0.05	0.00	0.00	0.010	0.146	0.725	7.761	0.68	1.11	5.63	0.09
27	4	4	2.78	2.66	0.05	0.00	0.00	0.010	0.146	0.725	7.761	0.68	1.11	5.63	0.09
28	4	5	2.66	2.54	0.05	0.00	0.00	0.010	0.146	0.725	7.761	0.68	1.11	5.63	0.09
29	4	6	2.54	2.41	0.05	0.00	0.00	0.010	0.146	0.725	7.761	0.68	1.11	5.63	0.09
30	4	7	2.41	2.29	0.05	0.00	0.00	0.010	0.146	0.725	7.761	0.68	1.11	5.63	0.09

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31	4	8	2.29	2.17	0.05	0.00	0.00	0.010	0.146	0.725	7.761	0.68	1.11	5.63	0.09
32	4	9	2.17	2.05	0.05	0.00	0.00	0.010	0.146	0.725	7.761	0.68	1.11	5.63	0.09
33	4	10	2.05	1.93	0.05	0.00	0.00	0.010	0.146	0.725	7.761	0.68	1.11	5.63	0.09
34	5	1	1.81	1.69	0.05	0.00	0.00	0.007	0.187	1.184	6.104	0.87	1.03	7.23	1.03
35	5	2	1.69	1.57	0.05	0.00	0.00	0.007	0.187	1.184	6.104	0.87	1.03	7.23	1.03
36	5	3	1.57	1.45	0.05	0.00	0.00	0.007	0.187	1.184	6.104	0.87	1.03	7.23	1.03
37	5	4	1.45	1.33	0.05	0.00	0.00	0.007	0.187	1.184	6.104	0.87	1.03	7.23	1.03
38	5	5	1.33	1.20	0.05	0.00	0.00	0.007	0.187	1.184	6.104	0.87	1.03	7.23	1.03

STREAM QUALITY SIMULATION  
QUAL-2E STREAM QUALITY ROUTING MODEL

OUTPUT PAGE NUMBER 2  
Version 3.22 -- May 1996

\*\*\*\*\* STEADY STATE SIMULATION \*\*\*\*\*

\*\* HYDRAULICS SUMMARY \*\*

ELE ORD	RCH NUM	ELE NUM	BEGIN LOC KILO	END LOC KILO	FLOW CMS	POINT SRCE CMS	INCR FLOW CMS	VEL MPS	TRVL TIME DAY	DEPTH M	WIDTH M	VOLUME K-CU-M	BOTTOM AREA K-SQ-M	X-SECT AREA SQ-M	DSPRSN COEF SQ-M/S
39	5	6	1.20	1.08	0.05	0.00	0.00	0.007	0.187	1.184	6.104	0.87	1.03	7.23	1.03
40	5	7	1.08	0.96	0.05	0.00	0.00	0.007	0.187	1.184	6.104	0.87	1.03	7.23	1.03
41	5	8	0.96	0.84	0.05	0.00	0.00	0.007	0.187	1.184	6.104	0.87	1.03	7.23	1.03
42	5	9	0.84	0.72	0.05	0.00	0.00	0.007	0.187	1.184	6.104	0.87	1.03	7.23	1.03
43	5	10	0.72	0.60	0.05	0.00	0.00	0.007	0.187	1.184	6.104	0.87	1.03	7.23	1.03
44	5	11	0.60	0.48	0.05	0.00	0.00	0.007	0.187	1.184	6.104	0.87	1.03	7.23	1.03
45	5	12	0.48	0.36	0.05	0.00	0.00	0.007	0.187	1.184	6.104	0.87	1.03	7.23	1.03
46	5	13	0.36	0.24	0.05	0.00	0.00	0.007	0.187	1.184	6.104	0.87	1.03	7.23	1.03
47	5	14	0.24	0.12	0.05	0.00	0.00	0.007	0.187	1.184	6.104	0.87	1.03	7.23	1.03
48	5	15	0.12	-0.01	0.05	0.00	0.00	0.007	0.187	1.184	6.104	0.87	1.03	7.23	1.03
49	5	16	-0.01	-0.13	0.05	0.00	0.00	0.007	0.187	1.184	6.104	0.87	1.03	7.23	1.03

STREAM QUALITY SIMULATION  
 QUAL-2E STREAM QUALITY ROUTING MODEL

OUTPUT PAGE NUMBER 3  
 Version 3.22 -- May 1996

\*\*\*\*\* STEADY STATE SIMULATION \*\*\*\*\*

\*\* REACTION COEFFICIENT SUMMARY \*\*

RCH NUM	ELE NUM	DO SAT	K2 OPT	OXYGN REAIR	BOD DECAY	BOD SETT	SOD RATE	ORGN DECAY	ORGN SETT	NH3 DECAY	NH3 SRCE	NO2 DECAY	ORGP DECAY	ORGP SETT	DISP SRCE	COLI DECAY	ANC DECAY	ANC SETT	ANC SRCE
		MG/L		1/DAY	1/DAY	1/DAY	G/M2D	1/DAY	1/DAY	1/DAY	MG/M2D	1/DAY	1/DAY	1/DAY	MG/M2D	1/DAY	1/DAY	1/DAY	MG/M2D
1	1	8.53	1	0.52	0.06	0.00	1.03	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1	2	8.24	1	0.53	0.06	0.00	1.14	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1	3	8.03	1	0.54	0.06	0.00	1.24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1	4	7.87	1	0.55	0.07	0.00	1.32	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1	5	7.74	1	0.56	0.07	0.00	1.39	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1	6	7.65	1	0.57	0.07	0.00	1.44	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1	7	7.58	1	0.57	0.07	0.00	1.49	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1	8	7.53	1	0.58	0.08	0.00	1.52	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1	9	7.49	1	0.58	0.08	0.00	1.55	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1	10	7.46	1	0.58	0.08	0.00	1.57	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	1	7.43	1	0.58	0.08	0.00	1.59	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	2	7.43	1	0.58	0.08	0.00	1.59	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	3	7.60	1	0.57	0.07	0.00	1.48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	4	7.55	1	0.57	0.08	0.00	1.51	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5	7.51	1	0.58	0.08	0.00	1.53	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	6	7.48	1	0.58	0.08	0.00	1.56	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	7	7.46	1	0.58	0.08	0.00	1.57	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	8	7.44	1	0.58	0.08	0.00	1.59	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	9	7.42	1	0.58	0.08	0.00	1.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	10	7.42	1	0.58	0.08	0.00	1.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	1	8.50	1	0.52	0.06	0.00	1.03	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	2	8.20	1	0.53	0.06	0.00	1.15	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	3	7.98	1	0.55	0.07	0.00	1.26	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	1	7.61	1	0.57	0.07	0.00	1.47	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	2	7.58	1	0.57	0.07	0.00	1.49	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	3	7.55	1	0.57	0.08	0.00	1.51	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	4	7.53	1	0.58	0.08	0.00	1.52	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	5	7.51	1	0.58	0.08	0.00	1.54	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	6	7.49	1	0.58	0.08	0.00	1.55	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	7	7.47	1	0.58	0.08	0.00	1.56	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00



4	8	7.46	1	0.58	0.08	0.00	1.57	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	9	7.44	1	0.58	0.08	0.00	1.58	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	10	7.43	1	0.58	0.08	0.00	1.59	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	1	7.42	1	0.88	0.08	0.00	1.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	2	7.42	1	1.17	0.08	0.00	1.60	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	3	7.41	1	1.17	0.08	0.00	1.61	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	4	7.40	1	1.17	0.08	0.00	1.61	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	5	7.40	1	1.17	0.08	0.00	1.61	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

STREAM QUALITY SIMULATION  
QUAL-2E STREAM QUALITY ROUTING MODEL

OUTPUT PAGE NUMBER 4  
Version 3.22 -- May 1996

\*\*\*\*\* STEADY STATE SIMULATION \*\*\*\*\*

\*\* REACTION COEFFICIENT SUMMARY \*\*

RCH NUM	ELE NUM	DO SAT	K2 OPT	OXYGN REAIR	BOD DECAY	BOD SETT	SOD RATE	ORGN DECAY	ORGN SETT	NH3 DECAY	NH3 SRCE	NO2 DECAY	ORGP DECAY	ORGP SETT	DISP SRCE	COLI DECAY	ANC DECAY	ANC SETT	ANC SRCE
		MG/L		1/DAY	1/DAY	1/DAY	G/M2D	1/DAY	1/DAY	1/DAY	MG/M2D	1/DAY	1/DAY	1/DAY	MG/M2D	1/DAY	1/DAY	1/DAY	MG/M2D
5	6	7.40	1	1.17	0.08	0.00	1.62	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	7	7.39	1	1.17	0.08	0.00	1.62	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	8	7.39	1	1.17	0.08	0.00	1.62	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	9	7.39	1	1.17	0.08	0.00	1.62	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	10	7.38	1	1.17	0.08	0.00	1.62	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	11	7.38	1	1.17	0.08	0.00	1.63	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	12	7.38	1	1.17	0.08	0.00	1.63	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	13	7.38	1	1.17	0.08	0.00	1.63	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	14	7.38	1	1.17	0.08	0.00	1.63	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	15	7.38	1	1.17	0.08	0.00	1.63	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	16	7.38	1	1.17	0.08	0.00	1.63	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

STREAM QUALITY SIMULATION  
 QUAL-2E STREAM QUALITY ROUTING MODEL

OUTPUT PAGE NUMBER 5  
 Version 3.22 -- May 1996

\*\*\*\*\* STEADY STATE SIMULATION \*\*\*\*\*

\*\* WATER QUALITY VARIABLES \*\*

RCH NUM	ELE NUM	TEMP DEG-C	CM-1 TDS MG/L	CM-2	CM-3	DO MG/L	BOD MG/L	ORGN MG/L	NH3N MG/L	NO2N MG/L	NO3N MG/L	SUM-N MG/L	ORGP MG/L	DIS-P MG/L	SUM-P MG/L	COLI #/100ML	ANC	CHLA UG/L
1	1	22.23	0.00	0.00	0.00	3.13	8.84	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1	2	24.02	0.00	0.00	0.00	3.16	8.67	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1	3	25.44	0.00	0.00	0.00	3.12	8.49	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1	4	26.55	0.00	0.00	0.00	3.04	8.31	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1	5	27.42	0.00	0.00	0.00	2.92	8.13	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1	6	28.09	0.00	0.00	0.00	2.79	7.94	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1	7	28.62	0.00	0.00	0.00	2.66	7.75	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1	8	29.02	0.00	0.00	0.00	2.52	7.57	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1	9	29.33	0.00	0.00	0.00	2.40	7.39	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1	10	29.57	0.00	0.00	0.00	2.29	7.21	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	1	29.75	0.00	0.00	0.00	2.18	7.11	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	2	29.81	0.00	0.00	0.00	2.03	8.07	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	3	28.48	0.00	0.00	0.00	1.33	24.17	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	4	28.85	0.00	0.00	0.00	1.02	23.71	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	5	29.15	0.00	0.00	0.00	0.74	23.24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	6	29.39	0.00	0.00	0.00	0.49	22.79	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	7	29.58	0.00	0.00	0.00	0.28	22.33	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	8	29.73	0.00	0.00	0.00	0.10	21.88	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	9	29.85	0.00	0.00	0.00	0.00	21.42	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
2	10	29.86	0.00	0.00	0.00	0.00	20.66	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	1	22.38	0.00	0.00	0.00	3.13	8.83	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	2	24.26	0.00	0.00	0.00	3.15	8.68	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	3	25.80	0.00	0.00	0.00	2.97	8.95	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	1	28.40	0.00	0.00	0.00	1.17	15.50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	2	28.63	0.00	0.00	0.00	1.12	15.34	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	3	28.84	0.00	0.00	0.00	1.07	15.17	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	4	29.02	0.00	0.00	0.00	1.03	15.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	5	29.19	0.00	0.00	0.00	0.98	14.84	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	6	29.33	0.00	0.00	0.00	0.93	14.67	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	7	29.46	0.00	0.00	0.00	0.89	14.51	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

4	8	29.57	0.00	0.00	0.00	0.84	14.35	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	9	29.66	0.00	0.00	0.00	0.81	14.19	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4	10	29.75	0.00	0.00	0.00	0.82	14.01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	1	29.85	0.00	0.00	0.00	1.50	13.67	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	2	29.89	0.00	0.00	0.00	1.98	13.47	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	3	29.93	0.00	0.00	0.00	2.38	13.28	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	4	29.97	0.00	0.00	0.00	2.73	13.09	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	5	30.00	0.00	0.00	0.00	3.02	12.90	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

STREAM QUALITY SIMULATION  
 QUAL-2E STREAM QUALITY ROUTING MODEL

OUTPUT PAGE NUMBER 6  
 Version 3.22 -- May 1996

\*\*\*\*\* STEADY STATE SIMULATION \*\*\*\*\*

\*\* WATER QUALITY VARIABLES \*\*

RCH NUM	ELE NUM	TEMP DEG-C	CM-1 TDS MG/L	CM-2	CM-3	DO MG/L	BOD MG/L	ORGN MG/L	NH3N MG/L	NO2N MG/L	NO3N MG/L	SUM-N MG/L	ORGP MG/L	DIS-P MG/L	SUM-P MG/L	COLI #/100ML	ANC	CHLA UG/L
5	6	30.03	0.00	0.00	0.00	3.26	12.71	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	7	30.06	0.00	0.00	0.00	3.47	12.53	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	8	30.09	0.00	0.00	0.00	3.65	12.35	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	9	30.11	0.00	0.00	0.00	3.80	12.17	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	10	30.13	0.00	0.00	0.00	3.93	12.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	11	30.15	0.00	0.00	0.00	4.04	11.83	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	12	30.16	0.00	0.00	0.00	4.14	11.66	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	13	30.18	0.00	0.00	0.00	4.22	11.51	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	14	30.19	0.00	0.00	0.00	4.28	11.37	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	15	30.20	0.00	0.00	0.00	4.33	11.27	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	16	30.20	0.00	0.00	0.00	4.35	11.22	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

STREAM QUALITY SIMULATION  
 QUAL-2E STREAM QUALITY ROUTING MODEL

OUTPUT PAGE NUMBER 7  
 Version 3.22 -- May 1996

\*\*\*\*\* STEADY STATE SIMULATION \*\*\*\*\*

\*\* DISSOLVED OXYGEN DATA \*\*

			COMPONENTS OF DISSOLVED OXYGEN MASS BALANCE (MG/L-DAY)												
ELE ORD	RCH NUM	ELE NUM	TEMP DEG-C	DO SAT MG/L	DO MG/L	DO DEF MG/L	DAM INPUT MG/L	NIT INHIB FACT	F-FNCTN INPUT	OXYGN REAIR	C-BOD	SOD	NET P-R	NH3-N	NO2-N
1	1	1	22.23	8.53	3.13	5.39	0.00	0.00	9.27	2.79	-0.49	-1.42	0.00	0.00	0.00
2	1	2	24.02	8.24	3.16	5.08	0.00	0.00	0.00	2.71	-0.52	-1.57	0.00	0.00	0.00
3	1	3	25.44	8.03	3.12	4.90	0.00	0.00	0.00	2.67	-0.55	-1.71	0.00	0.00	0.00
4	1	4	26.55	7.87	3.04	4.83	0.00	0.00	0.00	2.68	-0.56	-1.82	0.00	0.00	0.00
5	1	5	27.42	7.74	2.92	4.82	0.00	0.00	0.00	2.71	-0.57	-1.92	0.00	0.00	0.00
6	1	6	28.09	7.65	2.79	4.86	0.00	0.00	0.00	2.76	-0.58	-2.00	0.00	0.00	0.00
7	1	7	28.62	7.58	2.66	4.93	0.00	0.00	0.00	2.82	-0.58	-2.06	0.00	0.00	0.00
8	1	8	29.02	7.53	2.52	5.00	0.00	0.00	0.00	2.88	-0.57	-2.11	0.00	0.00	0.00
9	1	9	29.33	7.49	2.40	5.09	0.00	0.00	0.00	2.95	-0.57	-2.14	0.00	0.00	0.00
10	1	10	29.57	7.46	2.29	5.17	0.00	0.00	0.00	3.01	-0.56	-2.17	0.00	0.00	0.00
11	2	1	29.75	7.43	2.18	5.25	0.00	0.00	0.00	3.06	-0.56	-2.20	0.00	0.00	0.00
12	2	2	29.81	7.43	2.03	5.40	0.00	0.00	0.00	3.15	-0.63	-2.21	0.00	0.00	0.00
13	2	3	28.48	7.60	1.33	6.27	0.00	0.00	0.07	3.58	-1.78	-2.04	0.00	0.00	0.00
14	2	4	28.85	7.55	1.02	6.53	0.00	0.00	0.00	3.76	-1.78	-2.08	0.00	0.00	0.00
15	2	5	29.15	7.51	0.74	6.77	0.00	0.00	0.00	3.91	-1.77	-2.12	0.00	0.00	0.00
16	2	6	29.39	7.48	0.49	6.99	0.00	0.00	0.00	4.05	-1.75	-2.15	0.00	0.00	0.00
17	2	7	29.58	7.46	0.28	7.17	0.00	0.00	0.00	4.17	-1.73	-2.17	0.00	0.00	0.00
18	2	8	29.73	7.44	0.10	7.34	0.00	0.00	0.00	4.28	-1.71	-2.19	0.00	0.00	0.00
19	2	9	29.85	7.42	0.00	7.42	0.00	0.00	0.00	4.33	-1.68	-2.21	0.00	0.00	0.00
20	2	10	29.86	7.42	0.00	7.42	0.00	0.00	0.00	4.33	-1.63	-2.21	0.00	0.00	0.00
21	3	1	22.38	8.50	3.13	5.37	0.00	0.00	8.53	2.79	-0.49	-1.43	0.00	0.00	0.00
22	3	2	24.26	8.20	3.15	5.06	0.00	0.00	0.00	2.70	-0.53	-1.60	0.00	0.00	0.00
23	3	3	25.80	7.98	2.97	5.00	0.00	0.00	0.00	2.74	-0.58	-1.75	0.00	0.00	0.00
24	4	1	28.40	7.61	1.17	6.44	0.00	0.00	0.00	3.67	-1.14	-2.02	0.00	0.00	0.00
25	4	2	28.63	7.58	1.12	6.45	0.00	0.00	0.00	3.70	-1.14	-2.05	0.00	0.00	0.00
26	4	3	28.84	7.55	1.07	6.48	0.00	0.00	0.00	3.72	-1.14	-2.08	0.00	0.00	0.00
27	4	4	29.02	7.53	1.03	6.50	0.00	0.00	0.00	3.75	-1.14	-2.10	0.00	0.00	0.00
28	4	5	29.19	7.51	0.98	6.53	0.00	0.00	0.00	3.77	-1.13	-2.12	0.00	0.00	0.00
29	4	6	29.33	7.49	0.93	6.56	0.00	0.00	0.00	3.80	-1.13	-2.14	0.00	0.00	0.00

30	4	7	29.46	7.47	0.89	6.58	0.00	0.00	0.00	3.82	-1.12	-2.15	0.00	0.00	0.00
31	4	8	29.57	7.46	0.84	6.61	0.00	0.00	0.00	3.84	-1.11	-2.17	0.00	0.00	0.00
32	4	9	29.66	7.44	0.81	6.64	0.00	0.00	0.00	3.86	-1.11	-2.18	0.00	0.00	0.00
33	4	10	29.75	7.43	0.82	6.61	0.00	0.00	0.00	3.86	-1.10	-2.19	0.00	0.00	0.00
34	5	1	29.85	7.42	1.50	5.92	0.00	0.00	0.00	5.18	-1.07	-1.35	0.00	0.00	0.00
35	5	2	29.89	7.42	1.98	5.43	0.00	0.00	0.00	6.35	-1.06	-1.35	0.00	0.00	0.00
36	5	3	29.93	7.41	2.38	5.03	0.00	0.00	0.00	5.88	-1.05	-1.36	0.00	0.00	0.00
37	5	4	29.97	7.40	2.73	4.68	0.00	0.00	0.00	5.48	-1.03	-1.36	0.00	0.00	0.00
38	5	5	30.00	7.40	3.02	4.39	0.00	0.00	0.00	5.14	-1.02	-1.36	0.00	0.00	0.00

STREAM QUALITY SIMULATION  
 QUAL-2E STREAM QUALITY ROUTING MODEL

OUTPUT PAGE NUMBER 8  
 Version 3.22 -- May 1996

\*\*\*\*\* STEADY STATE SIMULATION \*\*\*\*\*

\*\* DISSOLVED OXYGEN DATA \*\*

										COMPONENTS OF DISSOLVED OXYGEN MASS BALANCE (MG/L-DAY)						
ELE	RCH	ELE		DO	DO	DAM	NIT			F-FNCTN	OXYGN			NET		
ORD	NUM	NUM	TEMP	SAT	DO	DEF	INPUT	INHIB		INPUT	REAIR	C-BOD	SOD	P-R	NH3-N	NO2-N
			DEG-C	MG/L	MG/L	MG/L	MG/L	FACT								
39	5	6	30.03	7.40	3.26	4.14	0.00	0.00		0.00	4.84	-1.01	-1.36	0.00	0.00	0.00
40	5	7	30.06	7.39	3.47	3.92	0.00	0.00		0.00	4.60	-0.99	-1.37	0.00	0.00	0.00
41	5	8	30.09	7.39	3.65	3.74	0.00	0.00		0.00	4.39	-0.98	-1.37	0.00	0.00	0.00
42	5	9	30.11	7.39	3.80	3.59	0.00	0.00		0.00	4.21	-0.97	-1.37	0.00	0.00	0.00
43	5	10	30.13	7.38	3.93	3.45	0.00	0.00		0.00	4.05	-0.96	-1.37	0.00	0.00	0.00
44	5	11	30.15	7.38	4.04	3.34	0.00	0.00		0.00	3.92	-0.94	-1.37	0.00	0.00	0.00
45	5	12	30.16	7.38	4.14	3.24	0.00	0.00		0.00	3.81	-0.93	-1.38	0.00	0.00	0.00
46	5	13	30.18	7.38	4.22	3.16	0.00	0.00		0.00	3.71	-0.92	-1.38	0.00	0.00	0.00
47	5	14	30.19	7.38	4.28	3.10	0.00	0.00		0.00	3.64	-0.91	-1.38	0.00	0.00	0.00
48	5	15	30.20	7.38	4.33	3.05	0.00	0.00		0.00	3.58	-0.90	-1.38	0.00	0.00	0.00
49	5	16	30.20	7.38	4.35	3.03	0.00	0.00		0.00	3.55	-0.90	-1.38	0.00	0.00	0.00

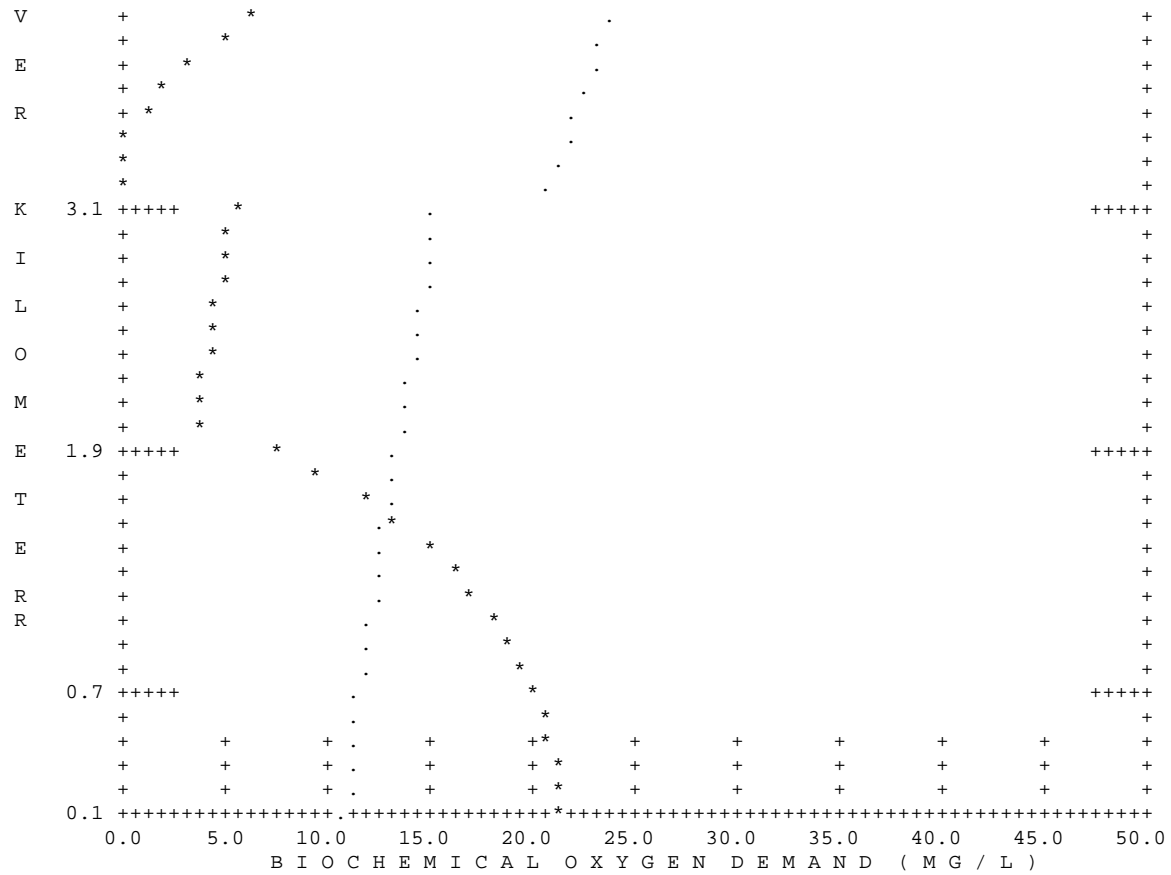
		D I S S O L V E D O X Y G E N ( M G / L )										
		0.0	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0	10.0
R	5.6	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++
		+	+	.	+	+	+	+	+	+	+	+
I		+	+	.	+	+	+	+	+	+	+	+
		+	+	.	+	+	+	+	+	+	+	+
V		+		.	*							+
		+		.	*							+
E		+	+	.	+	+	+	+	+	+	+	+
		+	+	.	+	+	+	+	+	+	+	+
R		+	+	.	+	+	+	+	+	+	+	+
	4.5	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++
		0.0	5.0	10.0	15.0	20.0	25.0	30.0	35.0	40.0	45.0	50.0
		B I O C H E M I C A L O X Y G E N D E M A N D ( M G / L )										

DISSOLVED OXYGEN = \* \* \* \*  
 BIOCHEMICAL OXYGEN DEMAND = . . . .

		D I S S O L V E D O X Y G E N ( M G / L )										
		0.0	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0	10.0
R	3.5	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++
		+	+	.	+	+	+	+	+	+	+	+
I	3.3	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++
		0.0	5.0	10.0	15.0	20.0	25.0	30.0	35.0	40.0	45.0	50.0
		B I O C H E M I C A L O X Y G E N D E M A N D ( M G / L )										

DISSOLVED OXYGEN = \* \* \* \*  
 BIOCHEMICAL OXYGEN DEMAND = . . . .

		D I S S O L V E D O X Y G E N ( M G / L )										
		0.0	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0	10.0
	5.6	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++	+++++
		+	+	.	+	+	+	+	+	+	+	+
		+	+	.	+	+	+	+	+	+	+	+
		+	+	.	+	+	+	+	+	+	+	+
		+		.	*							+
		+		.	*							+
		+		.	*							+
		+		.	*							+
R		+		.	*							+
		+		.	*							+
I	4.4	+++++		.	*							+++++
		+		.	*							+



DISSOLVED OXYGEN = \* \* \* \*  
 BIOCHEMICAL OXYGEN DEMAND = . . . .